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#### STATE OF HAWAII

DEPARTMENT OF ACCOUNTING AND GENERAL SERVICES

LETTER NOPM-1473.2

P. O. BOX 119, HONOLULU, HAWAII 96810

NOV 19 1992

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Mr. Brian Choy
Director
Office of Environmental
Quality Control
220 South King Street, 4th Floor
Honolulu, Hawaii 96813

OFC. OF THE STATE

Dear Mr. Choy:

Subject: Maui Community College

New Buildings S and J

D.A.G.S. Job No. 15-31-4953

TMK: 3-8-7:40, 125, Kahului, Maui, Hawaii

In accordance with the requirements of Chapter 343, Hawaii Revised Statutes, and Chapter 200 of Title 11, Administrative Rules, a Draft Environmental Assessment has been prepared for the subject project.

As the proposing agency, we believe that there will be no significant impacts as a result of the project and anticipate the filing of a negative declaration. One copy of the OEQC Bulletin Publication Form and four copies of the Draft Environmental Assessment will be submitted to your office under separate cover. We respectfully request that notice of the Draft Environmental Assessment be published in the December 8, 1992 OEQC Bulletin.

If there are any questions, please contact Mr. Eric Nishimoto at 586-0468.

Very truly yours,

GORDON MATSUOKA

tate Public Works Engineer

EN/lc

FILE COPY

## Maui Community College Buildings "J" and "S"

# Final Environmental Assessment and Negative Declaration

Prepared for:



January 1993



## Maul Community College Buildings "J" and "S"

# Final Environmental Assessment and Negative Declaration

Prepared for:



January 1993



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#### <u>Preface</u>

The State of Hawaii, Department of Accounting and General Services, proposes to construct two new buildings, designated as Buildings "J" and "S", parking lot expansion and a new retention basin drainage system, for Maui Community College (TMK 3-8-07:40, 125) located in Kahului, Maui, Hawaii. Pursuant to Chapter 343, Hawaii Revised Statutes, and Chapter 200, of Title 11, Administrative Rules, Environmental Impact Statement Rules, this Final Environmental Assessment documents the project's technical characteristics and environmental impacts, and advances findings and conclusions relative to the significance of the project.

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#### Final Environmental Assessment and Negative Declaration **Summary**

#### Applicant and Landowner

The Applicant for the proposed project is the State of Hawaii, Department of Accounting and General Services. The landowner of the affected property is the University of Hawaii.

#### Contact Person

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For further information, contact Eric Nishimoto, State of Hawaii, Department of Accounting and General Services, Kalanimoku Building, 1151 Punchbowl Street, Honolulu, Hawaii 96813, or at telephone (808) 586-0468.

#### **Property Location and Description**

The proposed project involves lands near the periphery of the Mauí Community College (MCC) Campus, in Kahului, Maui, Hawaii (TMK 3-8-7:40 and 125).

Lands to the north and northeast of the campus are occupied by the Maui Community Arts and Cultural Center, Kahului Beach Road, Kahului Harbor, and Pacific Ocean. Lands to the east of the campus, are occupied by the Harbor Lights Condominium complex. Kaahumanu Avenue forms the southern border of the MCC campus. Kaahumanu Shopping Center and the Kahului residential area lie beyond Kaahumanu Avenue. To the west of the MCC campus, lies the War Memorial Center and Baldwin High School.

#### Proposed Action

The proposed project involves the construction of two new structures on the MCC campus, Buildings "J" and "S", as well as the expansion of existing main parking lot and a new retention basin.

Building "J" is proposed to be located on the west side of the MCC campus, adjacent to the existing library. Building "J" would be 2 stories and 60 feet in height, containing approximately 35,000 square feet of building area. The building would contain a lecture hall, 7 classrooms, a multi-purpose room, faculty office spaces, workrooms and storage spaces.

Building "S" is proposed to be located in the northeast section of campus on a portion of an existing main parking lot. It is proposed to be 2 stories and 66 feet in height, containing approximately 33,000 square feet of building area. Building "S" would provide space for 6 classrooms, 3 computer labs, a desktop publishing room, reading room, office spaces, and administrative and storage spaces.

The existing 397 stall main parking lot, which is located north of the existing Student Center Building, will be expanded to the northeast. Approximately 171 parking stalls would be displaced by the construction of Building "S" and the parking lot expansion. With restriping and expansion of the parking lot, a total of approximately 640 parking stalls are proposed to meet parking needs generated by increased floor area and to comply with County of Maui parking requirements.

An off-site retention basin is being proposed on an adjacent property (TMK 3-8-07:125). This will consist of a four feet deep pond with two feet of freeboard, approximately 140 feet wide and 900 feet long.

#### **Determination**

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Construction of the proposed project will involve short-term environmental impacts typically associated with construction activities. These include air quality and noise impacts. Dust control measures such as watering or sprinkling will be undertaken to minimize dust. Construction activities are also anticipated to be limited to daylight hours. Impacts generated from construction activities are not considered adverse.

From a long-term perspective, the proposed project is not anticipated to result in adverse environmental impacts. The sites for Buildings "J" and "S" have already been extensively modified. No significant archaeological or historic sites were found. There are no known rare/threatened species of flora and fauna at the project site.

Impacts to the local economy are anticipated to be positive involving support for construction-related employment in the short-term. After completion of construction, the project will provide liberal arts, vocational and technical training to benefit the local employment market.

With regard to traffic, there are a number of proposed new roadways in the region which provide alternative travel routes in the Wailuku-Kahului area. This relieves much of the traffic congestion on Kaahumanu Avenue in the project vicinity. In addition, there are a number of intersection improvements and site access recommendations designed to mitigate increases in traffic due to the project. With the implementation of these recommendations, the proposed project should not have any significant impact on traffic in the vicinity of the project.

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With regard to drainage improvements, the proposed project involves the construction of a retention basin which collects runoff and controls discharge into Kahului Harbor. The ultimate discharge rate would not exceed existing discharge levels.

The project is also not anticipated to have adverse impacts upon medical, police and fire protection services as well as other infrastructure systems.

In light of the foregoing findings, it is concluded that the proposed action will not result in any significant impacts. Thus, this Final Environmental Assessment is being filed as a negative declaration.

# Chapter I

Project Overview

#### I. PROJECT OVERVIEW

#### A. PROPERTY LOCATION, BACKGROUND AND LAND OWNERSHIP

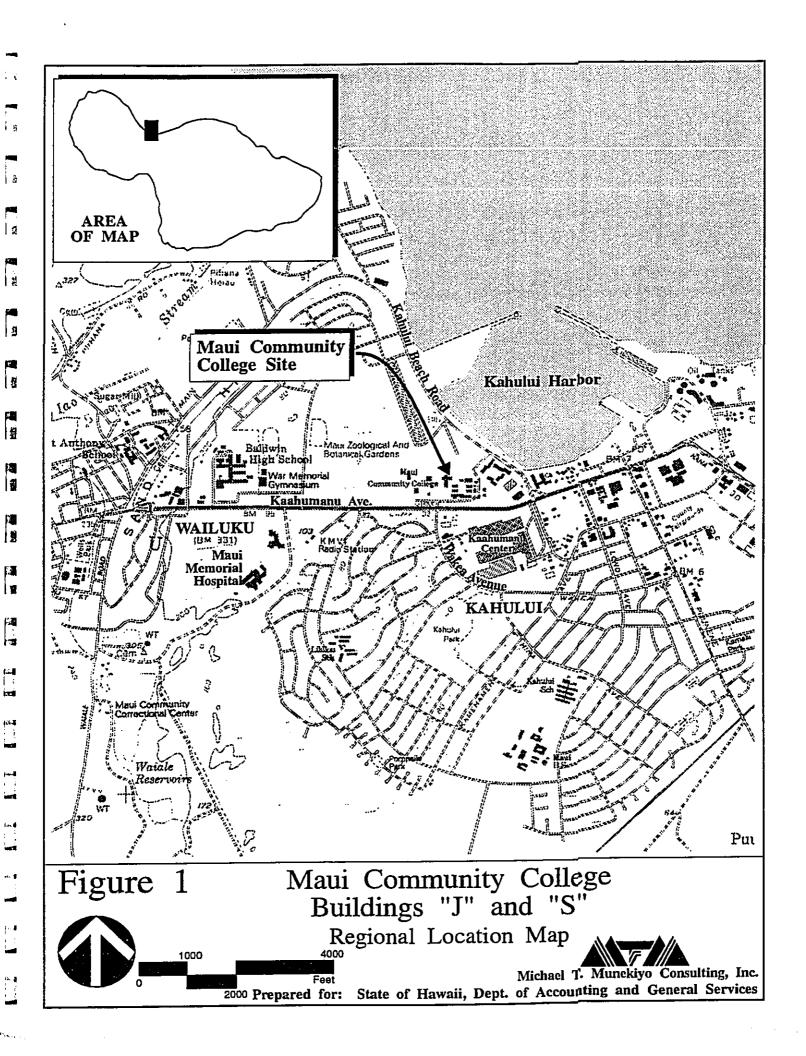
The applicant, the State of Hawaii, Department of Accounting and General Services (DAGS), proposes to construct two new buildings, designated as Buildings "J" and "S", on the Maui Community College (MCC) campus (TMK 3-8-07:40) located in Kahului, Maui, Hawaii. Along with these two new buildings, DAGS proposes to construct an extension of the existing parking lot on the northeastern portion of the campus and a retention basin drainage system on an abutting property (TMK 3-8-07:125). See Figure 1.

The history of MCC dates back to 1931, when it was called Maui Vocational School, and offered courses in carpentry, auto mechanics and machine shop. In 1950-51, the physical plant expanded and new programs were added. In 1958, the school changed its name to Maui Technical School.

In 1964, the State Legislature established the statewide community college system as part of the University of Hawaii. In 1966, the school's name was changed to Maui Community College. Course offerings were broadened to include vocational, technical as well as liberal arts classes. Currently, MCC services approximately 2,800 full and part-time students housed in 32 buildings.

In the long-term, MCC's master plan anticipates future expansion of facilities and programs. This expansion could provide for additional classroom space to accommodate as many as 5,000 full-time equivalent students by the year 2000.

The MCC site (TMK 3-8-07:40), as well as lands on which the retention basin are proposed to be located (TMK 3-8-07:125), are owned by the University of Hawaii.



#### B. PROPOSED ACTION

#### 1. Project Need

At its present enrollment, the MCC campus exceeds capacity. During the past three years, student population has increased by approximately 35 percent. Many faculty members do not have offices of their own or must use storerooms and closets for office space. Moreover, classes which might otherwise be offered are commonly not made available for students because of lack of classroom and lecture hall space. The proposed Buildings "J" and "S" are necessary in order to help meet the increase in student population as well as faculty needs.

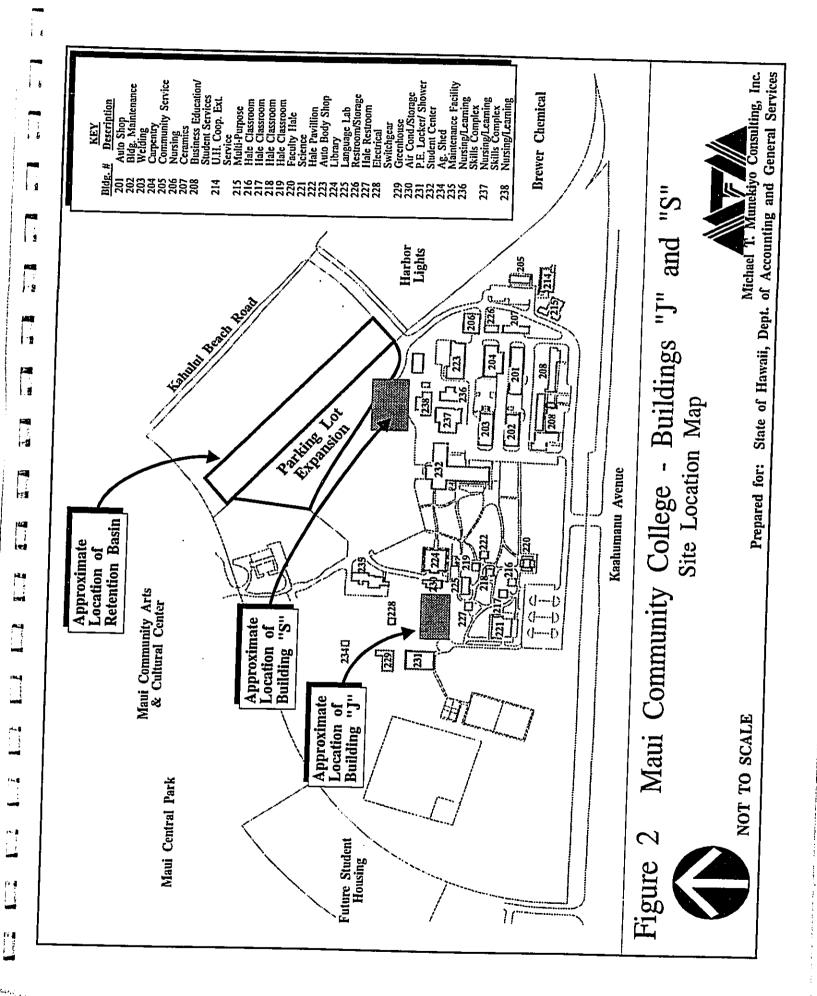
#### 2. Proposed improvements

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Building "J", is proposed to be located on the west side of the MCC campus adjacent to the existing library. Building "J" would be 2 stories and 60 feet in height, containing approximately 35,000 square feet of building area. Building "J" contains a lecture hall, 7 classrooms, a multi-purpose room, faculty office spaces, workrooms and storage spaces. See Figure 2.

Building "S" is proposed to be located in the northeast section of campus. It is proposed to be 2 stories and 66 feet in height, containing approximately 33,000 square feet of building area. Building "S" would provide space for 6 classrooms, 3 computer labs, a desktop publishing room, reading room, office spaces, and administrative and storage spaces.

The existing 397 stall main parking lot, which is located north of the existing Student Center Building, will be expanded to the northeast. Approximately 171 parking stalls will be displaced by the construction



of Building "S" and the parking lot expansion. With restriping and expansion of the parking lot, a total of approximately 640 parking stalls are proposed to meet parking needs generated by increased floor area and to comply with County of Maui parking requirements.

An off-site retention basin is being proposed on an adjacent property (TMK 3-8-07:125). This will consist of a four feet deep pond with two feet of freeboard, approximately 140 feet wide and 900 feet long.

The total cost of all improvements for the project is estimated to be \$21.6 million. Assuming all applicable permits are obtained, construction is projected to start in July 1993 and be completed by January 1995.

# Chapter II

Description of the Existing Environment

### II. DESCRIPTION OF THE EXISTING ENVIRONMENT

#### A. PHYSICAL ENVIRONMENT

#### 1. Surrounding Land Uses

MCC is located in the heart of Kahului, the Island of Maui's center of commerce. Kahului is home to Kahului Harbor, the Island's only deep water port, and the Kahului Airport, the second busiest airport in the State. With its proximity to the Harbor and Airport, the Kahului region has emerged as the focal point for heavy industrial, light industrial and commercial activities and services such as warehousing, baseyard operations, automotive sales and maintenance, and retailing for equipment and materials suppliers. The region is considered Central Maui's commercial retailing center with the Kaahumanu Center, the Maui Mall and the Kahului Shopping Center, located within a mile of MCC.

MCC, the only Community College on Maui, is also centrally located in Kahului.

Surrounding this commercial core is an expansive residential area comprised principally of single-family residential units. Residential uses encompass the area extending from Maui Memorial Hospital to Puunene Avenue.

Building "J" is proposed to be located in the western portion of the MCC campus between the existing library and physical education locker/shower rooms.

Building "S", the parking lot extension and retention basin would be located in the northeast portion of campus. A portion of the parking lot extension and the retention basin extend into vacant lands

vegetated by koa haole, kiawe, and various exotic shrubs. These improvements would be adjacent to an existing access road which connects MCC's existing parking lot with Kahului Beach Road. Kahului Harbor and the Pacific Ocean borders the Kahului Beach Road. Lands to the southeast of the access road are occupied by the Harbor Lights Condominium. To the northwest of the site is the Maui Community Arts and Cultural Center Project which is currently under construction.

#### 2. Climate

Like most areas of Hawaii, Maui's climate is relatively uniform year-round. Characteristic of Hawaii's climate, the project site experiences mild and uniform temperatures year round, moderate humidity and a relatively consistent northeasterly tradewind. Variation in climate on the Island is largely left to local terrain.

Average temperatures at the project site (based on temperatures recorded at Kahului Airport) range from lows in the 60°s to highs in the 80°s. August is historically the warmest month, while January and February are the coolest. Rainfall at the project site averages approximately 20 inches per year. Winds in the Kahului region are predominantly out of the north-northeast and northeast.

#### 3. Topography and Soil Characteristics

The project site is located on Maui's flat central isthmus ranging in elevations from 8 to 40 feet. The high point, along the west side of campus, near Kaahumanu Avenue, gently slopes down to the northeast side of campus. There are no significant topographical constraints within the project site.

Underlying the proposed site and surrounding lands are soils belonging to the Pulehu-Ewa-Jaucas association. See Figure 3. This soil association is characteristically deep and well-drained and located on alluvial fans and in basins. The soil type specific to the project site is of the Puuone Series' Puuone Sand classification (PZUE). See Figure 4. PZUE soils predominate in the Kahului region and is typified by a sandy surface layer underlain by cemented sand. Naturally occurring vegetation on this series include bermuda grass, kiawe, and lantana.

#### 4. Flood and Tsunami Hazard

The sites for Buildings "J" and "S" are designated Zone "C" by the Flood Insurance Rate Map. See Figure 5. Zone "C" is an area of minimal flooding. Most of the area for the extension of the parking lot is also designated as Zone C. However, portions of the parking lot extension and the retention basin, are designated as Zone "A-4" (areas inundated by the 100-year flood with a base flood elevation of 17 feet above mean sea level). The makai portion of the retention basin is designated as Zone "V23" (areas inundated by the 100-year coastal flood with velocity hazards and a base flood elevation of 17-18 feet above mean sea level).

#### 5. Flora and Fauna

Surrounding the project site to the south and east is the urbanized center of Kahului. Areas of the MCC campus that encompass the projects are characteristic of the urban nature of Kahului. Lands on campus are landscaped with palm trees and other shade trees, ground cover, and other exotic vegetation. Lands north and west of

Base flood elevations for Zones "A-4" and "V23" were confirmed by Steven Yamamoto of the Corps of Engineers (telephone conversation, November 23, 1992).

#### **LEGEND**

1 Pulchu-Ewa-Jaucas association

Waiakoa-Keahua-Molokai association

3 Honolun-Olelo association

4 Rock land-Rough mountainous land association

(5) Puu Pa-Kula-Pane association

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6 Hydrandepts-Tropaquods association

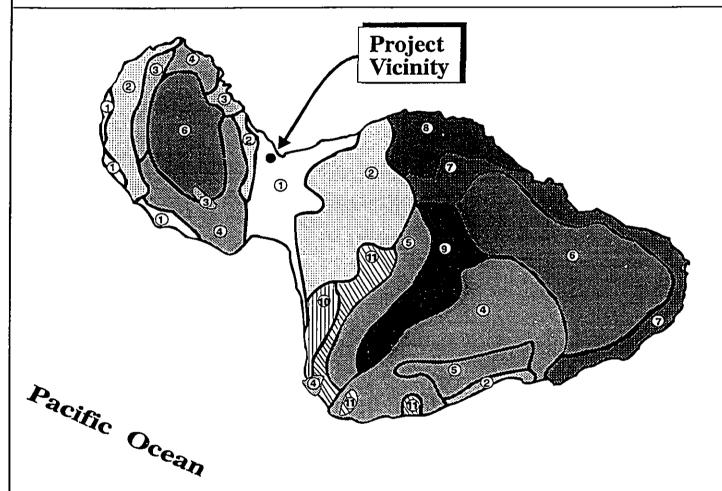
7 Hana-Makaalae-Kailua association

Pauwela-Haiku association

-9 -- Laumaia-Kaipoipoi-Olinda association

Keawakapu-Makena association

Kamaole-Oanapuka association



Map Source: USDA Soil Conservation Service

Figure 3



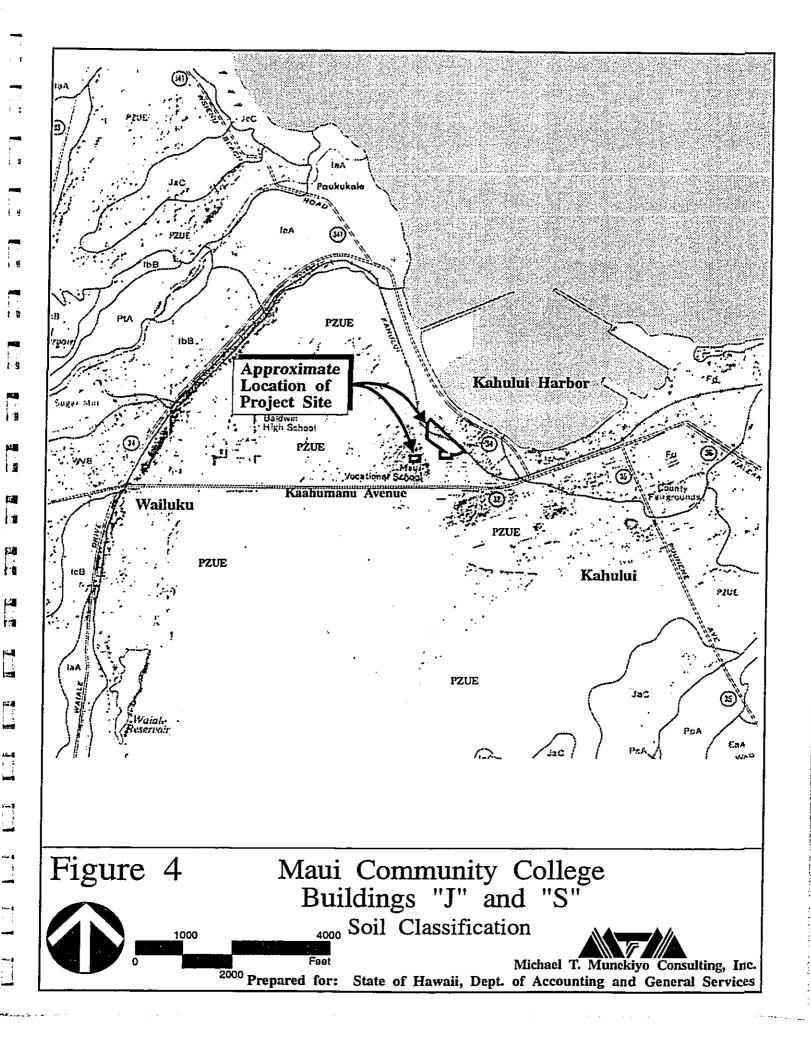
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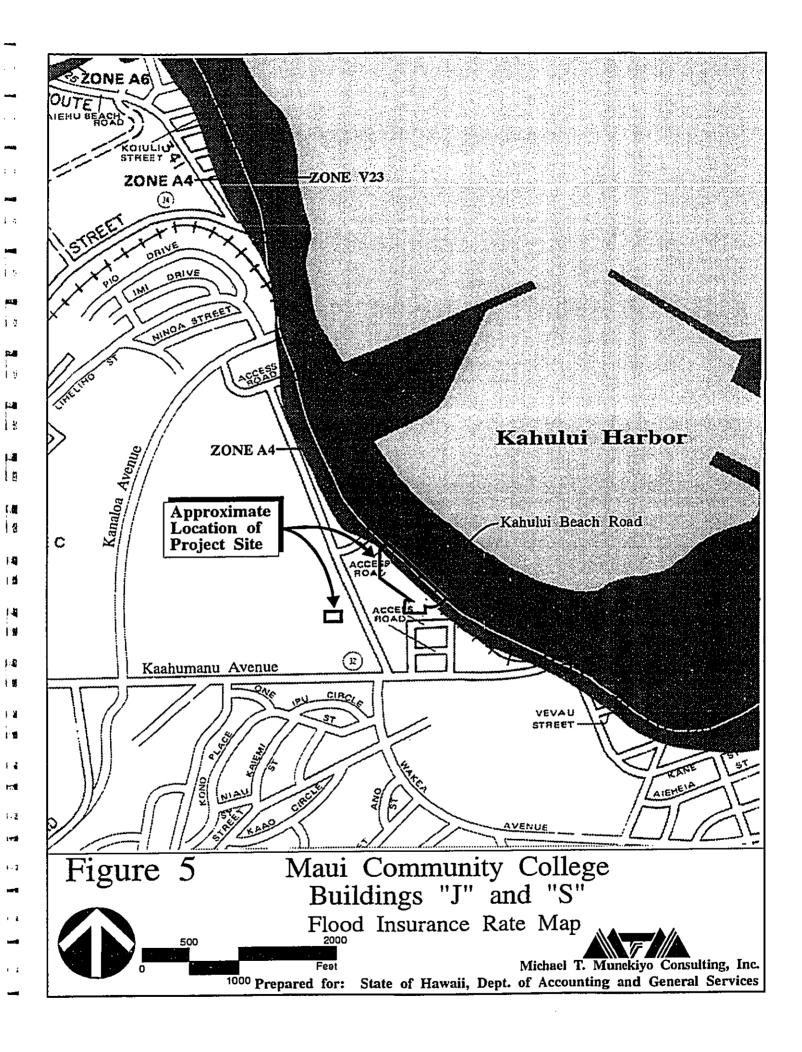
Maui Community College Buildings "J" and "S" Soil Association Map

Michael T Munekiyo Const

Michael T. Munekiyo Consulting, Inc.

Prepared for: State of Hawaii, Dept. of Accounting and General Services





the parking lot extension and the retention basin contain koa haole, kiawe, bermuda grass, fingergrass and lowland shrubs. There are no known rare, endangered or threatened species of plants within the project sites.

Fauna and avifauna are also characteristic of urban areas. Fauna typically found in the vicinity include mongoose, rats, dogs and cats. Avifauna typically include mynas, several types of doves, and house sparrows. There are no rare or endangered species of fauna or avifauna found at the project site.

#### 6. Archaeological Resources

An archaeological inventory survey was conducted for the area of the proposed parking lot which extends beyond the existing parking area and the area of the proposed retention basin. See Appendix A.

The study notes that the expansion area contains considerable quantities of refuse as well as remains of various poured, concrete floors and foundations of buildings which were associated with the 18th U.S.M.C. Service Battalion camp during World War II. The study also found other structural and concrete floor slabs which form a horseshoe-shaped flower bed and sidewalk. The property exhibits considerable signs of surface and subsurface disturbance from earthmoving equipment. Landfill materials have also been freely deposited on the site. Other materials occupying the site include discarded construction material and equipment, rusted automobiles and parts, and household litter.

The study included a surface observation walkover by archaeological personnel. In addition, 22 backhoe test trenches were excavated,

recorded and mapped. No recognizable features or identifiable precontact Hawaiian artifacts were recovered from any of the trenches.

#### 7. Air Quality

Air quality in the Wailuku-Kahului region is considered good as point sources (e.g., Maui Electric Power Plant, HC&S Mill) and non-point sources (e.g., automobile emissions) of emission are not significant to generate high concentration of pollutants. The relatively high quality of air can also be attributed to the region's constant exposure to winds which quickly disperse concentrations of emissions. This rapid dispersion is evident during burning of sugar cane in fields located to the southeast of the Kahului residential core.

#### 8. Noise

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Traffic noise is the predominant source of background noise in the vicinity of the projects. To the east, the Kahului Harbor activity can also add to the background noise levels in the surrounding region.

#### 9. <u>Visual Resources</u>

Scenic resources to the west of MCC include Iao Valley and the West Maui Mountain Range. Looking southeast, Haleakala is clearly visible. To the northeast, lies the Kahului Harbor and the Pacific Ocean. South of MCC, the Kahului commercial center is visible.

#### B. SOCIO-ECONOMIC ENVIRONMENT

#### 1. Population

The population of the County of Maui has exhibited relatively strong growth over the past decade with the 1990 population estimated to be 100,374, a 41.7% increase over the 1980 population of 70,847. Growth in the County is expected to continue, with resident

population projections to the years 2000 and 2010, estimated to be 123,900 and 145,200, respectively (DBED,1990).

The Wailuku-Kahului Community Plan region follows the Countywide pattern of population growth, with the region's 1990 population of 32,816 expected to rise to 40,119 by the year 2000 and to 47,597 by the year 2010 (Community Resources, Inc., 1992).

#### 2. Economy

As noted previously, the Kahului region is the Island's center of commerce. Combined with neighboring Wailuku, the region's economic character encompasses a broad range of commercial, service, and governmental activities. In addition, the region is surrounded by significant agricultural acreages which include sugar cane fields, pineapple fields, and macadamia nut orchards. The vast expanse of agricultural land, managed by Hawaiian Commercial & Sugar (HC&S) and Wailuku Agribusiness Company, is considered a key component of the local economy.

#### C. PUBLIC SERVICES

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#### 1. Recreational Facilities

The Wailuku-Kahului region encompasses a full range of recreational opportunities, including shoreline and boating activities at the Kahului Harbor and adjoining beach parks, and individual and organized athletic activities offered at numerous County parks and the War Memorial Complex. MCC is in close proximity to the Kahului Community Center, the County's Kanaha Beach Park and Iao Valley State Park.

#### 2. Police and Fire Protection

Police protection for the Wailuku-Kahului region is provided by the County Police Department headquartered at the Wailuku Station, approximately 0.8 mile from MCC. The region is served by the Department's Central Maui patrol.

Fire prevention, suppression, and protection services for the Wailuku-Kahului region is provided by the County Department of Fire Control's Wailuku Station, located in Wailuku Town, approximately 1.8 miles from MCC. In addition, the Department has constructed a new Kahului Station (located on Dairy Road). Portions of the MCC campus are within the 2.0 mile service radius of the Kahului Station.

#### 3. Solid Waste

Single-family residential solid waste collection service is provided by the County of Maui on a once-a-week basis. Residential solid waste collected by County crews are disposed at the County's 55-acre Central Maui Landfill, located four miles southeast of the Kahului Airport. In addition to County-collected refuse, the Central Maui Landfill accepts commercial waste from private collection companies. Refuse collection for MCC is provided by a private collection company.

#### 4. Health Care

Maui Memorial Hospital, the only major medical facility on the Island, services the Wailuku-Kahului region. Acute, general and emergency care services are provided by the 145-bed facility. In addition, numerous privately operated medical/dental clinics and offices are located in the area to serve the region's residents.

#### 5. Schools

The Wailuku-Kahului region is served by the State Department of Education's public school system as well as several privately operated schools accommodating elementary, intermediate and high school students. Department of Education facilities in the Kahului area include Lihikai and Kahului Schools (Grades K-6), Maui Waena Intermediate School (Grades 7-8), and Maui High School (Grades 9-12). Existing facilities in the Wailuku area include Wailuku Elementary School (Grades K-5), Iao Intermediate School (Grades 6-8), and Baldwin High School (Grades 9-12). MCC, a branch of the University of Hawaii, serves as the Island's only Community College.

#### D. INFRASTRUCTURE

#### 1. Roadways

The Wailuku-Kahului region is served by a roadway network which includes arterial, collector and local roads. Major roadways include Kaahumanu Avenue, the principal linkage between Wailuku and Kahului, Lower Main/Kahului Beach Road, Hana Highway, and Puunene Avenue.

Access to MCC is provided by a primary entry at the four-way signalized intersection of Kaahumanu Avenue and Wakea Avenue. Kaihee Place provides a secondary access to the campus from Kahului Beach Road.

#### 2. Wastewater

Domestic wastewater generated in the Wailuku-Kahului region is conveyed to the County's Wailuku-Kahului Wastewater Reclamation Facility located one-half mile south of Kahului Harbor. The design capacity of the facility is 6.0 million gallons per day (MGD). Average

daily flow currently processed through the plant is approximately 5.3 MGD.

The MCC campus is currently serviced by two separate sewer lines. Wastewater from the west side of campus discharges into a 30-inch sewer trunk line which bisects the campus from its Waiehu border to the area near the intersection of Kaahumanu and Wakea Avenues. Wastewater from the east side of campus gravity flows into an oncampus sewage pump station (SPS) which is then pumped to an existing 24-inch line which extends from Kaahumanu Avenue along Kane Street.

#### 3. Water

The Wailuku-Kahului region is served by the Board of Water Supply's (BWS) domestic water system. Water drawn from the Iao Aquifer System is conveyed to this region for distribution and consumption. The Iao Aquifer, which serves the Central Maui region, has an estimated sustainable yield of 20 MGD. Recent estimates place the monthly average withdrawal from the aquifer at over 18 MGD.

Water service to MCC is provided via a 12-inch waterline located along Kaahumanu Avenue, and a 16-inch waterline that crosses through the campus originating from the Waiehu Heights reservoir.

#### 4. Drainage

The majority of on-site runoff sheet flows across the MCC campus towards Kahului Harbor and under Kahului Beach Road via two offsite drainage ditches and drainline systems. An existing 72-inch by 44-inch pipe, which collects approximately 16 acres of off-site drainage from Kaahumanu Avenue, crosses the campus and outlets

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	into an onsite drainage ditch which connects to the existing off-site ditch and drainline system which flows into Kahului Harbor.
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# Chapter III

Potential Impacts and Mitigation Measures

### III. POTENTIAL IMPACTS AND MITIGATION MEASURES

#### A. PHYSICAL ENVIRONMENT

#### 1. Surrounding Uses

The proposed Buildings "J" and "S" contain classrooms which are compatible to uses within the MCC campus. The project is also compatible with surrounding land uses close to MCC, such as the Kaahumanu Center, Haleakala Dairy, the Maui Land and Pineapple Company cannery, Harbor Lights Condominium, and the single-family residential area adjacent to Wakea Street.

#### 2. Flora and Fauna

There are no known significant habitats or rare, endangered or threatened species of flora and fauna located within the project sites. The proposed project is therefore not considered an adverse impact upon these environmental features.

#### 3. Archaeological Resources

The existing sites of Building "J" and "S" are already part of the MCC campus and have been extensively modified.

An archaeological inventory survey done for the area of parking lot extension and retention basin indicates the extreme land disturbance from previous construction projects on the parcel. Historic references note the unused, barren nature of the land in the vicinity of the study parcel in early historic times. See Appendix A. No cultural features or early artifacts were discovered during the survey. However, remains from the 18th U.S.M.C. Service Battalion camp from World War II were found. It is noted that the U.S.M.C. camp was constructed in 1944, making it less than 50 years old. The

survey concludes that it is very unlikely that significant archaeological materials are present on the property.

Should human osteological material be encountered during construction activities, applicable procedures to ensure compliance with Chapter 6E, HRS, will be followed.

#### 4. Air Quality

Air quality impacts attributed to the project will include dust generated by short-term, construction-related activities. Site work such as grading and utilities and parking lot construction, for example, will generate airborne particulates. Dust control measures such as regular watering and sprinkling will be implemented as needed to minimize wind-blown emissions.

The proposed redevelopment provides additional classroom space which will result in a larger volume of traffic flowing in and out of MCC during school hours. However, since MCC-related traffic represents a relatively small portion of overall traffic activity in the Kahului region, the proposed project is not anticipated to be detrimental to local air quality.

#### 5. Noise

As with air quality, ambient noise conditions will be impacted by construction activities. Heavy construction equipment, such as buildozers, front end loaders, and materials-carrying trucks and trailers, would be the dominant source of noise during the site construction period. Impact tools such as jack hammers and handheld pneumatic tools are also a major source of noise. To aid in the mitigation of construction noise impacts upon surrounding uses,

construction activities will be conducted during the daylight hours only.

On a long term basis, the project will not generate adverse noise conditions.

#### 6. <u>Visual Resources</u>

The addition of the two new buildings to MCC, along with the parking lot expansion and drainage improvements will enhance the visual character of the site and its immediate environs. The project is located mauka of Kahului Beach Road and will not encroach into view corridors along the shoreline.

#### B. SOCIO-ECONOMIC ENVIRONMENT

#### 1. Population and Local Economy

On a short-term basis, the project will support construction and construction-related employment.

On a long-term basis, the project will supply the facilities to provide upper-level educational classes, as well as vocational and technical training to students which will benefit the local employment market. Since MCC has been the fastest-growing community college in the State, the proposed project will help to fill student needs and the demands of the market place.

#### 2. Police, Fire, and Medical Services

Medical, police and fire protection services are not expected to be adversely impacted by the proposed project. The project will not extend existing service area limits for emergency services.

#### 3. Solid Waste

A solid waste management plan will be developed in coordination with the Solid Waste Division of the County Department of Public Works for the disposal of clearing and grubbing material from the site during construction.

Once completed, the proposed project will be served by a private refuse collection company. Solid waste generated from the project will be disposed at the County's Central Maui Landfill.

#### C. <u>INFRASTRUCTURE</u>

#### 1. <u>Traffic</u>

A traffic impact analysis report was done for the project. See Appendix B. The report analyzes traffic conditions at full build-out of MCC's master plan of 5,000 full-time equivalent students by the year 2000.

The projected increase in student enrollment is expected to generate 353 vehicles during the AM peak hour of traffic, 342 vehicles entering and 11 vehicles exiting. During the projected PM peak hour of traffic, the increase in student enrollment is expected to generate 264 vehicles, 198 vehicles entering and 66 vehicles exiting.

The report notes that there are a number of proposed highway improvements in the Wailuku-Kahului area which provide alternative travel routes. This relieves much of the traffic congestion on Kaahumanu Avenue in the project vicinity. Roadway improvements include:

The extension of Mahalani Street to Waiale Road.

- The construction of Maui Lani Parkway, a new roadway between Kaahumanu Avenue and Kuihelani Highway.
- The extension of Kamehameha Avenue through the new Maul Lani Parkway.
- The extension of Onehee Avenue to the new Maui Lani Parkway.
- The extension and widening of Waiale Road to Honoapiilani Highway.
- Roadway widening of Waiale Road between the Kahekili extension and the Mahalani Street extension.
- The Puunene Bypass Road between Mokulele Highway and Kuihelani Highway, connecting to the Maui Lari Parkway.

The highway capacity analysis performed for the study is based upon procedures presented in the "Highway Capacity Manual", Special Report 209, Transportation Research Board, 1985, and the "Highway Capacity Software", Federal Highway Administration.

Level of Service (LOS) is a quantitative and qualitative assessment on traffic operations. Levels of Service are defined by LOS "A" through "F"; LOS "A" being the best operating condition and LOS "F" the worst operating condition.

The total number of vehicles entering the intersection of Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue would increase by approximately 4% over projected traffic demands without the proposed MCC expansion during the AM peak hour. During the PM peak hour, the increase would be approximately 3% without the project.

At the intersection of Kaahumanu Avenue and South Papa Avenue, the north bound approach of South Papa Avenue would operate at LOS "D" during the AM peak hour with the proposed project. Under projected conditions without the proposed project, this approach would operate at LOS "C". The approach would continue to operate at LOS "C" during the PM peak hour. The other traffic movements at this intersection would operate at satisfactory LOS during the AM and PM peak hours with the proposed project.

Vehicular traffic entering the MCC main entrance at the intersection of Kaahumanu Avenue and Wakea Avenue is expected to increase by 105 vehicles during the projected AM peak hour of traffic with the proposed project. During the PM peak hour, entering traffic would increase by 45 vehicles. Despite the increase in traffic entering MCC, the traffic movements at this intersection would operate at satisfactory LOS during the projected AM and PM peak hours.

S S

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The total number of vehicles entering the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street is expected to increase by approximately 3% over projected conditions without the proposed project. During the PM peak hour, the increase is expected to be 2%.

Based on the analysis, the traffic impact analysis report recommends the following improvements:

## A. Improvements to Mitigate Existing Roadway Deficiencies

 Provide exclusive left-turn, through, and right-turn lanes on the north bound Mahalani Street approach at Kaahumanu Avenue. 2. Construct an exclusive left-turn lane, an optional left/through lane, and an exclusive right-turn lane on the north bound approach of Wakea Avenue at Kaahumanu Avenue.

#### B. Site Access Recommendations

- Restrict access from Kaihee Place and utilize the new road connecting Kanaloa Avenue to Kahului Beach Road as the access to the faculty/student parking lot located north of the campus.
- 2. Provide an exclusive right-turn lane and an optional left/through lane on the south bound approach exiting the MCC campus at the intersection of Kaahumanu Avenue and South Papa Avenue
- Provide an exclusive right-turn lane and an optional left/through lane on the south bound approach exiting the MCC campus at the intersection of Kaahumanu Avenue and Wakea Avenue.
- 4. Each of the campus parking lot driveways on the South Papa Avenue Extension should have exclusive left-turn and right-turn lanes for vehicles exiting the parking areas.
- 5. Left-turn lanes to the parking lot driveways should be constructed to maintain through traffic flow on the South Papa Avenue Extension.

With the implementation of roadway improvements and the construction of new roads providing alternative travel routes in the Wailuku-Kahului region, the proposed Maui Community College expansion should not have any significant impact on traffic in the vicinity of the project.

It is noted that traffic impacts and mitigation recommendations are based on 5,000 full-time students projected for the year 2000. Accordingly, the timing of improvements implementation and the prorata distribution of costs associated with the improvements will be coordinated with the State of Hawaii, Department of Transportation.

#### 2. Wastewater System

The design capacity of the County's Kahului Wastewater Treatment Facility is 6.0 million gallons per day (MGD). The facility serves the Kahului, Wailuku, Paia, Kuau and Spreckelsville areas. Current wastewater flows treated by the Kahului facility is approximately 5.2 MGD, excluding groundwater and stormwater infiltration.

It should be noted that improvements are being undertaken at the Kahului Wastewater Treatment Facility to increase treatment capacity to 7.9 MGD. The additional capacity is scheduled to be available in 1993.

The proposed project is estimated to generate an average daily flow of approximately 18,300 gallons per day of wastewater. An allocation of capacity will be coordinated with the Department of Public Works as part of the building permit process.

#### 3. Water System

Water will be furnished by the domestic system servicing the area. The average daily demand for the project is estimated to be about 49,700 gallons per day.

### 4. Drainage and Erosion Control

Drainage for the projects would involve two underground collection systems. Both systems will consist of inlets, catch basins, and underground piping which would discharge into the proposed retention basin.

The retention basin would consist of a four feet deep pond with two feet of freeboard, measuring approximately 140 feet by 900 feet in area. The basin is proposed to retain the difference in runoff volume between the existing on-site runoff and the on-site runoff generated by the proposed improvements. Existing on-site runoff for the MCC campus for a 10-year storm is approximately 91 cubic feet per second (CFS). With the proposed project, total run-off for the MCC campus is calculated at 146 CFS.

Off-site runoff, calculated at 26 CFS, will continue to pass through the campus to the retention basin.

Controlled flows from the retention basin would then be directed to Kahului Harbor. The ultimate discharge flow into Kahului Harbor will be limited to not exceed existing discharge levels.

Erosion control measures will be incorporated during the construction period to minimize soil loss. Dust will be minimized during construction by the implementation of water sprinkling.

All improvements will conform to County standards and be coordinated with the County of Maui, Department of Public Works. With installation of the proposed drainage improvements and implementation of erosion control measures, no adverse effects to adjacent and downstream properties are anticipated. See Appendix C.

#### 5. Electrical and Telephone Systems

Electrical power requirements associated with the proposed project will be supplied by Maui Electric Company, Ltd.

GTE Hawaiian Telephone Company (HTCO) maintains overhead telephone lines which serve MCC. Additional telephone system requirements generated by the project will be met by HTCO.

## Chapter IV

Relationship to Governmental Plans, Policies and Controls

## IV. RELATIONSHIP TO GOVERNMENTAL PLANS, POLICIES AND CONTROLS

#### A. STATE LAND USE DISTRICTS

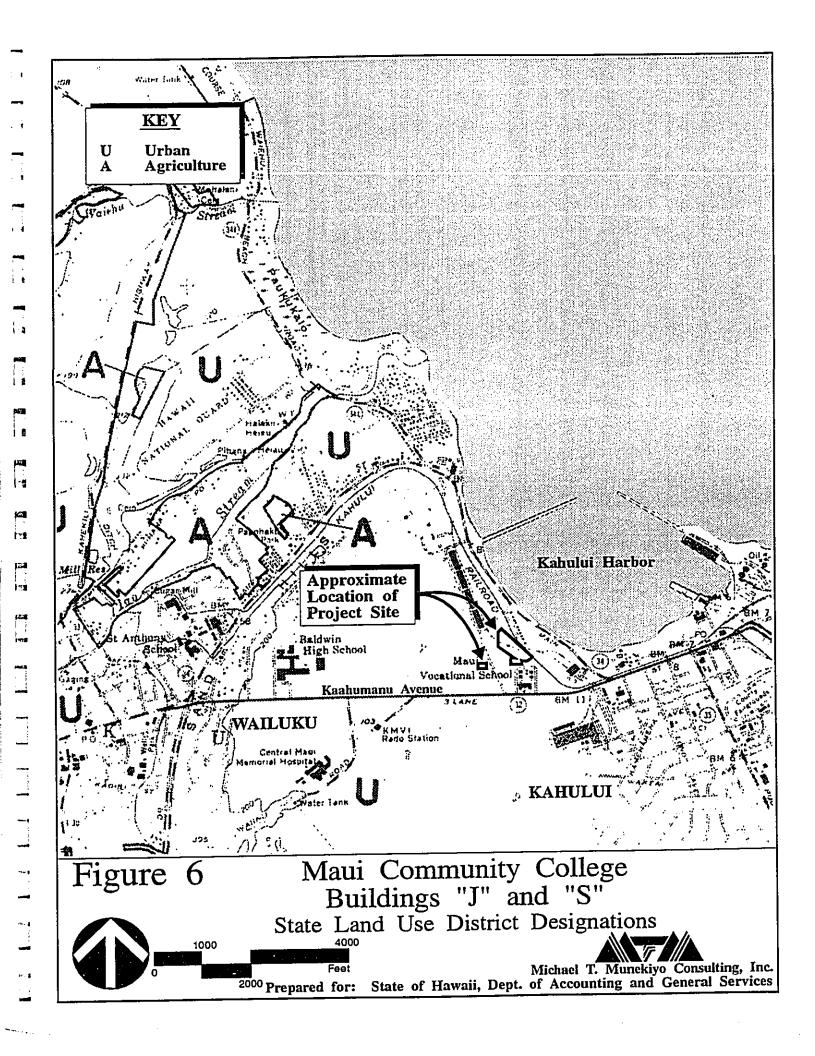
Chapter 205, Hawaii Revised Statutes, relating to the Land Use Commission, establishes the four major land use districts in which all lands in the State are placed. These districts are designated "Urban", "Rural", "Agricultural", and "Conservation". The subject parcel is within the "Urban" district. See Figure 6. The proposed action involves the use of the property for two new classroom buildings with attendant parking and drainage improvements. The proposed use of the property is consistent with "Urban" district provisions.

#### B. MAUI COUNTY GENERAL PLAN

The Maui County General Plan (1990 Update) sets forth broad objectives and policies to help guide the long-range development of the County. As stated in the Maui County Charter, "The purpose of the General Plan is to recognize and state the major problems and opportunities concerning the needs and the development of the County and the social, economic and environmental effects of such development and set forth the desired sequence, patterns and characteristics of future development".

The proposed action is in keeping with the General Plan's objectives:

- To see that all developments are well designed and are in harmony with their surroundings.
- 2. To provide Maui residents with continually improving quality educational opportunities which can help them better understand themselves and their surroundings and help them realize their ambitions.
- 3. Improve the delivery of services by government agencies to all community plan areas.



### C. WAILUKU-KAHULUI COMMUNITY PLAN

The subject parcel is located in the Wailuku-Kahului Community Plan region which is one of nine Community Plan regions established in the County of Maui. Planning for each region is guided by the respective Community Plans, which are designed to implement the Maui County General Plan. Each Community Plan contains recommendations and standards which guide the sequencing, patterns and characteristics of future development in the region.

Land use guidelines are set forth by the Walluku-Kahului Community Plan Land Use Map. See Figure 7. The subject parcel is designated "Public/Quasi-Public" by the Community Plan.

The proposed project is consistent with the Wailuku-Kahului Community Plan.

## D. SPECIAL MANAGEMENT AREA OBJECTIVES AND POLICIES

Pursuant to Chapter 205A, Hawaii Revised Statutes, and the Rules and Regulations of the Planning Commission of the County of Maui, projects located within the SMA are evaluated with respect to SMA objectives, policies and guidelines. This section addresses the project's relationship to applicable coastal zone management considerations, as set forth in Chapter 205A and the Rules and Regulations of the Planning Commission.

#### 1. Recreational Resources

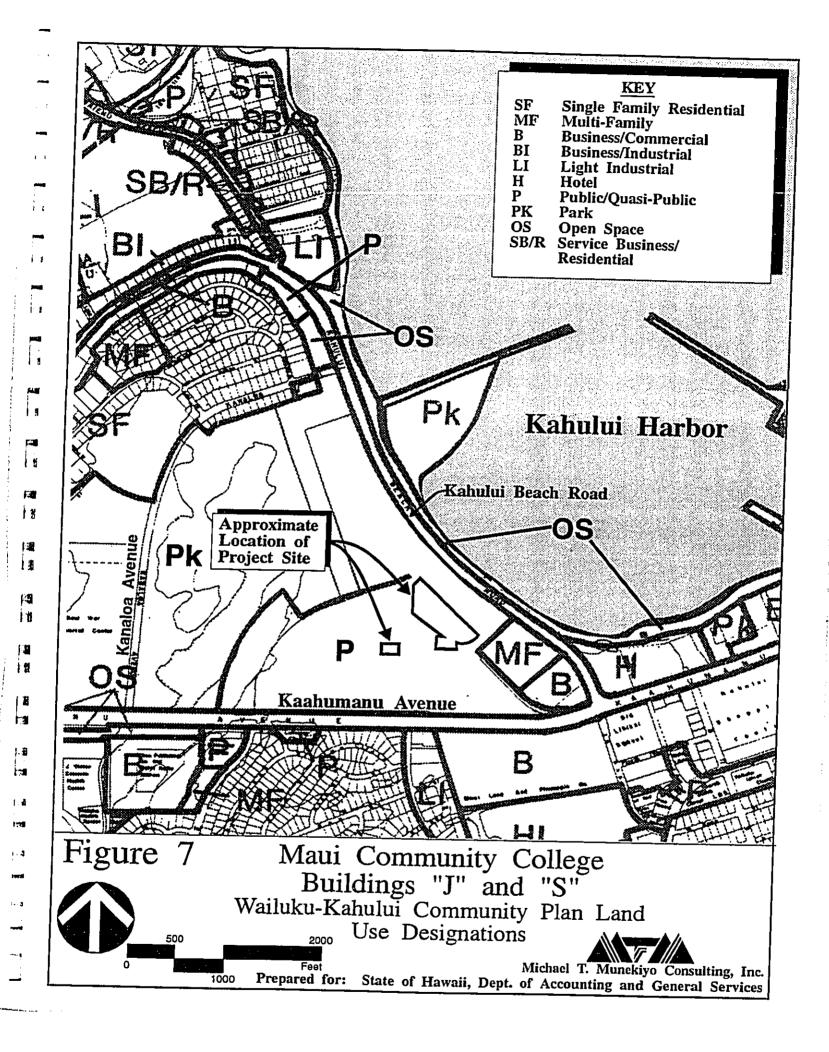
**Objective:** Provide coastal recreational resources accessible to the public.

#### Policies:

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 Improve coordination and funding of coastal recreation planning and management; and



- 2. Provide adequate, accessible and diverse recreational opportunities in the coastal zone management area by:
  - a. Protecting coastal resources uniquely suited for recreation activities that cannot be provided in other areas,
  - b. Requiring replacement of coastal resources having significant recreational value, including but not limited to surfing sites and sandy beaches, when such resources will be unavoidably damaged by development; or requiring reasonable monetary compensation to the State for recreation when replacement is not feasible or desirable;
  - Providing and managing adequate public access, consistent with conservation of natural resources, to and along shorelines with recreational value;
  - d. Providing an adequate supply of shoreline parks and other recreational facilities suitable for public recreation:
  - Encouraging expanding public recreational use of county, state, and federally owned or controlled shoreline lands and waters having recreational value;
  - f. Adopting water quality standards and regulating point and non-point sources of pollution to protect and where feasible, restore the recreational value of coastal waters; and
  - g. Encouraging reasonable dedication of shoreline areas with recreational value for public use as part of discretionary approvals or permits, and crediting such dedication against the requirements of Section 46-6 of the Hawaii Revised Statutes.

#### Response:

The proposed project will not affect coastal zone recreational opportunities. Accessibility to shoreline areas will not be impacted by the proposed action.

#### 2. Historical/Cultural Resources

<u>Objective:</u> Protect, preserve and where desirable, restore those natural and man-made historic and prehistoric resources in the coastal zone

management areas that are significant in Hawaiian and American history and culture.

#### **Policies:**

- 1. Identify and analyze significant archaeological resources;
- 2. Maximize information retention through preservation of remains and artifacts or salvage operations; and
- 3. Support state goals for protection, restoration, interpretation and display of historic resources.

#### Response:

The sites for Buildings "J" and "S" have already been extensively altered. An archaeological inventory survey for the area of the parking lot extension and retention basin did not find any pre-contact Hawaiian artifacts. Military camp remains from World War II have been reasonably well documented. The project is not anticipated to adversely affect significant historic or archaeological resources.

#### 3. Scenic and Open Space Resources

**Objective:** Protect, preserve and where desirable, restore or improve the quality of coastal scenic and open space resources.

#### Policies:

- Identify valued scenic resources in the coastal zone management area;
- 2. Insure that new developments are compatible with their visual environment by designing and locating such developments to minimize the alteration of natural land forms and existing public views to and along the shoreline;

- Preserve, maintain and, where desirable, improve and restore shoreline open space and scenic resources; and
- 4. Encourage those developments which are not coastal dependent to locate in inland areas.

#### Response:

The proposed project will not adversely impact scenic or open space resources. The proposed project will not involve significant alteration to the existing topographic character of the site and will not significantly affect public views to the shoreline.

#### 4. Coastal Ecosystems

#### Objective:

Protect valuable coastal ecosystems from disruption and minimize adverse impacts on all coastal ecosystems.

#### Policies:

- Improve the technical basis for natural resource management;
- Preserve valuable coastal ecosystems of significant biological or economic importance;
- Minimize disruption or degradation of coastal water ecosystems by effective regulation of stream diversions, channelization, and similar land and water uses, recognizing competing water needs; and
- Promote water quantity and quality planning and management practices which reflect the tolerance of fresh water and marine ecosystems and prohibit land and water uses which violate state water quality standards.

#### Response:

The completion of the proposed project will not significantly disrupt or impact coastal ecosystems. Appropriate soil erosion mitigation measures will be implemented during the construction of the project to minimize disruption of coastal water ecosystems. The proposed

retention basin also serves to collect and minimize runoff and siltation.

#### 5. Economic Uses

<u>Objective</u>: Provide public or private facilities and improvements important to the State's economy in suitable locations.

#### Policies:

- 1. Concentrate in appropriate areas the location of coastal dependent development necessary to the state's economy;
- 2. Insure that coastal dependent development such as harbors and ports, visitor facilities, and energy-generating facilities are located, designed, and constructed to minimize adverse social, visual and environmental impacts in the coastal zone management area; and
- 3. Direct the location and expansion of coastal dependent developments to areas presently designated and used for such developments and permit reasonable long-term growth at such areas, and permit coastal dependent development outside of presently designated areas when:
  - a. Utilization of presently designated locations is not feasible,
  - b. Adverse environmental effects are minimized, and
  - c. Important to the state's economy.

#### Response:

The proposed project is designed to accommodate an increase in student enrollment numbers. This growth will in turn increase the demand for additional instructors at MCC, which in turn will benefit the local employment market. Graduating students also provide an educated and trained workforce which are important to fulfill the demands of the marketplace and to the State's economy. This project will not generate any adverse economic impacts.

#### 6. Coastal Hazards

**Objective:** Reduce hazard to life and property from tsunami, storm waves, stream flooding, erosion and subsidence.

#### Policies:

- Develop and communicate adequate information on storm wave, tsunami, flood, erosion and subsidence hazard;
- 2. Control development in areas subject to storm wave, tsunami, flood, erosion and subsidence hazard;
- 3. Ensure that developments comply with requirements of the Federal Flood Insurance Program; and
- Prevent coastal flooding from inland projects.

#### Response:

Buildings "J" and "S" are located in an area of minimal flooding. However, portions of the parking lot expansion and retention basin are located in Zone A4 which has a base flood elevation of 17 feet above mean sea level. Makai portions of the retention basin are located in Zone V23, which includes areas inundated by the 100-year coastal flood with velocity hazards and a base flood elevation of 17-18 feet above mean sea level. Storm runoff should be addressed through on-campus drainage improvements and the construction of the retention basin. This would control the discharge rate to not exceed existing discharge levels. The project is not anticipated to adversely impact downstream or adjacent properties.

#### 7. Managing Development

**Objective:** Improve the development review process, communication, and public participation in the management of coastal resources and hazard.

#### **Policies:**

- Effectively utilize and implement existing law to the maximum extent possible in managing present and future coastal zone development;
- 2. Facilitate timely processing of application for development permits and resolve overlapping of conflicting permit requirements; and
- 3. Communicate the potential short and long-term impacts of proposed significant coastal developments early in their life-cycle and in terms understandable to the general public to facilitate public participation in the planning and review process.

#### Response:

Early consultation is provided through the process of preparing the Environmental Assessment. Public comments are also afforded during the review period of the Draft Environmental Assessment. The County's Special Management Area permitting process provides another avenue for review.

Applicable State and County requirements will be adhered to in the design and construction of the proposed project.

# Chapter V

Findings and Conclusion

### V. FINDINGS AND CONCLUSION

The proposed project would involve the construction of two new structures on the MCC campus, Buildings "J" and "S", as well as parking lot expansion and the construction of new retention basin.

Construction of the proposed project will involve short-term environmental impacts typically associated with construction activities. These include air quality and noise impacts. Dust control measures such as watering or sprinkling will be undertaken to minimize dust. Construction activities are also anticipated to be limited to daylight hours. Impacts generated from construction activities are not considered adverse.

From a long-term perspective, the proposed project is not anticipated to result in adverse environmental impacts. The sites for Buildings "J" and "S" have already been extensively modified. No significant archaeological or historic sites were found. There are no known rare/threatened species of flora and fauna at the project site.

Impacts to the local economy are anticipated to be positive involving support for construction-related employment in the short-term. After completion of construction, the project will provide liberal arts, vocational and technical training to benefit the local employment market.

With regard to traffic, there are a number of proposed new roadways in the region which provide alternative travel routes in the Wailuku-Kahului area. This relieves much of the traffic congestion on Kaahumanu Avenue in the project vicinity. In addition, there are a number of intersection improvements and site access recommendations designed to mitigate increases in traffic due to the project. With the implementation of these recommendations, the proposed project should not have any significant impact on traffic in the vicinity of the project.

With regard to drainage improvements, the proposed project involves the construction of a retention basin which collects runoff and controls discharge into Kahului Harbor. The ultimate discharge rate would not exceed existing discharge levels.

The project is also not anticipated to have adverse impacts upon medical, police and fire protection services as well as other infrastructure systems.

In light of the foregoing findings, it is concluded that the proposed action will not result in any significant impacts. Thus, this Environmental Assessment is being filed as a negative declaration.

## Chapter VI

Agencies Contacted in the Preparation of the Environmental Assessment and Responses Received

## VI. AGENCIES CONTACTED IN THE PREPARATION OF THE ENVIRONMENTAL ASSESSMENT AND RESPONSES RECEIVED

The following agencies were contacted during the preparation of the Environmental Assessment:

- U.S. Army Corps of Engineers Pacific Ocean Division Building 230 Fort Shafter, Hawaii 96858
- Mr. David Nakagawa, Chief Sanitarian Department of Health 54 High Street Wailuku, Hawaii 96793
- 3. Mr. Tom Arisumi, Division Chief
  Department of Health
  Environmental Management
  Division
  Five Waterfront Plaza, Suite 250
  500 Ala Moana Boulevard
  Honolulu, Hawaii 96813
- 4. Mr. Robert Siarot, Maui District
  Engineer
  Department of Transportation
  650 Palapala Drive
  Kahului, Hawaii 96732
- Department of Land and Natural Resources
   State Historic Preservation District
   1325 L. Main Street, #108
   Wailuku, Hawaii 96793
- 6. Mr. Brian Miskae, Director Department of Planning 250 South High Street Wailuku, Hawaii 96793

- 7. Mr. David Craddick, Director Department of Water Supply 200 South High Street Wailuku, Hawaii 96793
- 8. Mr. Lloyd Lee
  Department of Public Works
  Division of Engineering
  200 South High Street
  Wailuku, HI 96793
- 9. Mr. Aaron Shinmoto
  Chief Staff Engineer
  Department of Public Works
  200 South High Street
  Wailuku, Hawaii 96793
- Mr. Eassie Miller
   Department of Public Works
   Wastewater Reclamation Division
   200 South High Street
   Wailuku, Hawaii 96793
- 11. Mr. James Lawrence
  Kahului Town Association
  P. O. Box 156
  Kahului, Hawaii 96732



### DEPARTMENT OF THE ARMY U. S. ARMY ENGINEER DISTRICT, HONOLULU FORT SHAFTER, HAWAII 96858-5440

October 28, 1992



Planning Division

Mr. Michael T. Munekiyo, AICP Michael T. Munekiyo Consulting, Inc. 2035 Main Street Wailuku, Maui, Hawaii 96793

Dear Mr. Munekiyo:

Thank you for the opportunity to review and comment on the Proposed Construction of Maui Community College Buildings "S" and "J" (TMK 3-8-7: 40, 125). The following comments are provided pursuant to Corps of Engineers authorities to disseminate flood hazard information under the Flood Control Act of 1960 and to issue Department of the Army (DA) permits under the Clean Water Act; the Rivers and Harbors Act of 1899; and the Marine Protection, Research and Sanctuaries Act.

- a. The project does not involve work in waters of the U.S.; therefore, a DA permit is not required.
- b. According to the enclosed Federal Emergency Management Agency's Flood Insurance Rate Map, panel 150003-0190-C, dated September 6, 1989, the proposed project site is located in Zone C (areas of minimal flooding). The remainder of the parcels are located in Zone A4 (areas inundated by the 100-year flood with a base flood elevation of 34 feet above mean sea level) and Zone V23 (areas inundated by the 100-year coastal flood with velocity hazards and a base flood elevation of 18 to 34 feet above mean sea

Sincerely,

Kisuk Cheung, P.E. Director of Engineering

Enclosure

KEY TO	MAP `
Flood Boundary	San San Zone B
ar Flood Boundary	
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ear Flood Boundary	ZONE B
Flood Boundary	
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od Elevation in Feet	(EL 987)
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KPLANATION OF ZO	ONE DESIGNATIONS
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4 (1-44 harred tactors 00	ood; base flood elevations and ot determined.
Areas of 100-year stare between one (1) are of inundation are show	hallow flooding where depths nd three (3) feet; average depths wn, but no flood hazard factors
are determined.  Areas of 100-year s	hallow flooding where depths and three (3) feet; base flood , but no flood hazard factors

NATIONAL FLOOD INSURANCE PROGRAM

## FIRM

FLOOD INSURANCE RATE MAP

MAUI COUNTY, HAWAII

PANEL 190 OF 400 (SEE MAP INDEX FOR PANELS NOT PRINTED)

> COMMUNITY-PANEL NUMBER 150003 0190 C

> > MAP REVISED: SEPTEMBER 6, 1989

Federal Emergency Management Agency

NOTES TO USER

Areas of minimal flooding. (No shading)

Areas of 100-year flood; base flood clevations and flood hazard factors determined.

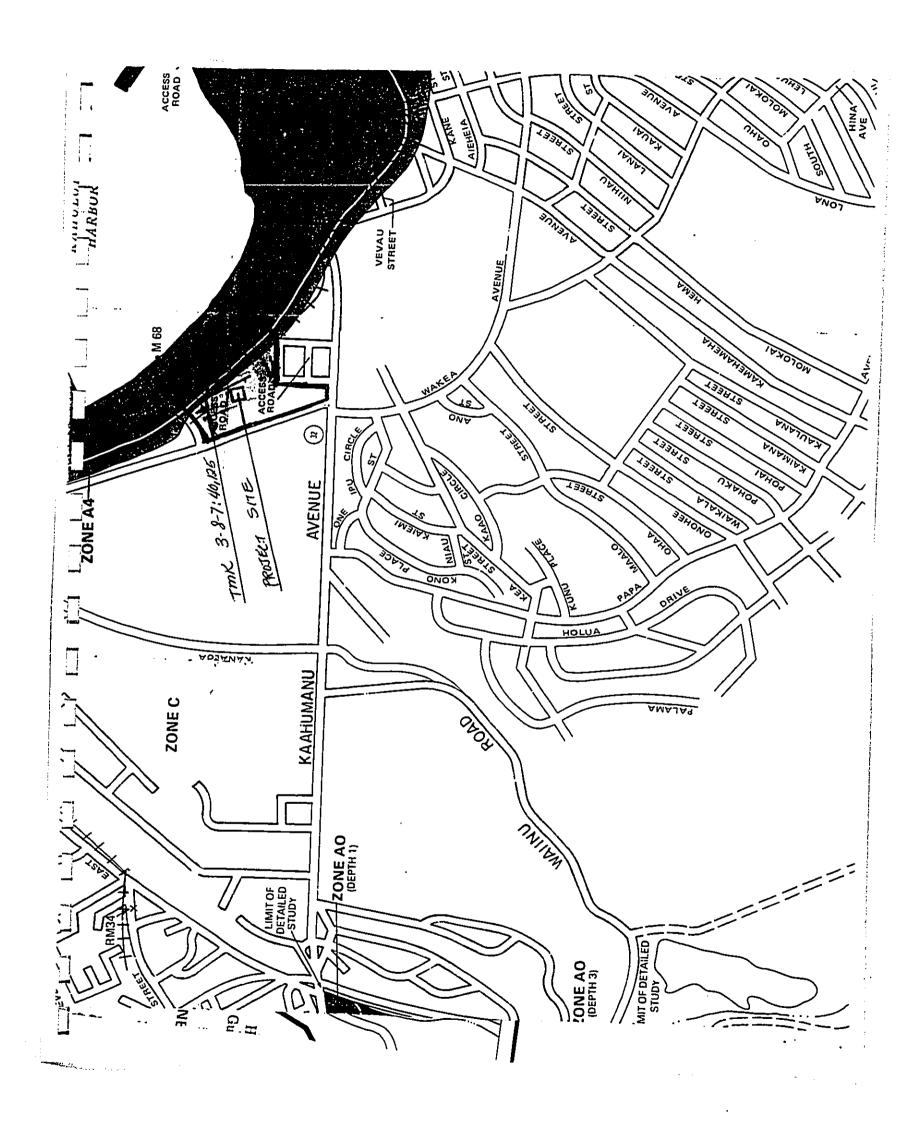
Areas of 100-year flood to be protected by flood protection system under construction; hase flood elevations and flood hazard factors not determined.

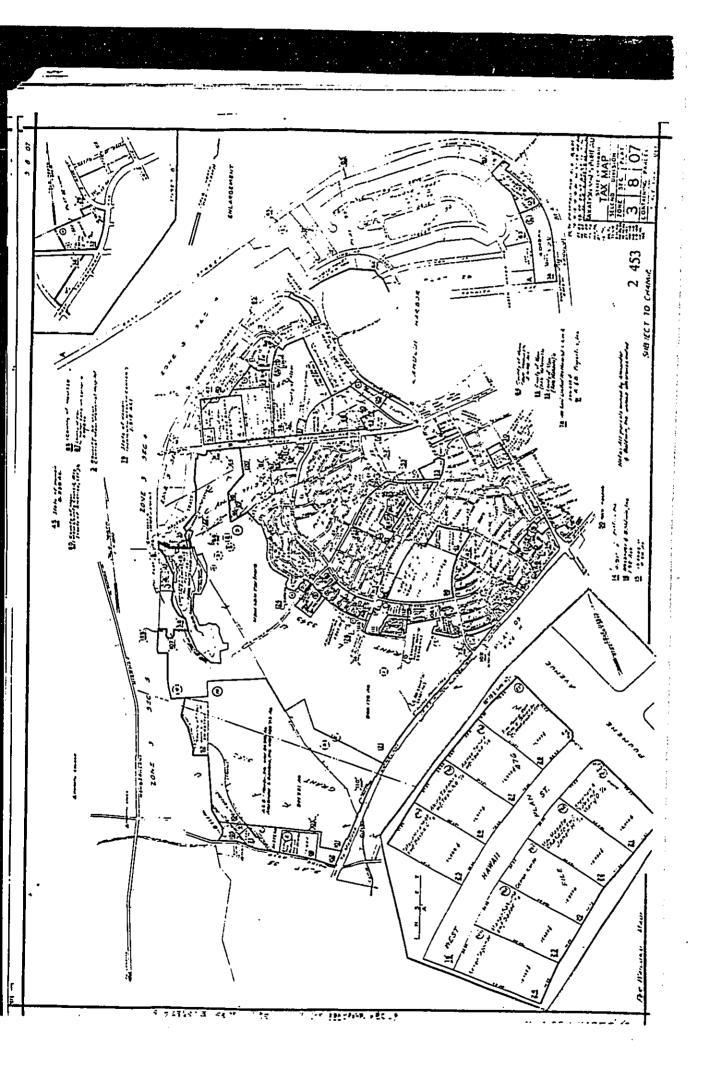
Areas between limits of the 100-year flood and 500-year flood; or certain areas subject to 100-year flooding with average depths less than one (1) foot or where the contributing drainage area is less than one square mile; or areas protected by levees from the base flood. (Medium shading)

Areas of undetermined, but possible, flood hazards. Areas of 100-year coastal flood with velocity (wave action); base flood elevations and flood hazard factors not determined.

Areas of 100-year coastal flood with velocity (wave action); base flood elevations and flood hazard factors determined.

are determined.





GOVERNOR OF HAWAII



JOHN C. LEWIN, M.D. DIRECTOR OF HEALTH

#### STATE OF HAWAII DEPARTMENT OF HEALTH

P. O. BOX 3378 HONOLULU, HAWAII 96801

in reply, please refer to:

November 5, 1992

92-383/epo

Mr. Michael T. Munekiyo, A.I.C.P. Michael T. Munekiyo Consulting, Inc. 2035 Main Street Wailuku, Hawaii 96793

Dear Mr. Munekiyo:

Subject: Pre-Environmental Assessment

New Classrooms - "S" and "J"

Maui Community College

Kahului, Maui TMK: 3-8-7: 125

Thank you for allowing us to review and comment on the subject project. We have no comments to offer at this time.

Due to preliminary plans being the sole source of discussion, we reserve the right to impose future environmental restrictions on the project at the time final plans are submitted to the Department of Health.

Very truly yours,

JOHN C. LEWIN, M.D.

Director of Health

Maui District Health Office (D. Nakagawa)

OCT 2 8 1293

JOHN WATHEE



#### STATE OF HAWAII

DEPARTMENT OF HEALTH
MAUI DISTRICT HEALTH OFFICE

54 HIGH STREET WAILUKU, MAUI, HAWAII 96793 PAUL E. HOFFMAN. M.D., M.P.H.
DISTRICT HEALTH SERVICES ADMINISTRATOR (M.D.)

JOHN C. LEWIN. M.D.

October 27, 1992

Mr. Michael T. Munekiyo, A.I.C.P. Michael T. Munekiyo Consulting, Inc. 2035 Main Street Wailuku, Hawaii 96793

Dear Mr. Munekiyo:

Subject:

Proposed Construction of MCC Buildings "S" and "J"

Thank you for allowing us to review and comment on the subject proposal. We have the following comments:

Dust control and noise nuisance during construction should be properly monitored especially when classes are in session.

Should you have any questions on the above, please call me at 243-5255.

Sincerely,

DAVID H. NAKAGAWA

Environmental Health Program Supervisor

JOHN WAIHEE GOVERNOR OF HAWAII



#### STATE OF HAWAII

#### DEPARTMENT OF LAND AND NATURAL RESOURCES

HAWAII HISTORIC PLACES REVIEW BOARD 33 SOUTH KING STREET, 6TH FLOOR HONOLULU, HAWAII 96813

October 26, 1992

OCT 2 8 1992

WILLIAM W. PATY, CHAIRPERSON BOARD OF LAND AND NATURAL RESOURCE

DEPUTIES

JOHN P. KEPPELER, II DONA L. HANAIKE

AQUACULTURE DEVELOPMENT PROGRAM

AQUATIC RESOURCES CONSERVATION AND

ENVIRONMENTAL AFFAIRS CONSERVATION AND
RESOURCES ENFORCEMENT

CONVEYANCES CONVEYANCES
FORESTRY AND WILDLIFE
HISTORIC PRESERVATION
DIVISION
LAND MANAGEMENT
STATE PARKS
WATER AND LAND DEVELOPMENT
LOG NO.: 6612

DOC NO.: 9210AG17

Mr. Michael T. Munekiyo 2035 Main Street Wailuku, Hawaii 96793

Dear Mr. Munekiyo:

SUBJECT: Historic Preservation Review of the Proposed

Construction of MCC Buildings "S" and "J"

Kahului, Wailuku, Maui

TMK: 3-8-07: 40

This is in reponse to your letter dated October 14, 1992, requesting our comments on Maui Community College's proposal to construct two classroom buildings, "S" and "J".

The location of the proposed Building "J" was previously inspected by our staff archaeologist and we sent you our comments in a letter dated February 12, 1992. We determined that the proposed building will have "no effect" on significant historic sites. No historic sites were identified and the ground appeared to have been previously disturbed by previous construction activities.

The location of the proposed Building "S" is currently a parking lot. This area has also been extensively modified. It is not likely that historic sites are still present. Therefore, the proposed classroom will have "no effect" on historic sites.

Should you have any questions, please contact Ms. Annie Griffin at 587-0013.

Sincerely

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DON HIBBARD, Administrator

State Historic Preservation Division

AARON SHINMOTO, P.E. Land Use and Codes Administration

EASSIE MILLER, P.E. Wastewater Reclamation Division

RALPH NAGAMINE, P.E.

Engineering Division

BRIAN HASHIRO, P.E. Solid Waste Division

MELVIN HIPOLITO Highways Division

LINDA CROCKETT LINGLE
Mayor

GEORGE N. KAYA
Director

CHARLES JENCKS
Deputy Director

LLOYD P.C.W. LEE, P.E.
Chief Staff Engineer



## COUNTY OF MAU! DEPARTMENT OF PUBLIC WORKS

200 SOUTH HIGH STREET WAILUKU, MAUI, HAWAII 96793

October 20, 1992

Michael T. Munekiyo Michael T. Munekiyo Consulting, Inc. 2035 Main Street Wailuku, Hawaii 96793

Dear Mr. Munekiyo:

#### SUBJECT: PROPOSED CONSTRUCTION OF MCC BUILDINGS "S" AND "J"

This letter is in response to your letter of October 14, 1992. We wish to thank you for the opportunity to comment on the proposed project. We offer the following comments at this time:

- 1. The developer should be informed that the Wastewater Reclamation Division cannot insure that wastewater system capacity will be available for the project.
- 2. The developer may be asked to provide discussion and calculations (sewer impact study) to substantiate that the existing wastewater system is adequate to serve this project.
- 3. Wastewater contribution calculations are required before building permit is issued.
- 4. The developer may be assessed impact fees for treatment plant expansion costs.
- 5. The developer is required to fund any necessary off-site improvements to collection system and wastewater pump stations.

At the present time, capacity appears to be adequate and there are no major obstacles that we are aware of. If you have any questions or wish to discuss this matter further, please call Dave Taylor at 243-7428.

Sincerely,

Eassie Miller, Chief

Wastewater Reclamation Division

DT:gmd (CT93031)





#### DEPARTMENT OF THE ARMY

#### U. S. ARMY ENGINEER DISTRICT, HONOLULU

BUILDING 230 FT. SHAFTER, HAWAII 96858-5440

December 3, 1992

Planning Division

Mr. Michael T. Munekiyo, AICP Michael T. Munekiyo Consulting, Inc. 2035 Main Street Wailuku, Maui, Hawaii 96793

Dear Mr. Munekiyo:

Thank you for the opportunity to review and comment on the Proposed Construction of Maui Community College Buildings "S" and "J" (TMK 3-8-7: 40, 125). The following comments are provided pursuant to Corps of Engineers authorities to disseminate flood hazard information under the Flood Control Act of 1960 and to issue Department of the Army (DA) permits under the Clean Water Act; the Rivers and Harbors Act of 1899; and the Marine Protection, Research and Sanctuaries Act.

- a. The project does not involve work in waters of the U.S.; therefore, a DA permit is not required.
- b. Please revise the flood hazard information to read:
  "According to the Federal Emergency Management Agency's Flood
  Insurance Rate Map, panel 150003-0190-C. dated September 6, 1989,
  the proposed project site is located in Zone C (areas of minimal
  flooding). The remainder of the parcels are located in Zone A4
  (areas inundated by the 100-year flood with a base flood
  elevation of 17 feet above mean sea level) and Zone V23 (areas
  inundated by the 100-year coastal flood with velocity hazards and
  a base flood elevation of 17 to 18 feet above mean sea level)."

Sincerely,

Kisuk Cheung, P.E. Director of Engineering

Enclosure

KEY TO MAP		
Flo	od Boundary	ZONE B
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ir ign:	ations	
<u>.</u>		
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FI	ood Boundary	
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bc	Elevation in Feet orm Within Zone**	/EF 2011
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O Areas of 100-year flood; base flood clevations and		
Areas of 100-year flood to be protected by flood protection system under construction; hase flood protections and flood hazard factors not determined.		
	Areas between limits year flood; or certain ing with average depth the contributing drain mile; or areas protects (Medium shading)	of the 100-year flood and 300- areas subject to 100-year flood s less than one (1) foot or where lage area is less than one square ed by levees from the base flood
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	<ul> <li>Areas of 100-year co action); base flood el determined.</li> </ul>	pastal flood with velocity (wavevations and flood hazard factor

NOTES TO USER .

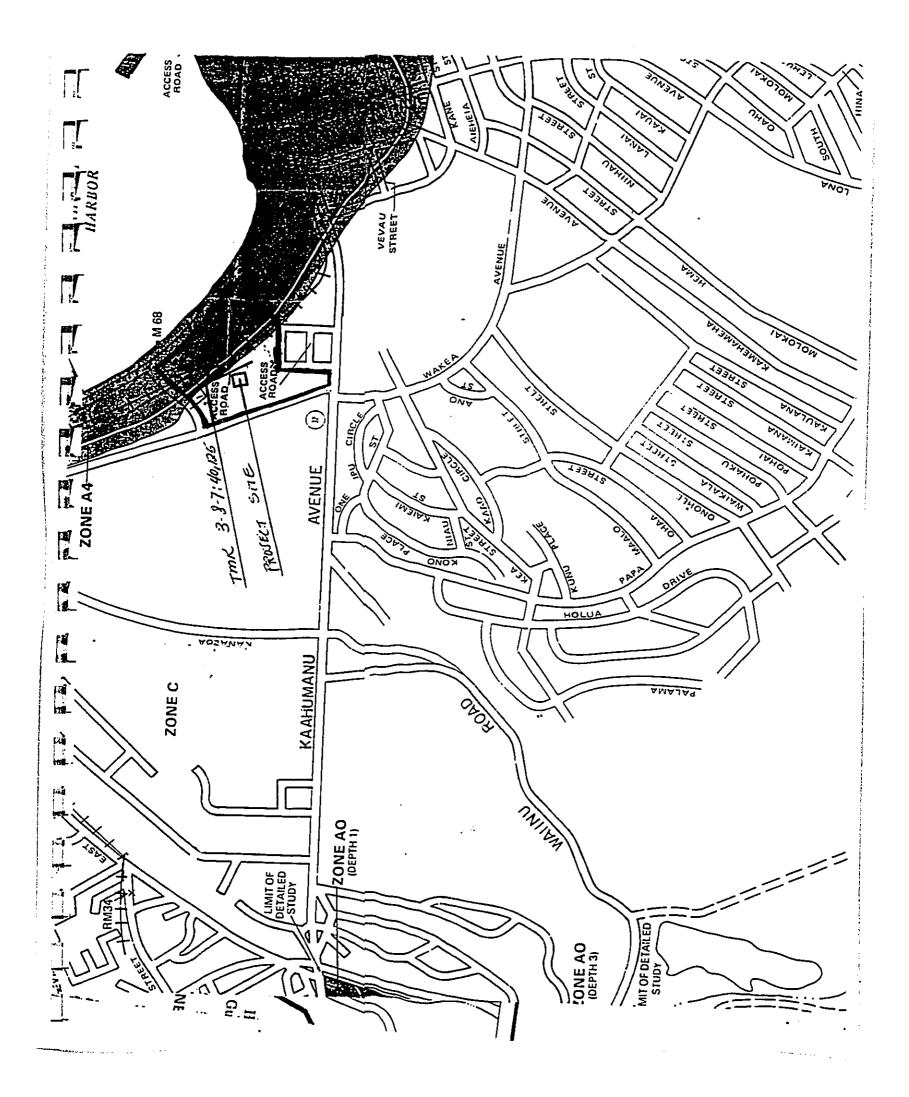
FIRM
FLOOD INSURANCE RATE MAP

MAUI COUNTY, HAWAII

PANEL 190 OF 400
(SEE MAP INDEX FOR PANELS NOT PRINTED)

COMMUNITY-PANEL NUMBER
150003 0190 C
MAP REVISED:
SEPTEMBER 6, 1989

Federal Emergency Management Agency



## Chapter VII

Letters Received During Public Comment Period and Agency Responses " GEORGE N NAYA
Director
CHARLES JENCKS
Deputy Director
LLOYD P.C.W. LEE, P.E.
Chief Staff Engineer



## COUNTY OF MAU! DEPARTMENT OF PUBLIC WORKS

200 SOUTH HIGH STREET WAILUKU, MAUI, HAWAII 96793

December 24, 1992

LARON SHINKOTO FE
Land Use and Code. Admir stration
EASSIE MILLER FE
Wastewater Reclamation Drusson
RALPH NAGAMINE. PE
Engineering Division
BRIAN HASHIRO P.E
Solid Waste Division

MELVIN HIPOLITO Highways Division

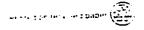
Mr. Milton Arakawa Michael T. Munekiyo Consulting Inc. 1823 Wells Street, Suite 3 Wailuku, HI 96793

Dear Mr. Arakawa:

SUBJECT: MAUI COMMUNITY COLLEGE BUILDINGS "S" and "J"

We have reviewed the above request and offer the following comments:

- That the architect and owner are advised that part of the project is subject to possible tsunami and flood inundation. As such, said project must conform to Ordinance No. 1145, pertaining to flood hazard districts.
- 2. That a road widening lot be provided for the extension of Papa Avenue to future Maui Central Park Roadway to provide for a 60 foot wide right-of-way.
- 3. That final detailed drainage and erosion control master plans including, but limited to, hydrologic and hydraulic calculations and scheme for controlling erosion and disposal of runoff water be submitted to the Department of Public Works, Engineering Division for our review and approval. The master plans shall provide verification that the grading and all runoff water generated by the project will not have an adverse effect on the adjacent and downstream properties. In addition, the developer shall contribute his pro-rata share to drainage improvements to be determined by the County and the drainage master plans and provide drainage easements as necessary in accordance to these drainage master plans. An agreement to the above prepared for filing with the State's Bureau of Conveyances shall be submitted by the applicant.
- 4. That the applicant shall prepare and submit a FY 2010 Traffic Master for the entire MCC site for review and approval by the County, Department of Public Works; the



Mr. Milton Arakawa December 24, 1992 Page 2

traffic master plan shall include the provision for the construction of the future Maui Central Park roadway and the extension of Papa Avenue from Kaahumanu Avenue to the new Central Park road, and address the construction of "onsite" and "offsite" traffic improvement recommendations.

- 5. That the applicant shall contribute his pro-rata share to traffic improvements to be determined by the County and traffic master plans. An agreement to the above prepared for filing with the State's Bureau of Conveyances shall be submitted by the developer.
- 6. That during the construction of this project, all construction employee parking shall be accommodated on the project site and not within the County road right-ofway.
- 7. Provide a water quality report including project mitigation measures (acceptable to the State Department of Health and Department of Public Works) which evaluates the quality of the storm water discharging into the ocean receiving waters. The report should include a discussion on sediment and nutrient loadings at all drainage outlets.
- That a SMA permit is required for this development.

Please contact Lloyd Lee at 243-7745 if you have any further questions.

Very truly yours,

JAN MIN LLOYD P.C.W LEE

Engineering Division Chief

cc: DIR

LUCA

Planning Department

LL: kb (ED92-725)

HN WAIHEE



ROBERT P. TAKUSHI COMPTROLLER

LLOYD I. UNEBASAMI DEPUTY COMPTROLLER

#### STATE OF HAWAII

### DEPARTMENT OF ACCOUNTING AND GENERAL SERVICES

P. O. BOX 119, HONOLULU, HAWAII 96810

LETTER NO. PM-1002.3

JAN 6 1993

Mr. Lloyd P.C.W. Lee Engineering Division Chief Department of Public Works County of Maui 200 South High Street Wailuku, Hawaii 96793

Dear Mr. Lee:

Subject: Maui Community College Buildings "J" and "S"

D.A.G.S. Job Nos. 15-31-4953 & 15-31-5871

Thank you for your letter of December 24, 1992, offering comments on the proposed project. We would like to provide a response to your comments as follows:

- We acknowledge that a portion of the project is subject to possible tsunami and flood inundation. Thus, we intend to conform with provisions of Ordinance No. 1145 pertaining to flood hazard districts.
- 2. Regarding the issues of Papa Avenue extension, completion of a traffic master plan to the year 2010, and contribution of a pro rata share of traffic improvements, it is noted that the extent of new building construction on the Maui Community College (MCC) campus and needed traffic improvements cannot be projected with a reasonable degree of accuracy at this time. Thus, the traffic report has been based on projections to the year 2000. It should be emphasized that as the MCC campus expands, future building plans and traffic improvements would be reviewed through the environmental review process. For Buildings "J" and "S", we acknowledge that an SMA permit is required and intend to

Mr. Lloyd P.C.W. Lee Letter No. PM-1002.3 Page 2

further discuss traffic concerns with you during this period.

- 3. Final detailed drainage and erosion control master plans will be submitted to the Department of Public Works for review. Our intent is to construct a project which has no adverse effect on adjacent and downstream properties, and will comply with all applicable ordinances and rules concerning this matter. We intend to further discuss the contribution of pro rata share to drainage improvements with you during the SMA permit process.
- Our intent is to provide construction employee parking on the MCC campus site rather than on County rights-of-way.
- Regarding water quality, necessary measures will be taken to comply with NPDES provisions, as required by law.

We appreciate the opportunity to respond to your comments. If you have any questions or desire further information, please contact Eric Nishimoto of our staff at 586-0468.

Very truly yours,

GORDON MATSUOKA

State Public Works Engineer

EN/si



## COUNTY OF MAUI PLANNING DEPARTMENT

RED D. HIGH STREET

WAILUKU, MAU!, HAWAII 96793

December 21, 1992

Milton Arakawa Mike T. Munekiyo Consulting, Inc. 1823 Wells Street, Suite 3 Wailuku, Hawaii 96793

Dear Mr. Arakawa:

RE:

Draft Environmental Assessment (EA) for Proposed New Buildings "S" and "J" at Maui Community College.

We have reviewed the draft EA and have no comments at this time.

A Special Management Area permit approval from the Maui Planning Commission is required. Please submit detailed site plans and elevations at time of filing.

Thank you for the opportunity to comment.

Very truly yours,

Brian Miskae, Director Department of Planning

:se

cc: C. Suyama

B. Medeiros

K. Fairbanks

JOHN WAIHEE GOVERNOR OF HAWAII



#### STATE OF HAWAII

#### **DEPARTMENT OF LAND AND NATURAL RESOURCES**

STATE HISTORIC PRESERVATION DIVISION 33 SOUTH KING STREET, 6TH FLOOR HONOLULU, HAWAII 86813

January 5, 1993

WILLIAM W. PATY, CHAIRPERSON BOARD OF LAND AND NATURAL RESOURCE

DEPUTIES

JOHN P. KEPPELER, II DONA L. HANAIKE

AQUACULTURE DEVELOPMENT PROGRAM

AQUATIC RESOURCES

CONSERVATION AND

**ENVIRONMENTAL AFFAIRS** CONSERVATION AND
RESOURCES ENFORCEMENT

CONVEYANCES

CONVEYANCES
FORESTRY AND WILDLIFE
HISTORIC PRESERVATION
DIVISION
LAND MANAGEMENT
STATE PARKS
WATER AND LAND DEVELOPMENT

LOG NO.: 6994 DOC NO.: 9212AG75

Mr. Milton Arakawa Michael T. Munekiyo Consulting, Inc. 1823 Wells St., Suite 3 Wailuku, Maui, Hawaii 96793

Dear Mr. Arakawa:

SUBJECT:

Historic Preservation Review of the Draft Environmental

Assessment for the Maui Community College Buildings "S" and "J"

Kahului, Wailuku, Maui

TMK: 3-8-07: 40

Thank you for the opportunity to comment on this document.

This EA indicates that no historic sites are present on the project area based on the findings of an archaeological survey and subsurface testing. Instead, evidence of previous land alteration activities was observed. We concur with evidence of previous land alteration activities was observed. We concur with the assessment that the proposed construction of these buildings will have "no effect" on historic sites. However, this determination is contingent upon the submission of a final report on the results of the survey. In general, we do not find preliminary reports acceptable for environmental documents. In this case, however, the findings are negative and there is sufficient information included in the report to enable us to review the adequacy of the study. The final report should be submitted to our office and used in the final EA.

Please contact Ms. Annie Griffin at 587-0013 if you have any questions about these comments.

DON HIBBARD, Administrator

State Historic Preservation Division

AG:aal

JOHN WAIHEE GOVERNOR



ROBERT P. TAKUSHI COMPTHOLLER

LLOYD I. UNEBASAMI DEPUTY COMPTROLLER

STATE OF HAWAII DEPARTMENT OF ACCOUNTING AND GENERAL SERVICES

P. O. BOX 119, HONDLULU, HAWAH 90810

LETTER NO. PM-1007. 3

8 1993 JAN

Mr. Don Hibbard Administrator State Historic Preservation Division Department of Land and Natural Resources 33 South King Street, 6th Floor Honolulu, Hawaii 96813

Dear Mr. Hibbard:

Subject: Maui Community College Buildings "J" and "S" D.A.G.S. Job Nos. 15-31-5871 and 15-31-3831

Thank you for your letter of January 5, 1993, pertaining to the Draft Environmental Assessment for the subject project.

The final archaeological inventory survey for the subject project has been submitted to your office and is included in the Final Environmental Assessment.

We appreciate the opportunity to respond to your comments. If you have any questions, please call Eric Nishimoto of our staff at 586-0468.

Very truly yours,

GORDON MATSUOKA

tate Public Works Engineer

EN/si

## References

#### **REFERENCES**

Community Resources, Inc. <u>Maui County Community Plan Update Program Socio-Economic Forecast Report</u>, March 1992.

County of Maui, The General Plan of the County of Maui, 1990 Update.

County of Maui, Wailuku-Kahului Community Plan, 1987.

State of Hawaii, Department of Business and Economic Development, <u>Data Book</u>, 1990.

Telephone Conversation with Steven Yamamoto, Corps of Engineers, November 23, 1992.

U.S. Department of Agriculture, Soil Conservation Service, <u>Soil Survey of Islands</u> of Kauai, <u>Oahu</u>, <u>Maui</u>, <u>Molokai and Lanai</u>, <u>State of Hawaii</u>, 1972.

University of Hawaii, Department of Geography, <u>Atlas of Hawaii</u>, Second Edition, 1983.

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# Appendices

## Appendix A

Archaeological Inventory Report

AN ARCHAEOLOGICAL INVENTORY
SURVEY FOR THE PARKING LOT EXPANSION
AND RETENTION BASIN ON MAUI
COMMUNITY COLLEGE CAMPUS
(TMK 3-8-07: 40 & 43),
AHUPUA'A OF WAILUKU, DISTRICT
OF WAILUKU, ISLAND OF MAUI

### Prepared for:

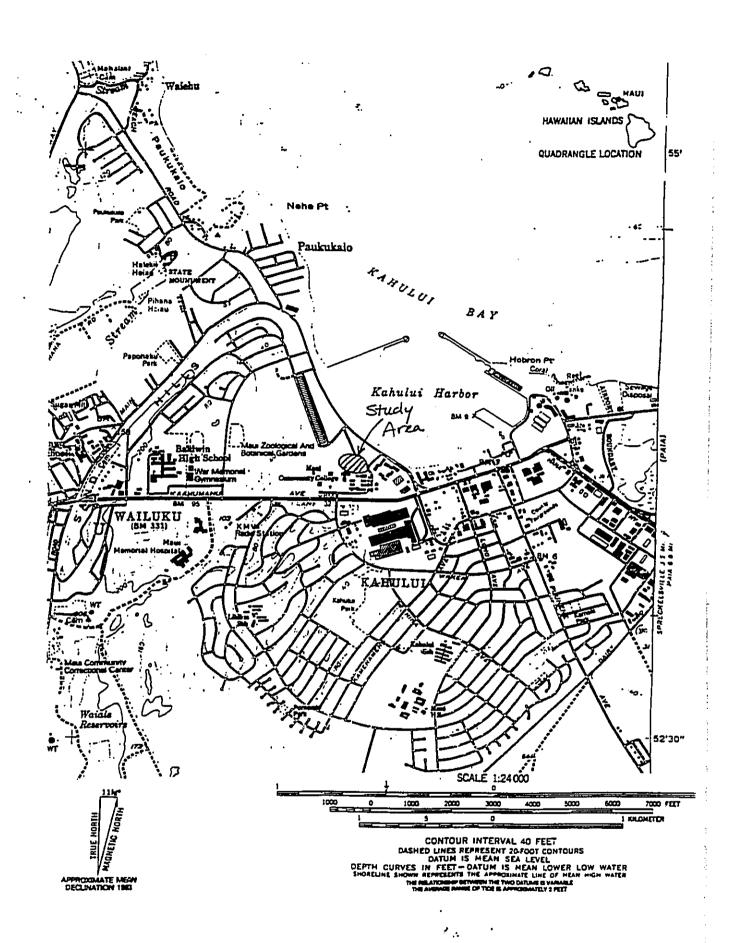
Gima, Yoshimori, Miyabara, Deguchi Inc. Wailuku, Hawaii

Prepared by:

Xamanek Researches P.O. Box 131 Pukalani, Hawaii, 96788

Walter M. Fredericksen Demaris L. Fredericksen

December 1992



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MAP 1 - Topograhic Map, U.S.G.S., Wailuku Quadrangle, Scale 1:24,000, 1983.

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PHOTO 1	-	View of early phase of construction of Maui Vocational School at the present Kaahumanu Avenue location of study parcel (1949).
DITORIO		

- Portion of a concrete floor which was part of the 18th Service and Supply Battalion, USMC.

- PHOTO 3 Stone and concrete planter, or flower bed, near the poured concrete floors, 18th Service and Supply Battalion, USMC.
- PHOTO 4 Row of coconut trees along the road into the USMC Base, taken in 1946 after the tsunami. (Maui Historical Society Archives)
  - PHOTO 5 Same line of trees, 1992.

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- PHOTO 6 View of the <u>makai</u> side of the study parcel showing damage done during the 1946 tsunami. (Maui Historical Society Archives)
- PHOTO 7 Quonset Huts which were part of the USMC Base, with house displaced by the 1946 tsunami. (Maui Historical Society Archives)
- PHOTO 8 Backhoe testing in dense vegetation on subject parcel

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#### INTRODUCTION

In early June, 1992, we were contacted by Gima, Yoshimori, Miyabara, Deguchi Inc. (GYA Inc.), regarding an archaeological inventory survey for a proposed State of Hawaii expansion of the University of Hawaii, Maui Community College Campus, at Kahului, Maui, Hawaii (D.A.G.S. Job No. 15-31-4953; GYA Project No. 91125.3). The proposed expansion plans include extension of the present, large automobile parking lot on the north side of the existing campus and construction of a large retention basin that will run along the north and east border of the extended parking included in the overall construction of Buildings "J" and "S", while part of archaeological research task. This was determined through of the proposed buildings slated for construction on the existing campus Parking Lot (Building "S"), and the other (Building "J") in area already disturbed and surveyed.

Following an initial assessment of the project area, we submitted a proposal for the completion of the necessary archaeological inventory survey work. Our proposal included on the findings of the survey. The proximity of a report parking lot and its attendant land disturbance during world war when the parcel was part of the United States Marine heavily disturbed nature of the site area.

Our proposal was submitted in June, 1992. It was accepted by October 6, 1992. Background research and fieldwork began in October and we submitted a preliminary report on findings of the survey November 4, 1992. Additional work in early/middle November was concluded, followed by preparation of the archaeological inventory survey report.

#### SURVEY AREA

The subject parcel comprises ca. 5 acres and is bordered on the north and east side by Kahului Beach Road and Kahului Harbor (Map 1). The south and east border is formed by the access road to the Harbor Lights Project and the west and north border lies adjacent to the Maui Community Arts and Cultural Center project. The harbor shoreline lies ca. 20 - 50 meters north of Kahului Beach Road. The land for the two projects is part of TMK: 3-8-07: 40 & 43 (Map 2), at "Owa and Kalua, Kahului, Wailuku, Maui, Hawaii", as stated on the topographic map prepared by Robert T. Tanaka, Civil Engineer, and is included in the present University of Hawaii, Maui Community College campus (Map 3). It is part of the former large ahupua'a of Wailuku.

#### NATURAL HISTORY

The entire University of Hawaii, Maui Community College campus is geologically part of the Kula series of lava flows. It lies in the interface area of dominant entisols made up of volcanic ash/beach sand derived soils and oxisols which are exceptionally stable lowland soils (University of Hawaii, 1983, pp. 39-41).

Puuone Sand (PZUE) soils are defined for sandhills near the ocean for Maui Island. A description includes the following-"In a representative profile the surface layer is grayish-brown, calcareous sand about 20 inches thick. This is underlain by grayish-brown, cemented sand. The soil is moderately alkaline in the surface....Permeability is rapid above the cemented layer. Runoff is slow, and the hazard of wind erosion is moderate to severe." (Foote, 1972, p. 117)

Soil description ranges from primarily aeolian sand and some sand lenses with color variations, through various mixes of landfill, including crushed rock, beach pebbles and coral chunks. During the survey, soil types were described using Foote (Ibid.) and the Munsell system. Some recent humus soil has formed in surface zones from vegetative decay, but no thick deposits were observed in the test trenches excavated. Root penetration of the loose, sandy soils is intense throughout the survey area, with depths up to 40 cm. below surface dominated by a matrix of intermeshed roots of grasses and woody plants.

Considerable disturbance from previous land use of the parcel has obliterated any previous surface features that might have been present, with the exception of poured concrete

foundations associated with World War II military use. Large coastal dunes lie to the west and north of the subject parcel. Presently, low elevations with averages between 6 and 12 above mean sea level feet are typical. Portions of the area appear to have been part of a lowland swamp wetland. Evidences of earth-fill activities were discovered during the archaeological survey. It was also noted that the portion of the parcel bordering Harbor Lights Complex is still a wet swamp-like depression. The maximum elevation of the area at the extreme western border increases to ca. 24 feet.

Elevations range from 6 to 14 feet above mean sea level, with ground water apparent from ca. 1.5 to over 3 meters below surface in most areas depending on the elevation. At the western border elevations increase to over 24 feet on the knoll along the bordering edge, although the knoll is off the property. Near the southeast border to the Harbor Lights Complex, elevations are uneven, alternating from 6 to 8 to 6 to 10 feet, creating a "washboard" effect from southwest to northeast, with Kaihee Place at an elevated 12 feet. This part of the parcel is presently quite swamp-like. Kahului Beach Road also has an elevation of ca. 12 feet. Land between the subject parcel and Kahului Beach Road (both part of Lot 1A, same TMK) exhibits an elevation on average of ca. 6-7 feet with a berm to 12 feet at the mauka Kahului Beach Road border. An additional berm on the mauka side of the low area to ca. 8-9 feet above sea level. This low-lying land serves as a catchment for storm runoff, with four pipe-culverts passing under Kahului Beach Road and the beach into Kahului Harbor for drainage.

The parcel is covered with heavy undergrowth of various soft fiber and woody plants. The most abundant and notable large plants include koa haole (Leucaena leucocephala), Kiawe (Prosopis pallida), Castor Bean (Ricinus communis), Tree Heliotrope (Messerschmidia argentea), and coconut palm (Cocos nucifera), which are found planted in rows bordering a former road on the Marine Base (Photos 4 & 5). Smaller species forming the understory include herbacious plants (Spinifera sp.), some Buffle grass (Cenchrus ciliarus), and other grasses, lantana (Lantana camara) and morning glory (Ipomoea indica). Other exotic, less noticable species are also present. No significant faunal remains were observed during the survey.

#### BACKGROUND HISTORICAL RESEARCH

The subject parcel is part of the large ahupua'a of Wailuku, which included land from Wailuku to Paia, and extending nearly halfway across the isthmus. This ahupua'a is located in,

foundations associated with World War II military use. Large coastal dunes lie to the west and north of the subject parcel. Presently, low elevations with averages between 6 and 12 above mean sea level feet are typical. Portions of the area appear to have been part of a lowland swamp wetland. Evidences of earth-fill activities were discovered during the archaeological survey. It was also noted that the portion of the parcel bordering Harbor Lights Complex is still a wet swamp-like depression. The maximum elevation of the area at the extreme western border increases to ca. 24 feet.

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#### BACKGROUND HISTORICAL RESEARCH

The subject parcel is part of the large ahupua'a of Wailuku, which included land from Wailuku to Paia, and extending nearly halfway across the isthmus. This ahupua'a is located in,

and encompasses roughly half the land area of the Wailuku District. A detailed summary of pre- and post-European contact history for this ahupua'a is presented in two recent reports on land parcels nearby the present, subject parcel (Kennedy, et. al., September 1992a, and September 1992b). The subject parcel is located in the eastern half of the ahupua'a, an area apparently little used for agriculture or habitation until late in the 19th century, when some of it was put into sugarcane production.

Descriptions from documents, and information on historic maps lead numbers of authors to conclude that the eastern portion of the ahupua'a was little used in pre-contact times, because of it's relatively barren landscape (Kennedy, 1992a, pp. 11-14). Referring to land use in the mid-eighteenth century, Kennedy states, "The only mention of habitation sites in the eastern portion that could be found is that referring to the fishermen's huts fronting Kahului Bay." (Ibid., p. 8). The population of the ahupua'a of Wailuku was listed as being 2,256 in 1831-32 (Cordy, 1978, p. 59).

After the Great Mehele in 1848 it was declared to be Crown Land, set aside to support the "royal state and dignity". After the death of the last Kamehameha (Lot, or Kamehameha V) in 1872, the land went to his sole heir, his sister Ruth. To settle debts, Princess Ruth Ke'elikolani sold one-half interest in the Crown Lands of Hawaii to Claus Spreckels in 1882. He had already leased 16,000 acres in the eastern portion of the ahupua'a in 1878. Land Grant 3343 from King Kalakaua in 1882 gave Spreckels the 24,000 acre ahupua'a of Wailuku in return for the surrender of his claim to one-half the Crown Lands of Hawaii (Adler, 1966, pp. 262-64).

By 1899, Alexander & Baldwin controlled the <u>ahupua'a</u> and it was utilized for pasturage and some agriculture. The Kahului Railroad Company acquired portions of the land for their transportation network in the 1870's, and the railroad track-bed was built, traversing the subject parcel. The railroad remained in service until the mid-1960's, when it was dismantled (Fredericksen, September 1988, p. 8). Map 5 shows the route of the Kahului Railroad System in 1955.

#### Oral History

Information on land use of the survey parcel during World War II was provided by Mr. Jack Crouse during an oral history interview in December, 1992. He was a U.S. Marine who came to Maui in 1944, and was stationed with the 18th

Service Battalion. Mr. Crouse, a local businessman, has lived on Maui since that time. He continues his involvement with the U.S. Marines, and is active in a number of their organizations. Information relevant to land use of the survey parcel is presented below.

During the Second World War there were a number of U.S. Military camps located on Maui. Two U.S. Navy Airwings were centered around Kahului and Pu'unene, each with their own landing field. These airwings trained pilots for air combat. The Fourth Division, U.S.M.C. was stationed at Camp Maui, and the camp served as a staging and training area for marines heading overseas in the Pacific Theater (See, Proehl, et al., 1946, for a detailed history of the Fourth Division Marines during World War II). NOTE: Other reference is made to Camp Maui as a rest and recreation camp for military personnel serving in the Pacific.

In 1944, the 18th Service Battalion (U.S.M.C.) was attached to the Fourth Marine Division at Camp Maui, and their camp was built and located between the present University of Hawaii, Maui Community College campus and Kanaloa Road, which is about .5 kilometer to the north. Remains of various poured, reinforced concrete floors and foundations of buildings and structures which were part of the 18th Service Battalion military camp complex are present on the survey parcel (Photos 2 and 3; Figure 8). The mission of the 18th was to provide supplies for the 4th Marines and to be responsible for the dispersal and disposal of military equipment and supplies on Maui following the conclusion of the war. Surplus material was shipped to the mainland and elsewhere.

NOTE: According to a Maui News article dated July 24, 1948, the former Marine camp of the 18th Service and Supply, was turned over to the new owner, the Kahului Railroad Company.

A series of Quonset Huts were built along Kahului Beach Road by the U.S. Navy "SeaBees", as part of the storage/office space for the camp (Photo 7 shows 1946 tsunami damage). They were fronted on the makai side by the railroad. After the camp mission was concluded in 1947, the Quonset Hut complex continued to be used by the Kahului Railroad Company until it terminated service in 1965. Private commercial businesses utilized them until their destruction and removal during the 1980's.

Mr. Crouse was the military Aide to the camp commander, Lt. Colonel Park. Colonel Park lived in one of a series of

homes on the beach where the present Maui Beach Hotel is located. The homes were originally built for plantation administrative personnel. The present Harbor Lights Complex sits on the site of a former plantation village or camp, housing workers and their families.

The coconut palm trees described earlier in this report lined one of the road entrances to the camp near the present Harbor Lights Project (Photos 4 and 5). A second camp entrance was located near Kanaloa Road. Professors Ernest Rezents and Bruce Palmer, University of Hawaii, Maui Community College, provided additional oral history on the existing palm-tree row. Apparently, the Kahului Railroad ran its track just to the north of the row of palms, drawing alongside the Quonset Huts complex for loading and offloading of materials. Professor Palmer recalls discovering railroad tie timbers in situ along the north border of the palms during several biology field trips in the late 1960's. "Raw Fish" Camp laid along the south border of the palm tree row and consisted of camp houses for plantation laborers and their families. Professor Rezents, a life-long resident of Maui, recalled "Raw Fish" camp as being an "old-style" plantation settlement in existence prior to World War II. He had no information regarding the significance of the camp name.

Landfill materials have been freely deposited on the parcel during the past. Some of these materials may have resulted from efforts to fill and raise the elevation of the barren lowlands reported in this area during the latter part of the 19th and early 20th centuries. An article in the Maui News, June 8, 1907, states: "The Kahului Railroad Company is filling in the lowlands in and about Kahului and will in time raise the level of the entire town site. When the work is completed and proper drains provided, the town should be free of mosquitos and the place a most desirable locality in which to live."

;

When the first harbor breakwater wall was built in 1904 the harbor was dredged and the dredge fill was used in filling the lowland areas of Kahului to make it more suitable for dwellings and commercial uses (Kennedy, 1992, p. 12). Jack Crouse (personal communication, 1992) witnessed Kahului Harbor dredged material being deposited on the survey parcel and other land in the areas around the harbor shortly after the war. Certainly, the 1946 Tsunami that devastated Hilo, Hawaii, and caused considerable damage to buildings and material at the Marine camp on the subject parcel contributed to land disturbance, as shown in photographs taken at the time (Photos 4, 6 and 7).

Following the war, the site was used for the new home of Maui Vocational School. This institution, originally located behind the Kahului School, was built in 1931 after the Territorial Legislature authorized \$20,000 for its construction in 1929, with legislation known as the Paschoal Bill. An article in the Maui News dated July 22, 1931, describes the new school:

"The building itself is an attractive one-storied structure finished with sand-colored walls, red roof, and a pale green trim in order to harmonized with the Kahului Elementary School close to which it has been erected.

It houses an automobile shop, a carpentry shop, a machine shop, an electric shop, one main classroom, and offices for the principal, store rooms and the like.

The three main wings are connected on the street side by arcade which also serves as a corrider between the main shop building and sinble classroom..."

Mr. Duncan Sinclair was initially the machine shop instructor, and became the school's Principal in 1941, serving in instructor, and became the school's Principal in 1941, serving in that capacity for 25 years until his retirement in 1966. He wrote a brief history of the Maui Vocational School following his retirement, from which portions of the following were taken. Upon taking over the job as principal, Mr. Sinclair recognized the need for expanding the existing programs and adding new ones. A dressmaking program was added. Having outgrown the existing campus, Sinclair approached Legislator Harold Rice, who negotiated with Frank Baldwin. Manager of Hawaiian Commercial and Sugar with Frank Baldwin, Manager of Hawaiian Commercial and Sugar Company. HC & S donated 11.5 acres on Kaahumanu Avenue "for the use of vocational education students who wanted to learn a trade, and also for those who could not afford to go to college." Three new shops were built at a cost of \$68,000 per building. These were the auto, carpentry and machine shops. In the next two years, an administrative/classroom complex was completed. By 1955, two small buildings were moved from NASKA to house the auto body shop and architecture/destricts specific house the auto body shop and architecture/drafting programs, and the expansion was completed. At this time 750 students were enrolled.

In 1958, the name was changed to Maui Technical School. In 1965 the University of Hawaii created the Community College System, and Maui Community College came into being. The addition of general education programs required the building of a "new, upper campus", to completment the "lower campus" buildings and vocational programs already in existence. The present expansion is a continuation of that process.

A Drive-in Movie Theater occupied much of the space presently used for the main MCC Parking Lot and part of the subject survey parcel. It was operational until the construction of Maui Community College campus began in 1967. No indicators of its former existence were discovered during the survey, although the theater and its concession buildings, viewing screen structure and land sculpting for automobile parking/viewing must have and land sculpting for automobile parking its construction. required considerable land disturbance during its construction. Landfill and leveling activities for the present MCC parking lot probably removed and/or covered any traces of this former complex, probably removed and/or covered foundations and floors for the including the poured concrete foundations and floors for the concession stand and projection building, as well as the viewing screen.

Present use of the study area includes several formerly bulldozed access roads, now heavily overgrown, and two, existing blacktop roadways in the area of the MCC campus Agricultural building and field complex. Notably in this area are numbers of heaped-up sand and construction detritus piles remaining from former unknown clearing or construction activities. Along the western border with the Maui Community Arts and Cultural Center, western border with the Maui Community Arts and Cultural Center, wall-like barrier up to 3 meters in height.

## BACKGROUND ARCHAEOLOGICAL RESEARCH

After perusing the available literature and consulting with the State Historic Preservation Division, we conclude no archaeological work has been done on the subject parcel. However, a number of surveys have been completed on parcels nearby. Archaeological Consultants of Hawaii (Kennedy, 1990), surveyed the site of the Maui Community Arts and Cultural Center, located on the property contiguous to the subject parcel. No archaeological data were recovered after extensive subsurface survey. Donham (1990) surveyed the Maui Palms site, which nearly borders the subject parcel to the north and east. Two potential historic sites were determined to be contained in introduced land fill, and thus not archaeological sites.

The authors conducted surveys along Kahului Beach Road and Lower Main Street at four different sites (Fredericksen, December, 1990; January, 1992; November, 1992, and December, 1992). Two of the survey areas are located along Lower Main Street about 2 the survey areas are located along Lower Main Street about 2 kilometers north and west of the subject parcel. Both of these kilometers north and west of the subject parcel. Both of these sites lie along the former Kahului Railroad track-bed. Nothing remains of it on one site (TMK 3-4-39: 77; Fredericksen, December, remains of it on one site (TMK 3-4-39: 77; Fredericksen, December, adjacent to Lower Main Street (TMK 3-4-39: 82; other, adjacent to Lower Main Street (TMK 3-4-39: 82;

Fredericksen, January, 1992). Neither of these sites produced any pre-contact archaeological materials.

The two most recent surveys, TMK 3-8-07: 123, the Nisei Veterans Memorial Center, and TMK 3-8-07: 38, the A & B Properties OWA Subdivision are located in the dune areas near the beach, only about one half kilometer to the north. Site 50-04-3119B, on the Nisei Veterans Memorial Center produced pre-contact archaeological cultural materials and a very early radiocarbon date of 1790 +/- 70 RCYBP (Stuiver and Pearson calibrated date: 233-410 A.D.). Cultural materials included midden, a one-piece fishhook, coral files, basalt flakes and hammerstones. The artifacts are consistent with what would be expected in coastal dune cultural deposits, i.e., representing activities related to coastal resource exploitation (Fredericksen, December, 1992). Data recovery at that site is presently continuing.

The OWA Subdivision site, TMK 3-8-07: 38, contains historic materials and remnants of the former Kahului Railroad track-bed (Site 3112) and Makaweli Rock Crusher Mill (Site 3135), but no clear pre-contact Hawaiian archaeological sites were discovered (Fredericksen, November 1992). The coastal beach dune no longer extends eastward beyond this site, having been removed during numerous landmoving activities for other construction projects lying to the south and east.

A Bernice P. Bishop Museum survey party described site 50-04-1172, TMK 3-8-36: 94, during a 1972 survey. It was an archaeological site containing midden and 3 pre-contact artifacts, but no further work was carried out although it was recommended that further work be done at a future time (Connolly, 1973).

With the exception of Site 50-04-3119 and Site 50-04-1172, all perused archaeological literature of surveys nearby the subject parcel did not produce indications of pre-contact land use in the past.

Settlement Pattern Summary

The ahupua'a of Wailuku was large, covering 24,000 acres and extended through several cultural zones. It was most heavily inhabited in its western portion, where there was more available water. The eastern portion was comparatively dry and barren. Cordy (1978, p. 59) gives population figures for the ahupua'a in 1831-32 as 2256 persons.

The subject parcel lies near the beach in what is

described by Kirch (1985) as a "coastal living zone". Prehistorically, these were zones where housing might be found, especially in dry areas with a nearby freshwater source. In beach coastal areas, "dry areas" would probably be limited to elevated dunes. Land around Kahului Bay was swampy, mauka of the coastal dunes, shifting to barren lands inland in the eastern portion of the ahupua'a.

Reports of fishing huts fringing Kahului Bay in the mid-eighteenth century have not been substantiated by recent archaeological work in the area. This is most likely due to the disrupting effects of constructing Kahului Harbor coupled with the temporary, insubstantial construction of the huts. Expansion and enclosure of the Harbor within the Bay required considerable dredging and the construction of seawalls, breakwaters and piers. These activities probably destroyed any traces of fishing huts that may have been in the area.

As discussed earlier, the study parcel has been greatly disturbed by various landfill and construction activities. If there were coastal dunes originally present on the property, they are long gone. If there were any temporary subsistence habitats, they probably were also destroyed during these processes.

Human burials historically occur in coastal dune areas. Because of the destruction of possible dunes on the parcel, the probability for discovery of human burials is low, but cannot be ruled out.

#### ARCHAEOLOGICAL FIELD SURVEY

From four to six field personnel were involved in the field survey. Principal investigators were Demaris L. Fredericksen, MA, PhD (ABD) and Walter M. Fredericksen, MA, PhD (ABD). The research strategy included a walk-over reconnaissence surface survey of all accessible areas. Possible features were flagged for further observation and/or testing by manual or mechanical excavation. The subsurface backhoe tests emphasized exploration of what appeared to be relatively untouched portions of the parcel. Floral and faunal observations were made and recorded during the initial survey.

#### SURVEY FINDINGS

This parcel is best described as being very disturbed by

previous landfill projects, military construction, railroad bed construction, commercial projects and most recently, State of Hawaii projects concerned with construction of the University of Hawaii, Maui Community College campus. If there were coastal dunes on this parcel, they were probably destroyed during early landfill and other construction activities, further reducing the possibility of any archaeological features remaining on the parcel.

A line of coconut palms dominate the vegetative landscape. These lined a Kahului Railroad Line, and later the roadway leading into the 18th Marine camp and are shown in Photo 4, as they looked in 1946, just after the tsunami. Photo 5 shows the trees today. Mr. Ernest Rezents estimates that they are about 75 years old. The Kahului Railroad tracks, restored after the war, ran north of them, and ties remained until the 1970's.

Several features exist on the property associated with the military operations in World War II. Three poured concrete foundations remain, two which appear to be latrines or showers. These are 5.5 meters long and 3 meters wide and are bifercated by a wall foundation. Each bifercation has a 75 cm. doorway on the northern side. The wall foundations are elevated about 5 cm. above the floor level, and are 10 cm. thick. Each room has a drain, one is covered with grating and the other open. The other foundation is somewhat smaller, being 3.7 meters in length and 3 meters wide, and has only one 75 cm. wide entrance.

Another feature appears to be a pond of some sort. It is made of concrete and rounded stones, and stands about 60 cm. above ground level. The depression is 45 cm. deep. Along side it is a horse-shoe shaped concrete and rounded stone flower bed, 2.4 meters in width, and 1.6 meters long. A large kiawe tree is growing in the middle. A poured concrete L-shaped sidewalk, 1 meter wide completes the configuration. See Figure 8, and Photos 2 and 3.

Broken concrete slabs, pipes of all types and sizes (i.e., steel, galvanized steel, concrete, plastic and composite), discarded automobiles and parts, quantities of discarded construction material and equipment, including electrical wiring and insulators, timber, and general types of household refuse and/or litter are found on or buried in the parcel. Remnants of basic shelter camps, some apparently only recently abandoned by their itinerant builder/dwellers, were found scattered throughout the parcel.

Landfill from the construction of the present campus parking lot extends up to 40 meters into the subject parcel, increasing

elevations as much as 1.5 meters. Composition of the fill varies, but includes discarded construction materials, oxidized machinery and remnants of other equipment.

Surface features included floor and foundation remains of World War II military installations mentioned above, and the Kahului Railroad trackbed which crossed just north of the parcel and was removed during construction of the Harbor Lights Project. There had also been several residential dwellings just north of the project area which were removed during the early 1970's. Nothing remains of these former residences. Their architectural style can be seen in Photos 6 and 7.

Subsurface excavation was done with a mechanical backhoe. Trenches were similar in dimensions throughout the site, measuring ca. 4 meters in length, by .5 meters in width, and 2.5 meters in depth, depending on depth of groundwater. Most trenches were placed in areas of least apparent disturbance. Sifting was done only where it seemed cultural materials might be present in the fill. A .25 mesh screen was used.

A total of 22 backhoe test trenches were excavated, recorded and mapped. Profiles were prepared for trenches that were stratigraphically representative of other trenches or were unusual for their content. Map 4 shows the location of the backhoe trench series. Table #1 summarizes detailed information on the backhoe test trenches.

No recognizable features or identifiable pre-contact Hawaiian artifacts were recovered from any of these trenches. Recent historic artifacts discovered consisted of aluminum drink containers, glass bottle sherds, plastic bits, oxidized metal (e.g., auto parts, machinery, etc.), and the like. Several abandoned automobiles decorate the landscape. A nearly complete "shot" glass was recovered, and may have been from the World War II Marine military base, as were several sherds of white crockery characteristic of military issue.

#### SUMMARY AND CONCLUSIONS

As noted throughout this report, the most notable features of this survey include the extreme land disturbance from previous construction projects on the parcel. Historic references note the unused, barren nature of the land in the vicinity of the study parcel in early historic times. Archaeological work, including numerous backhoe tests, done on the contiguous property (Maui Community Arts and Cultural Center) produced no cultural

materials (Kennedy, 1990). Nothing remains of the coastal sand dunes that may have existed in the immediate area, or on the study property. No cultural features or early artifacts were discovered in any of the backhoe test trenches during the present survey.

We conclude it is very unlikely that significant archaeological materials are present on the parcel. The remains of the World War II, 18th Service Battalion, U.S.M.C. base camp might have significance under Criterion D. However, it is noted here that the U.S.M.C. camp was not built until 1944, making it less than 50 years old. Significance would have to be on the basis of public interest.

Although no evidences of human burials were found, the ever present possibility of their occurrence in sandy areas exists. For this reason we suggest monitoring in the early phases of site preparation and ground excavating/leveling, most particularly for the Retention Basin.

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#### TABLE 1

#### BACKHOE TRENCHES

- Test Trench #1: Dimensions: 4 m x .5 m x 2 m in depth.

  Located near eastern corner of survey parcel.

  Surface matrix (decaying vegetation, root penetration), 15-20 cmbs, sand (7.5 YR 4/2) to 1.3

  cmbs, chunks of reef coral fill at c. 1 mbs.

  Light-gray sand lens (7.5 YR 5/0) and ground water, 1.6 mbs. Sterile, no artifacts.

  See Figure 1.
- Test Trench #2: Dimensions: 4 m x .5 m x 2 m in depth.
  Yellowish-brown sand (7.5 YR 4/2) to ground water
  at 1.45 mbs. Chunk of reef coral at ca. 1 m. No
  stratigraphy. Sterile, no artifacts.
- Test Trench #3: Dimensions: 4 m x .5 m x 1 m in depth.

  This trench on sloping ground from c. 10' 12'

  amsl. This represents the edge of the fill that

  was dozed in from the MCC parking lot

  construction. Mixed fill, excavation aborted at

  c. 1 mbs. No stratigraphy. Sterile, no

  artifacts.
- Test Trench #4: Dimensions: 4 m x .5 m x 2.5 m in depth.

  Located in a depression north of dozed campus
  fill. Surface, light gray sand (7.5 YR 6/1) to
  c. 1.2 mbs, where a narrow band of crushed coral
  (c. 5 10 cm) separates upper layer from lower
  yellowish sand (7.5 YR 5/3) layer which continues
  to ground water at 2.25 mbs. Sterile, no
  artifacts.
- Test Trench #5: Dimensions: 4 m x .5 m x 2.5 m in depth.

  Located in depression bordered with dozed fill,
  near fire hydrant (un-numbered). Yellowish
  brown sand (7.5 YR 4/2) to c. 2.4 mbs and
  ground water. Several large root castings,
  otherwise, sterile, no artifacts.
- Test Trench #6: Dimensions: 4 m x .5 m x 4 m in depth.

  Located near near parking lot embankment.

Light-gray/beige sandy, compacted lens, (10 YR 7/2), slightly mineralized. At c. 2.6 mbs, some lithified sand. At a depth of c. 3.8 mbs, no apparent ground water. This was maximum backhoe depth. Sterile, no artifacts.

Test Trench #7: Dimensions: 4 m x .5 m x 3 m in depth.

Surface to 45 cmbs, light- gray sand lens

(10 YR 7/2), mixed fill. Some burned root and
charcoal, recent deposit. From c. 45 cmbs to 1.6

mbs, light-gray sand with some lithified sand
inclusions and whitish mottling (10 YR 7/1).

From 1.6 to 2.75 mbs sand (10 YR 5/2) with some
large chunks of beach coral fill. At 2.75 mbs,
moist sand indicates ground water.

See Figure 2.

A dense, rounded basalt stone was recovered in the upper 45 cm lens, with no provenience. A broken piece of a possible canoe rubbing stone was also recovered near the surface in mixed matrix, with no provenience. There was no indication of any other possible artifact content and the recovered bits may have been brought in with fill material.

- Test Trench #8: Dimensions: 4 m x .5 m x 2.5 m in depth.
  Sandy loam with grass and roots in matrix, to 20
  cmbs. Light yellowish, coarse grained sand to 40
  cmbs (7.5 YR 5/3). An intrusive lithified sand
  lens with same sand as above to 1.25 mbs. From
  1.25 mbs, dark brown sand with some coral chunks
  as fill, to ground water at c. 2.2 mbs. No
  artifacts.
  See Figure 3.
- Test Trench #9: Dimensions: 4 m x .5 m x 1.5 m in depth.
  Surface root/grass matrix to 50 cmbs. Aeolian
  sand (7.5 YR 5/3), fine-grained and unstable.
  Trench wall continually collapsed and the trench
  was abandoned at 1.5 mbs. Sterile, no artifacts
  or inclusions.
- Test Trench #10: Dimensions: 4 m x .5 m x 3 m in depth.

  Near property border with MCACC Project.

  Recently burned undergrowth and charcoal in

  matrix of sandy loam/humus to c. 40 cmbs. Bands

  of lithified sand to 2.2 mbs, with red clay lens

(2 YR 3/6) to bottom of trench (3 m mbs). Some large angular rocks at c. 2.75 mbs. Sterile, no artifacts. See Figure 4.

Test Trench #11: Dimensions: 4 m x .5 m x 2 m in depth.

Near property border with MCACC Project.

This trench is characterized by large rocks,
electrical insulators, construction lumber, piles
of electrical wire, copper ground-strapping and
other materials likely cleared from the old
Quonset buildings formerly on the MCACC Project
parcel.

Some of the insulator brand names included: Brookfield, New York, Whitall, Tatum, Hemigray, Green-Patent May 2 1893.

This portion of the parcel is extremely disturbed from various construction projects and the materials that were discarded here. These materials are mixed with sand and obviously have been moved to their present site.

Test Trench #12: Dimensions: 4 m x .5 m x 2 m in depth.

Mixed surface overburden to 60 cmbs of sand
and root/grass material. Coral/stone fill to
1.25 mbs, gray stone/coral gravel mix to 1.85
mbs. A one foot long piece of 8" diameter
concrete pipe was found at from 60-80 cmbs, along
with a 2' long x 2" diameter metal pipe, a Coca
Cola bottle glass sherd, a nearly complete
"shot" glass, some porcelain sherds and beer
bottle glass sherds. The material looks like
possible harbor dredging, with stones
and recent historic artifacts mixed with grayish
"muck". Ground water was reached at 1.85 mbs.

Test Trench #13: Dimensions: 4 m x .5 m x 2 m in depth.

This trench bisects an old waterline pipe in its stone-filled trench. The fill is similar to TT #12, apparent dredged coral, sand and gray muck. There are numbers of pieces of concrete throughout the matrix. No artifacts.

See Figure 5.

Test Trench #14: Dimensions: 4 m x .5 m x 1.5 m in depth.

Located in a cleared road area, ground water

was reached through a homogeneous sand (10 YR 5/2) matrix at a depth of 1.40 mbs. Two pieces of coral were found in the matrix. No artifacts.

- Test Trench #15: Dimensions: 4 m x .5 m x 1.60 m in depth.

  Homogeneous fill, with coral and sand (10 YR 6/2)

  matrix to groundwater at 1.60 mbs. At 55 cm to

  85 cmbs, a 20 cm thick lens of 10 YR 4/3 sand
  intrudes the matrix. A single sherd of small
  bowl crockery was recovered.

  See Figure 6.
- Test Trench #16: Dimensions: 4 m x .5 m x 2 m in depth.
  From surface to 60 cmbs, a reddish-sand matrix
  with grasses and roots dominates. From 50 to 60
  cmbs, a charcoal lens nearly runs the length of
  the trench. From 60 cm to 1.25 mbs, probable
  Harbor dredging fill of coral and sand dominates,
  with a yellow sand lens (10 YR 4/3) to 1.50 mbs.
  Dark gray harbor dredged "muck"
  extends to ground water at 1.65 mbs.
  Sterile, no artifacts.
  See Figure 7.
- Test Trench #17: Dimensions: 4 m x .5 m x 1.4 m in depth.
  Surface covered with swampgrass and recent
  bottles (vitamin, ink, catsup) and glass sherds,
  several broken, shellfish fragments and a
  marble.
  Sand/coral probable harbor-dredging fill to
  ground water at 1.3 mbs. Subsurface fill was
  sterile, no artifacts.
- Test Trench #18: Dimensions: 4 m x .5 m x 1.60 m in depth.

  As in TT #17, surface covered with swampgrass, and surface littered chunks of cement, beer bottles and glass sherds (Budweiser), a few sawn pig bones and a few unidentifiable shellfish fragments.

  Coral/stone fill to 65-70 cmbs with numerous recent artifacts, e.g., rusted metal pipe, Dainia brown bottle, crockery fragments, rice vinegar bottle, green Star Ice & Soda Works bottle, a milk bottle top, a brown Purex bottle (top portion), Lysol Inc., Bloomfield, NJ. From 1 1.5 mbs, the fill matrix is dominated by the probable harbor dredged "muck". Ground water was encountered at 1.60 mbs.

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Test Trench #19: Dimensions: 4 m x .5 m x 1.9 m in depth.

Located in area of dense undergrowth. Same
coral/sand fill typical of other trenches.
Ground water at 1.9 mbs. With the exception of
a few beer bottle sherds at the surface, there
were no artifacts, and the subsurface fill was
sterile.

Test Trench #20: Dimensions: 4 m x .5 m x 2.2 m in depth.

This trench exhibits the same homogeneous sand fill to 1.4 mbs and the dredged gray "muck" to 2.2 mbs, where ground water was encountered. Sterile, no artifacts.

Test Trench #21: Dimensions: 4 m x .5 m x 2 m in depth.

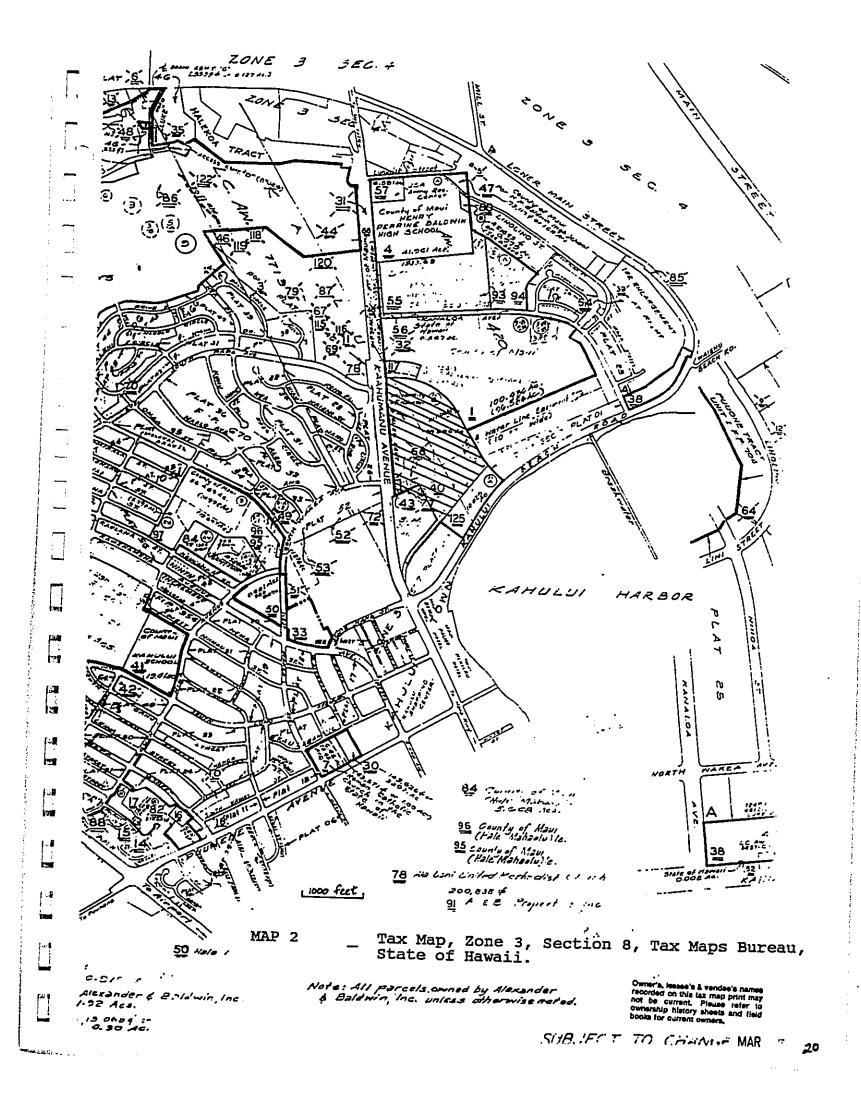
From surface to c. 1 mbs, sand matrix (7.5 YR 5/2), with several calcified, cemented root castings. Brown sand (10 YR 3/3) to 2 mbs.

no ground water. Sterile, no artifacts.

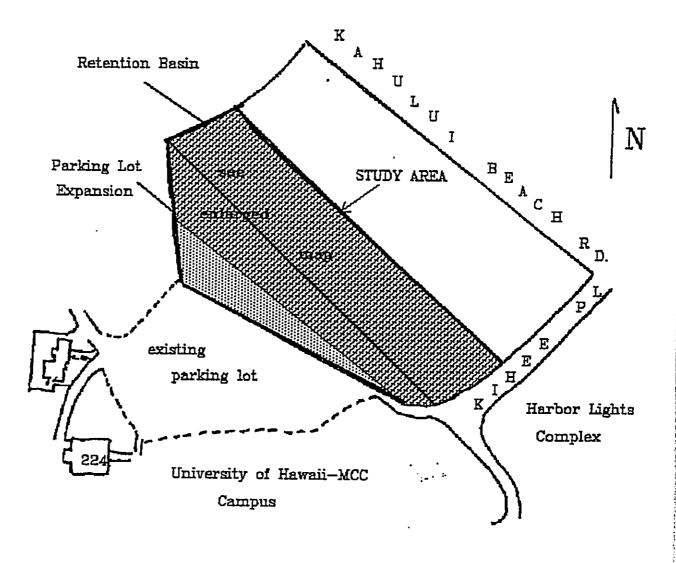
Test Trench #22: Dimensions: 4 m x .5 m x 1.3 m in depth.

This trench was excavated to check the
extent of disturbance from MCC parking lot
construction activity. It was mixed matrix
including discarded construction materials,
equipment and various cast-off food and
beverage containers, etc. Soil was mixed
sand, concrete pieces and broken rock. There
were no features or archaeological artifacts.
The trench was aborted at 1.3 mbs.

The test trenches were generally sterile and showed mixed, confused soil. This probably resulted from fill materials being brought in from outside the parcel, or from cast-off materials and mechanical equipment grading activities connected with various past construction activities. A rather uniform consistency fill was discovered in numbers of the trenches, most probably being harbor dredged material described as "muck".

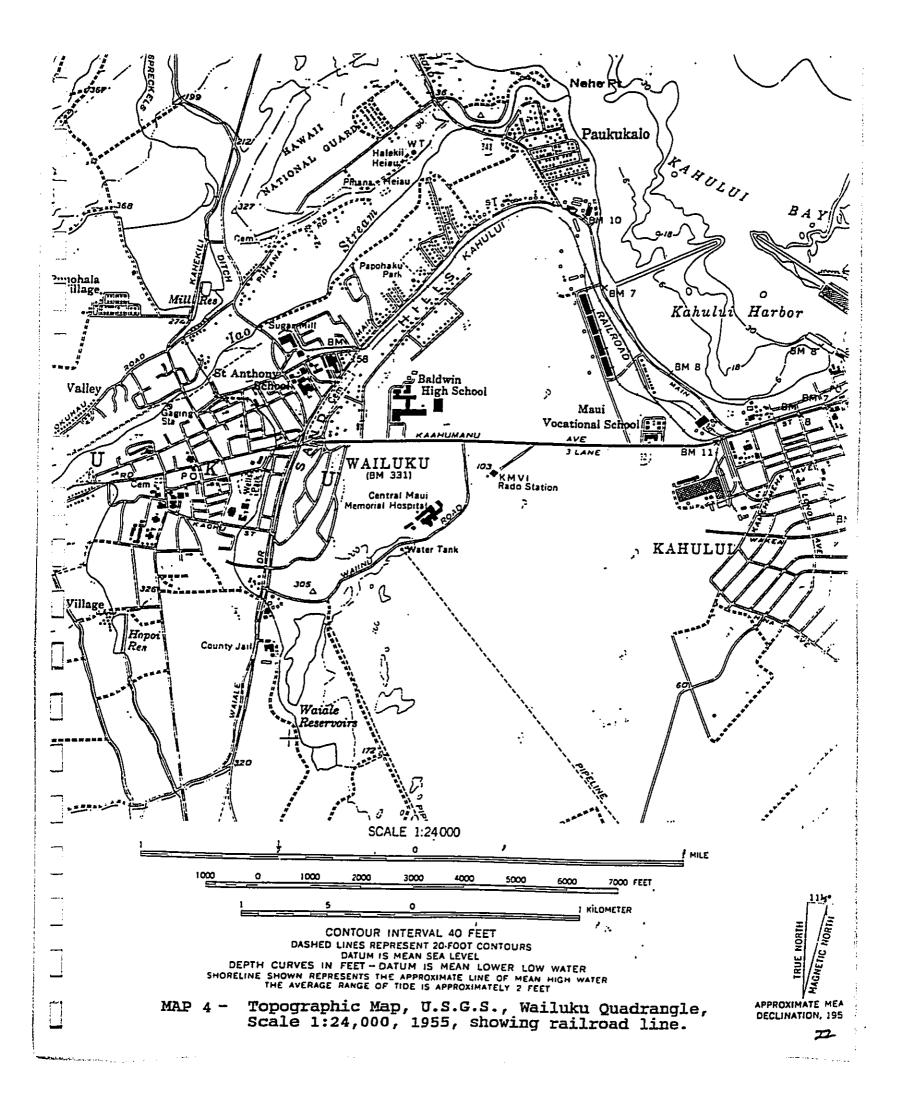


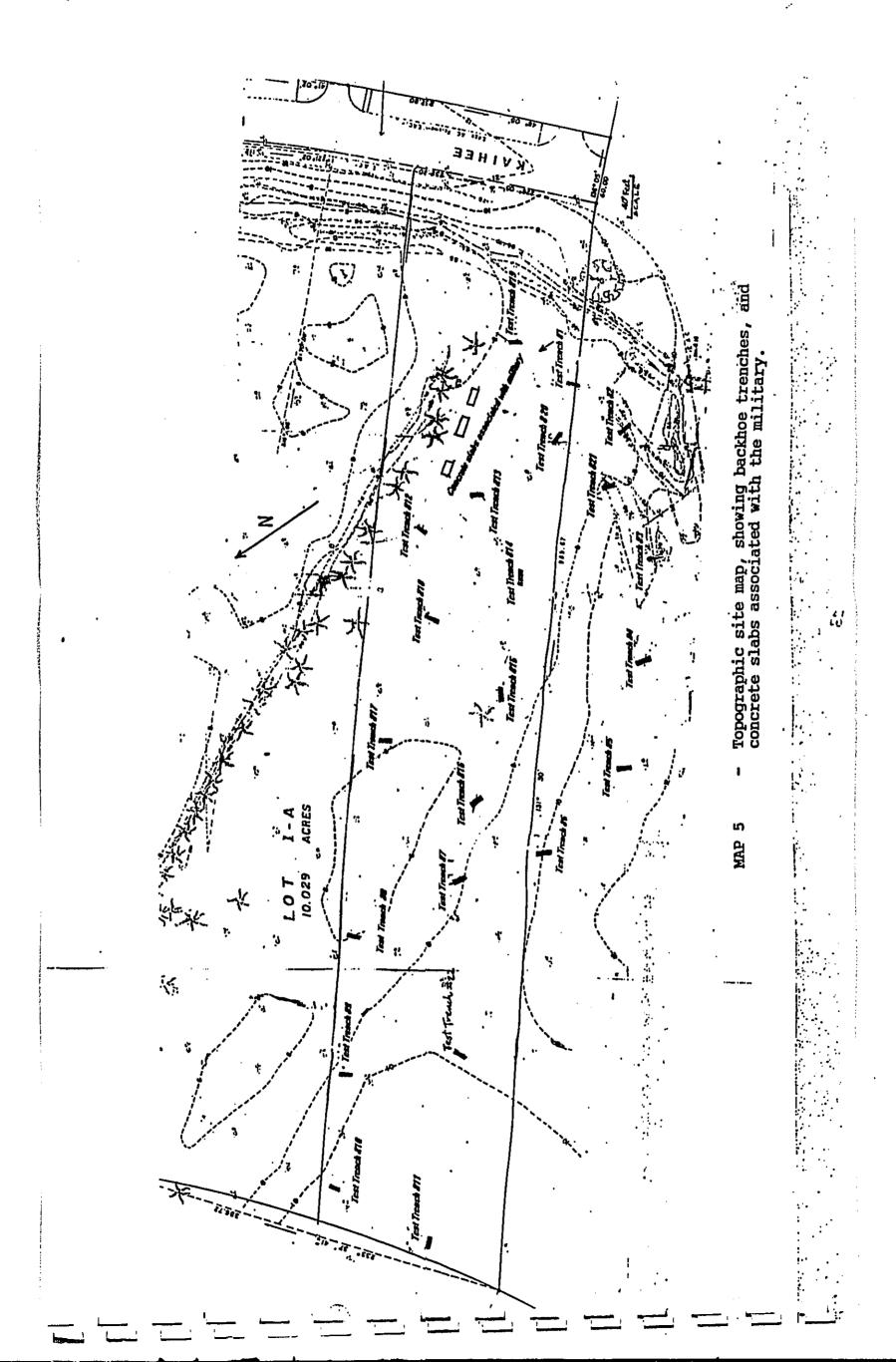
## MAUI COMMUNITY ARTS AND CULTURAL CENTER

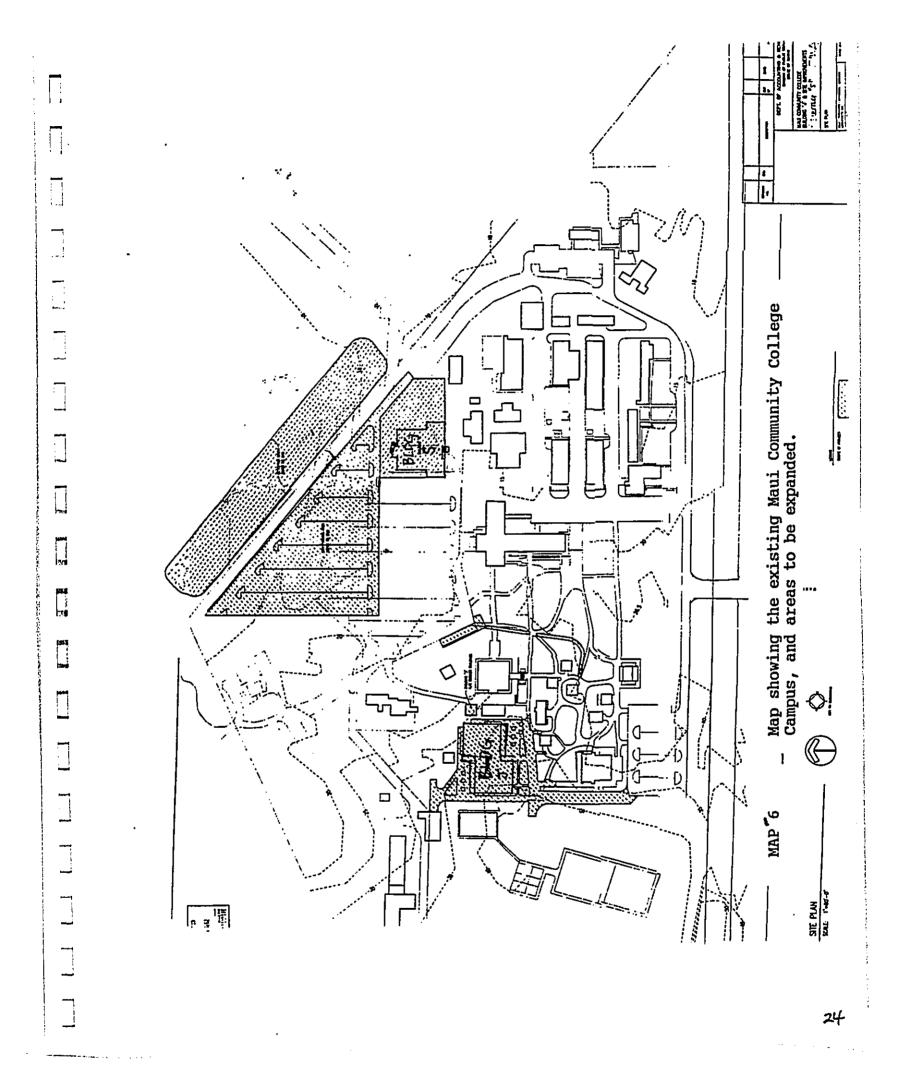


## KAAHUMANU AVENUE

MAP 3 - Map showing the area contained in the inventory survey.







### FIGURE 1: PROFILE TEST TRENCH 1

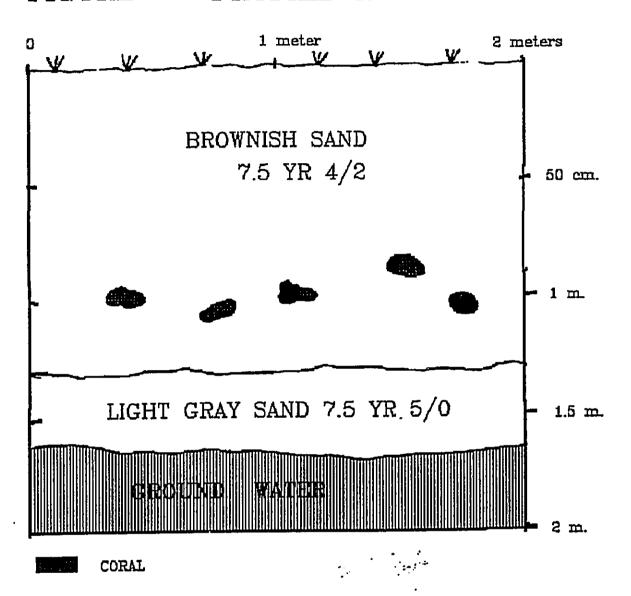
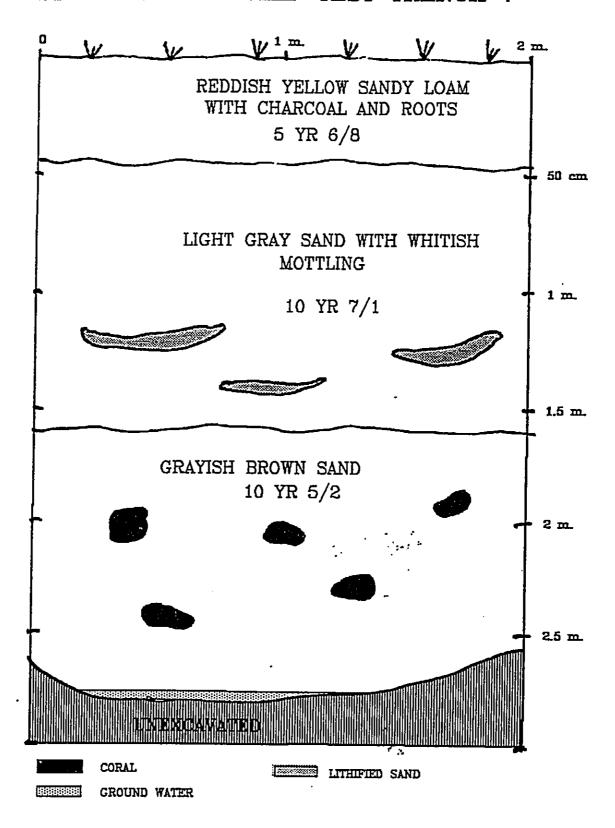


FIGURE 2: PROFILE TEST TRENCH 7



# FIGURE 3: PROFILE TEST TRENCH 8

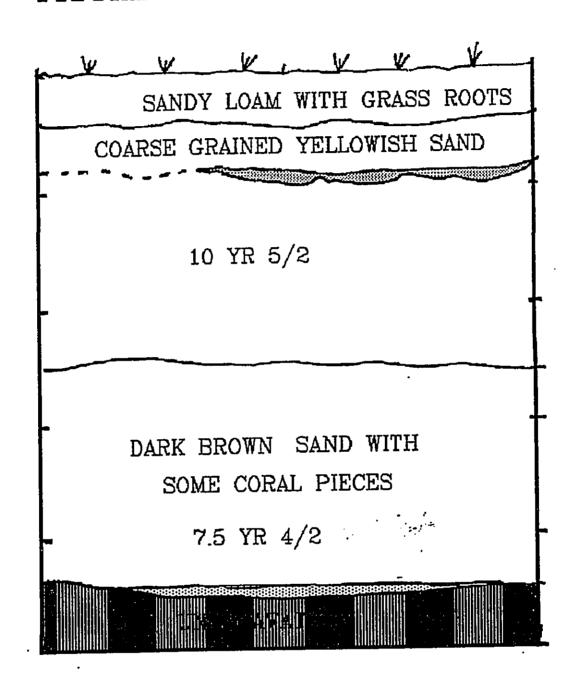


FIGURE 4: PROFILE TEST TRENCH 10

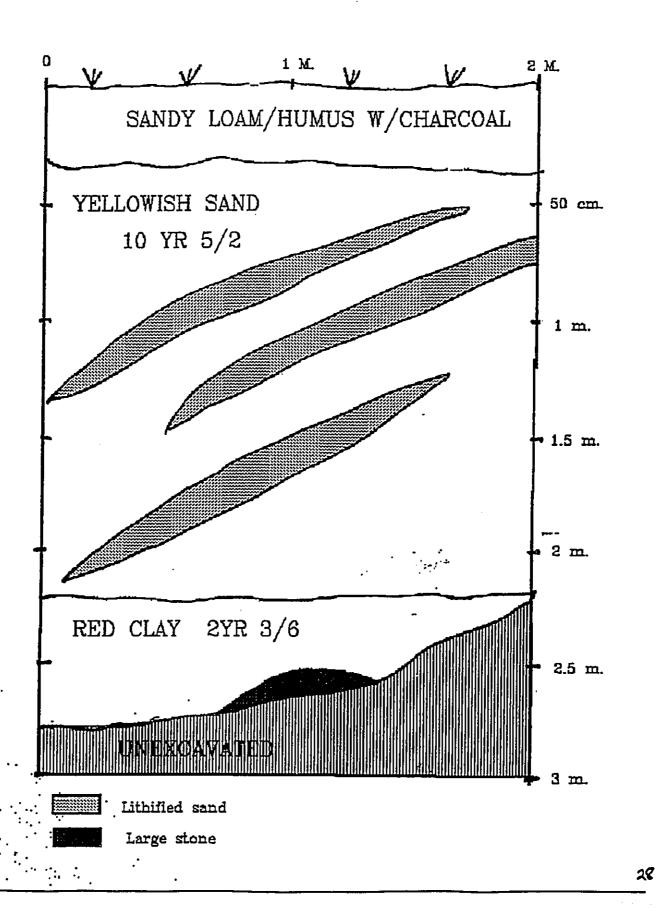


FIGURE 5: PROFILE TEST TRENCH 13

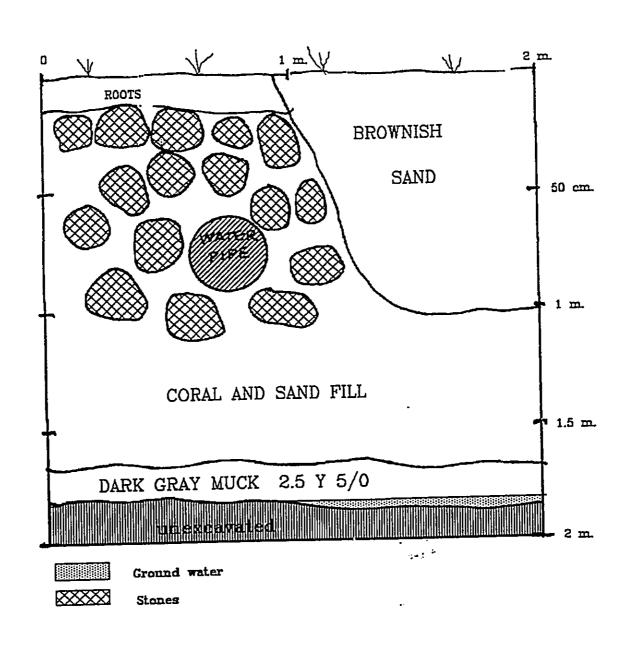


FIGURE 6: PROFILE TEST TRENCH 15

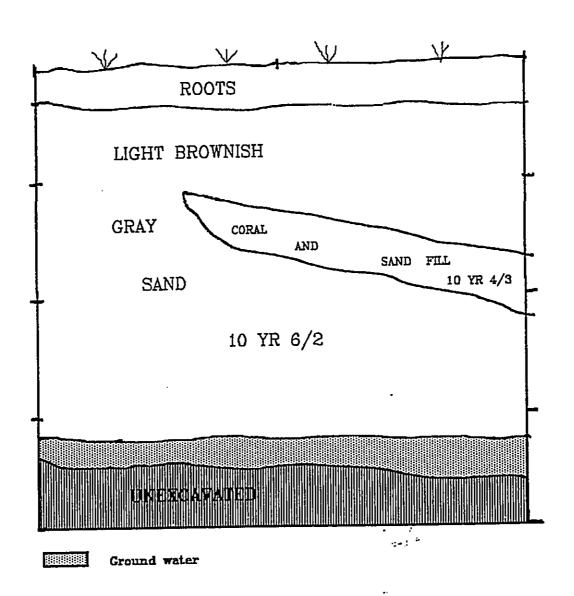


FIGURE 7: PROFILE TEST TRENCH 16

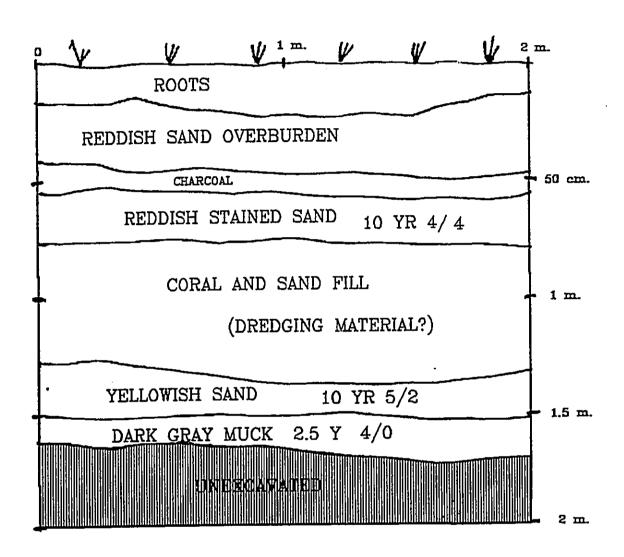
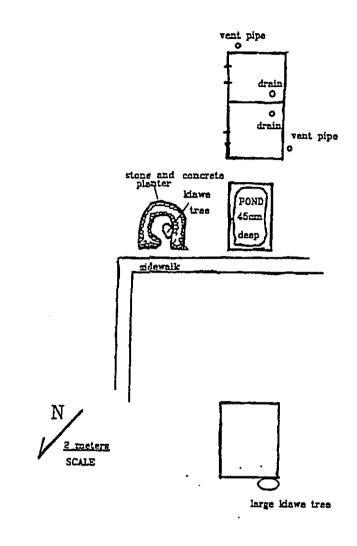


FIGURE 8: Concrete Foundations



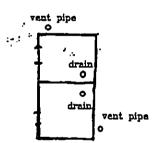


FIGURE 8 - Plan map of the concrete foundations associated with military activities during the Second World War.



View of early phase of construction of Maui.
 Vocational School at the present Kaahumanu Avenue location of study parcel (1949).









PHOTO 5 - Same line of trees, 1992.



 View of the makai side of the study parcel showing damage done during the 1946 tsunami. (Maui Historical Society Archives)





PHOTO 8 - Backhoe testin in dense vegetation on subject parcel

# Appendix B

Traffic Impact Assessment

# TRAFFIC IMPACT ANALYSIS REPORT FOR THE PROPOSED MAUI COMMUNITY COLLEGE EXPANSION

PREPARED FOR

GIMA YOSHIMORI MIYABARA DEGUCHI
ARCHITECTS, INC.

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Prepared by

Austin, Tsutsumi & Associates, Inc. Engineers • Surveyors Honolulu • Wailuku • Hilo, Hawaii

# TRAFFIC IMPACT ANALYSIS REPORT FOR THE PROPOSED MAUI COMMUNITY COLLEGE EXPANSION

# PREPARED FOR GIMA YOSHIMORI MIYABARA DEGUCHI ARCHITECTS, INC.



Prepared by

Austin, Tsutsumi & Associates, Inc. Engineers • Surveyors Honolulu • Wailuku • Hilo, Hawaii

November 1992

### AUSTIN. TSUTSUMI & ASSOCIATES, INC.

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#### EXECUTIVE SUMMARY

#### I. INTRODUCTION

The purpose of this study is to analyze the traffic impacts resulting from the proposed Maui Community College (MCC) expansion in Kahului, Maui, Hawaii. The expansion of MCC is expected to provide additional classroom space to accommodate 5,000 full-time equivalent (FTE) students, an increase of 2,200 students over the 1992 enrollment. This study is analyzed at full build-out condition by the Year 2000 at the 5,000 FTE enrollment level.

The study area includes the intersections of:

- 1. Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue
- 2. Kaahumanu Avenue and South Papa Avenue
- 3. Kaahumanu Avenue and Wakea Avenue
- 4. Kaahumanu Avenue and Kahului Beach Road/Kane Street
- 5. Kahului Beach Road and Kaihee Place

#### A. Existing Traffic Operations

From a regional perspective, Kaahumanu Avenue and Kahului Beach Road/Lower Main Street are the only major roadways linking the two urban centers of Wailuku and Kahului. These roadways are heavily utilized during the peak periods of commuter traffic. Although relatively heavy, vehicular traffic generally moves well on Kaahumanu Avenue in the project vicinity. No apparent problems were noted during the AM peak hour of traffic. However, during the PM peak hour, the east bound right-turn movement on Kaahumanu Avenue to south bound South Papa Avenue queues on Kaahumanu Avenue. Much of the east bound Kaahumanu Avenue traffic entering the South Papa Avenue intersection appear to be trips primarily generated by the

Maui Memorial Hospital on Mahalani Street, and Baldwin High School and the War Memorial Complex on Kanaloa Avenue.

#### B. Projected Traffic

#### 1. Site-Generated Traffic

The projected increase in student enrollment is expected to generate 353 vehicles during the AM peak hour of traffic, 342 vehicles entering and 11 vehicles exiting. During the projected PM peak hour of traffic, the increase in student enrollment is expected to generate 264 vehicles, 198 vehicles entering and 66 vehicles exiting.

#### 2. External Traffic

The State Department of Transportation and the County of Maui have developed the "Maui Long-Range Highway Planning Study, Wailuku-Kahului Plan " (Wailuku-Kahului Plan), dated May 1991 as a supplement to the "Maui Long-Range Highway Planning Study, Island-Wide Plan" (Island-Wide Plan), dated May 1991, both prepared by Austin, Tsutsumi & Associates, Inc. The travel forecasts developed by this study indicate a reduction in traffic demand on Kaahumanu Avenue for the Year 2000 from the present. The travel forecasts are based upon land use forecasts and highway improvements for the Year 2000 resulting from regional developments expected to occur in the Kahului/Wailuku area during this time frame. The roadway improvements cited in the report include:

- The extension of Mahalani Street to Waiale Road.
- The construction of Maui Lani Parkway, a new roadway between Kaahumanu Avenue and Kuihelani Highway.
- The extension of Kamehameha Avenue through the new Maui Lani Parkway.

- The extension of Onehee Avenue to the new Maui Lani Parkway.
- The extension and widening of Waiale Road to Honoapiilani Highway.
- Roadway widening of Waiale Road between the Kahekili Extension and the Mahalani Street Extension.
- The Puunene Bypass Road between Mokulele Highway and Kuihelani Highway, connecting to the Maui Lani Parkway.

#### C. Traffic Impact Analysis

The total number of vehicles entering the intersection of Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue would increase by approximately 4% over projected traffic demands without the proposed MCC expansion during the AM peak hour. During the PM peak hour, the increase would be approximately 3%.

At the intersection of Kaahumanu Avenue and South Papa Avenue, the north bound approach of South Papa Avenue would operate at LOS "D" during the AM peak hour. Under projected conditions without the proposed project, this approach would operate at LOS "C". The approach would continue to operate at LOS "C" during the PM peak hour. The other traffic movements at this intersection would operate at satisfactory LOS during the AM and PM peak hours with the proposed project.

Vehicular traffic entering the MCC main entrance at the intersection of Kaahumanu Avenue and Wakea Avenue is expected to increase by 105 vehicles during the projected AM peak hour of traffic with the proposed project. During the PM peak hour, entering traffic would increase by 45 vehicles. Despite the increase in traffic entering MCC, the traffic movements at this intersection would operate at satisfactory LOS during the projected AM and PM peak hours.

The total number of vehicles entering the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street is expected to increase by approximately 3% over projected conditions without the proposed project. During the PM peak hour, the increase is expected to be 2%.

#### II. PROPOSED IMPROVEMENTS AND RECOMMENDATIONS

#### A. Improvements to Mitigate Existing Roadway Deficiencies

- Provide exclusive left-turn, through, and right-turn lanes on the north bound Mahalani Street approach at Kaahumanu Avenue.
- Construct an exclusive left-turn lane, an optional left/through lane, and an exclusive right-turn lane on the north bound approach of Wakea Avenue at Kaahumanu Avenue.

#### B. Site Access Recommendations

- Restrict access from Kaihee Place and utilize the new road connecting Kanaloa Avenue to Kahului Beach Road as the access to the faculty/ student parking lot located north of the campus.
- Provide an exclusive right-turn lane and an optional left/through lane on the south bound approach exiting the MCC campus at the intersection of Kaahumanu Avenue and South Papa Avenue.
- Provide an exclusive right-turn lane and an optional left/through lane on the south bound approach exiting the MCC campus at the intersection of Kaahumanu Avenue and Wakea Avenue.
- Each of the campus parking lot driveways on the South Papa Avenue Extension should have exclusive left-turn and right-turn lanes for vehicles exiting the parking areas.
- Left-turn lanes to the parking lot driveways should be constructed to maintain through traffic flow on the South Papa Avenue Extension.

#### III. CONCLUSIONS

Based upon the "Wailuku-Kahului Plan", the proposed roadway improvements and new roadways included in the development plans of other projects in the Kahului/Wailuku area are expected to reduce the traffic demand on Kaahumanu Avenue. The projected AM and PM peak hour traffic volumes on Kaahumanu Avenue, west of Mahalani Street/Kanaloa Avenue, with the proposed MCC expansion, would increase by 3% and 2%, respectively, over projected conditions without the proposed project. East of Kahului Beach Road/Kane Street, the AM and PM peak hour traffic projections on Kaahumanu Avenue, with the proposed expansion, would each increase by approximately 2%.

With the implementation of roadway improvements and the construction of new roads discussed in this report, the proposed Maui Community College expansion should not have any significant impact on traffic in the vicinity of the project.



AUSTIN, TSUTSUMI & ASSOCIATES, INC.

CIVIL ENGINEERS . SURVEYORS

CONTINUING THE ENGINEERING PRACTICE FOUNDED BY H. A. R. AUSTIN IN 1934

TED S. KAWAHIGASHI, P.E. GEORGE M. NEUFFER, P.E. KENNETH K. KUROKAWA, P.E. THOMAS S. OTAGURO IVAN K. NAKATSUKA, P.E.

#### TRAFFIC IMPACT ANALYSIS REPORT

#### FOR THE PROPOSED

#### MAUI COMMUNITY COLLEGE EXPANSION

#### I. INTRODUCTION

#### A. Purpose of Study

The purpose of this study is to analyze the traffic impacts resulting from the proposed Maui Community College expansion in Kahului, Maui, Hawaii. This report presents the findings and recommendations of the study.

#### B. Scope of Study

The scope of this study includes:

- Description of the proposed project.
- 2. Evaluation of existing roadway and traffic conditions.
- 3. Analysis of future roadway and traffic conditions without the proposed project.
- 4. Development of trip generation characteristics for the proposed project.
- 5. Superimposing the site-generated traffic over future traffic conditions.
- 6. The identification and analysis of traffic impacts resulting from the proposed project.
- Recommendation of improvements, if appropriate, that would mitigate the traffic impacts resulting from the development of the proposed project.

#### II. PROJECT DESCRIPTION

#### A. Location

Maui Community College (MCC) is located immediately north of Kaahumanu Avenue, between the intersections with Kahului Beach Road and South Papa Avenue. MCC is near residential areas and shopping centers to the south and Kahului Harbor to the east. The vicinity of the project is shown in Exhibit 1.

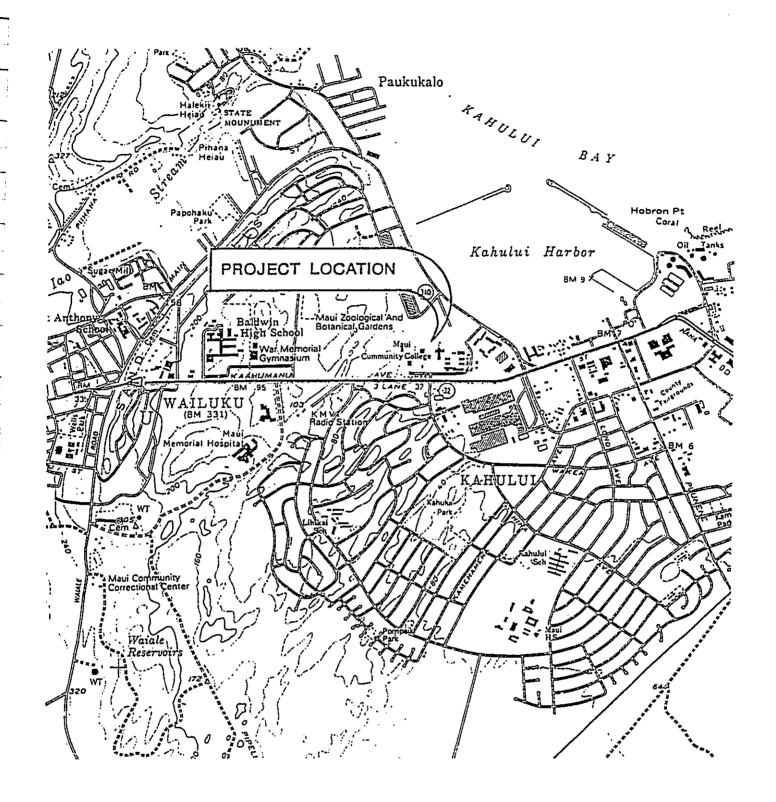
#### B. Site Characteristics

#### 1. Project Site Access

Site access would be provided by three access points. The existing main entrance on Kaahumanu Avenue at Wakea Avenue would remain as the main entrance. The two other access points would be provided on an extension of South Papa Avenue and on a new road connecting Kanaloa Avenue to Kahului Beach Road, north of the project site. This new road is expected to be completed with the construction of the new Maui Community Arts and Cultural Center, located north of the MCC campus. The access driveways on the South Papa Avenue extension, and on the new road, would lead to student and faculty parking lots. For the purpose of this traffic impact analysis, site access is assumed to be provided by these access points. The site plan is shown in Exhibit 2.

#### 2. Proposed Land Use Intensity

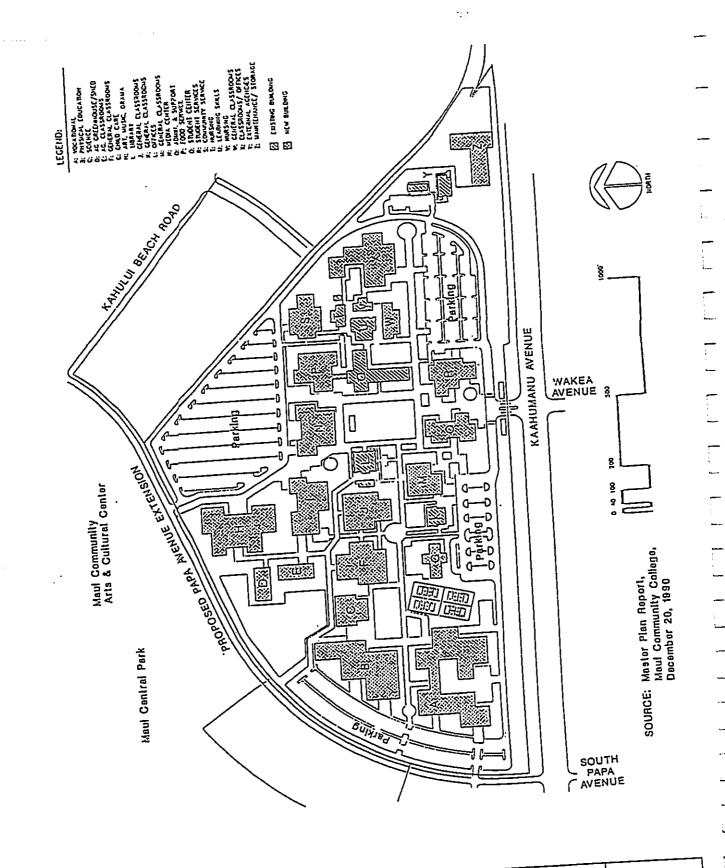
MCC enrollment in the Year 1992 is approximately 2,800 students. The "Maui Community College Master Plan Report", prepared by Gima, Yoshimori & Associates, AIA, Inc., dated December 20, 1990, includes an Ultimate Site Plan that provides additional classroom space to



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TRAFFIC IMPACT REPORT
MAUI COMMUNITY COLLEGE
VICINITY MAP



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TRAFFIC IMPACT REPORT

MAUI COMMUNITY COLLEGE

PROJECT SITE PLAN

2

-4-

accommodate 5,000 full-time equivalent (FTE) students, an increase of 2,200 students over the 1992 enrollment. For the purpose of this study, the project is analyzed at full build-out condition by the target Year 2000 at the 5,000 FTE enrollment level.

#### III. EXISTING TRAFFIC CONDITIONS

#### A. General

The traffic signal system on Kaahumanu Avenue is coordinated between Wharf Street on the east and Mahalani Street/Kanaloa Avenue on the west by an on-street master controller. Kaahumanu Avenue is a four- to six-lane divided urban arterial roadway generally oriented in the east-west direction. Kaahumanu Avenue is a major roadway that connects the two urban centers of Kahului and Wailuku. Kaahumanu Avenue is signalized at the intersections fronting MCC, with left-turn storage lanes and right-turn deceleration and acceleration lanes at major intersections. Kaahumanu Avenue intersects Kahului Beach Road, which leads to Lower Main Street and Waiehu Beach Road in Wailuku. Kanaloa Avenue connects Kahului Beach Road and Kaahumanu Avenue. The posted speed on Kaahumanu Avenue, in the project vicinity, is 45 miles per hour.

South Papa Avenue and Wakea Avenue are two-lane, two-way collector roads which begin in the industrial area of South Kahului and connect to Kaahumanu Avenue through the Kahului residential area. South Papa Avenue, north of Kaahumanu Avenue, provides access to the MCC faculty housing. The main entrance to MCC is at the Kaahumanu Avenue and Wakea Avenue intersection. The entrance to the faculty/student parking lot is through Kaihee Place off Kahului Beach Road.

## 1. Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue Intersection

The intersection of Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue is controlled by a traffic signal system. Kaahumanu Avenue at this intersection has exclusive left-turn lanes to south bound Mahalani Street and north bound Kanaloa Avenue. Right-turn deceleration and acceleration lanes on west bound and east bound Kaahumanu Avenue are provided. The south bound Kanaloa Avenue approach consists of three lanes: an exclusive left-turn lane, and optional left-turn/through lane, and an exclusive right-turn lane to west bound Kaahumanu Avenue. There is an optional left-turn/through lane and an exclusive right-turn lane on the north bound Mahalani Street approach. This intersection is controlled by a six-phase traffic signal system with protected/permissive left-turn movements on Kaahumanu Avenue.

#### 2. Kaahumanu Avenue and South Papa Avenue Intersection

The intersection of Kaahumanu Avenue and South Papa Avenue is a signalized intersection. The east bound approach on Kaahumanu Avenue has an exclusive left-turn lane, two through lanes, and a right-turn deceleration lane to south bound South Papa Avenue. The west bound approach of Kaahumanu Avenue has an exclusive left-turn lane and two through lanes. The north bound South Papa Avenue approach has an exclusive left-turn lane and a shared left, through and right-turn lane. An acceleration lane is provided on Kaahumanu Avenue for the right-turn movement from north bound South Papa Avenue to east bound Kaahumanu Avenue. The south bound approach of South Papa Avenue has one lane for the left turn, through and right-turn movements. This intersection is controlled by a three phase traffic signal system with protected/permissive left-turn movements on Kaahumanu Avenue.

# 3. Kaahumanu Avenue and Wakea Avenue Intersection

The intersection of Kaahumanu Avenue and Wakea Avenue operates as a signalized intersection. Right-turn deceleration and acceleration lanes are provided on Kaahumanu Avenue for both east bound and west bound traffic. The east bound and west bound approaches on Kaahumanu Avenue at Wakea Avenue each have an exclusive left-turn lane, two through lanes, and a right-turn deceleration lane. The north bound approach of Wakea Avenue has an optional left-turn/through lane and an exclusive right-turn lane. The south bound approach exiting MCC has a shared left-turn, through and right-turn lane. This intersection is controlled by a five phase traffic signal system with protected/permissive left-turn movements on Kaahumanu Avenue.

# 4. Kaahumanu Avenue and Kahului Beach Road/Kane Street Intersection

The intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street operates as a signalized intersection. The west bound approach on Kaahumanu Avenue has four lanes: an exclusive left-turn lane, two through lanes, and a mandatory right-turn lane to north bound Kahului Beach Road. The east bound approach has an exclusive left-turn lane, two through lanes, and a right-turn deceleration lane to south bound Kane Street. The north bound Kane Street approach at Kaahumanu Avenue has an exclusive left-turn lane and a shared through/right-turn lane. The south bound approach of Kahului Beach Road has an exclusive left-turn lane, an optional left-turn/through lane, and a right-turn lane to west bound Kaahumanu Avenue. This intersection is controlled by a six phase traffic signal system with protected/permissive left-turn movements on Kaahumanu Avenue.

# 5. Kahului Beach Road and Kaihee Place Intersection

The entrance to the MCC faculty/student parking lot is through Kaihee Place. Kaihee Place intersects Kahului Beach Road, north of Kaahumanu Avenue, as an unsignalized tee-intersection. It is a two-lane, two-way roadway, which also provides access to the Harbor Lights Condominium. A left-turn lane on north bound Kahului Beach Road to Kaihee Place, and a right-turn deceleration lane on south bound Kahului Beach Road to Kaihee Place are provided. Kaihee Place, at Kahului Beach Road, is stop-controlled with a right-turn acceleration lane for vehicles from Kaihee Place to south bound Kahului Beach Road.

#### B. Traffic Volumes and Conditions

#### 1. General

#### a. Field Investigation

A field investigation was conducted in September 1992 to establish baseline traffic conditions. The field investigation was comprised of a site inspection of the road and traffic conditions and a traffic count survey. The traffic count survey was conducted between the hours of 6:30 and 8:30 AM in the morning, and between 3:30 and 5:30 PM in the afternoon at the intersections of Kaahumanu Avenue with Kahului Beach Road/Kane Street, Wakea Avenue, South Papa Avenue, and Mahalani Street/Kanaloa Avenue; and at the intersection of Kahului Beach Road and Kaihee Place.

#### b. Capacity Analysis Methodology

The highway capacity analysis performed for this study is based upon procedures presented in the "Highway Capacity Manual", Special Report 209, Transportation Research Board, 1985 and the "Highway Capacity Software", Federal Highway Administration.

Level of Service (LOS) is a quantitative and qualitative assessment of traffic operations. Levels of Service are defined by LOS "A" through LOS "F"; LOS "A" being the best operating condition and LOS "F" the worst operating condition.

"Volume-to-capacity (v/c) ratio is another measure indicating the relative traffic demand to the road's carrying capacity. A v/c ratio of 1.00 indicates that the roadway is operating at 100% of its capacity. A v/c ratio greater than one (1.00) indicates that the projected traffic demand exceeds the road's traffic handling capacity.

The definitions for the various Levels of Service are included in the Appendix.

#### 2. Traffic Operations

From a regional perspective, Kaahumanu Avenue and Kahului Beach Road/Lower Main Street are the only major roadways linking the two urban centers of Wailuku and Kahului. Although relatively heavy, vehicular traffic generally moves well on Kaahumanu Avenue in the project vicinity. No apparent problems were noted during the AM peak hour of traffic. However, during the PM peak hour of traffic, the east bound right-turn movement on Kaahumanu Avenue to south bound South Papa Avenue queues on Kaahumanu Avenue. Much of the east bound Kaahumanu Avenue traffic entering the South Papa Avenue

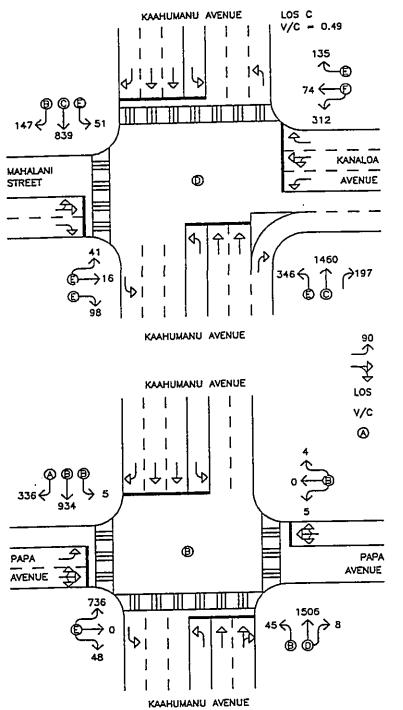
intersection may be trips generated by the Maui Memorial Hospital on Mahalani Street and the War Memorial Complex on Kanaloa Avenue. During the PM peak hour, the traffic counts show a high north bound right-turn volume from Mahalani Street to east bound Kaahumanu Avenue, and a high south bound left-turn volume from Kanaloa Avenue to east bound Kaahumanu Avenue. South Papa Avenue would provide access to the Kahului residential area for these trips. South Papa Avenue also serves as an alternate route for motorists destined to areas south and Up Country via South Puunene Avenue, Kuihelani Highway and Hana Highway. This route would enable motorists to bypass the retail centers on Kaahumanu Avenue and avoid entering the heavily utilized Kaahumanu Avenue and Kahului Beach Road intersection.

# 3. Existing AM Peak Hour Traffic Analysis

The AM peak hour of traffic in the study area occurs between 7:15 and 8:15 AM. The existing AM peak hour traffic volumes and operating Levels of Service are shown in Exhibits 3 and 4.

Kaahumanu Avenue, west of Kanaloa Avenue/Mahalani Street, carries 2,676 vehicles per hour (vph) during the existing AM peak hour, 1,639 vph west bound and 1,037 vph east bound. West bound traffic on Kaahumanu Avenue, just west of Mahalani Street/Kanaloa Avenue, operates at LOS "C" and at a v/c ratio of 0.49 during the existing AM peak hour of traffic.

The left-turn movement from Kanaloa Avenue to east bound Kaahumanu Avenue operates at LOS "F" during the AM peak hour of traffic. The other left-turn movements at the intersection of Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue operate at LOS "E". The





#### LEGEND

TRAFFIC MOVEMENT VOLUME (VPH)

LANE CONTROL

LEVEL OF SERVICE (MULTI-LANE HWY)

VOLUME—TO-CAPACITY RATIO

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

DATE OF COUNT: SEPTEMBER 30, 1992

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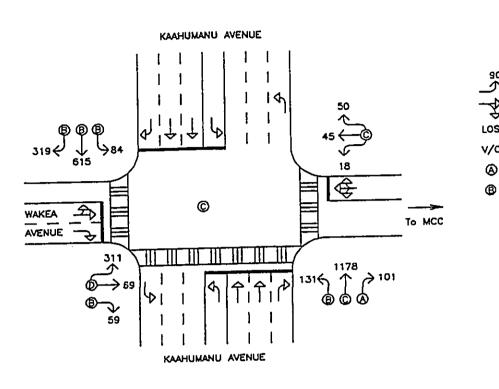
TRAFFIC IMPACT REPORT

MAUI COMMUNITY COLLEGE

EXISTING AM PEAK HOUR

**EXHIBIT** 





LEGEND

TRAFFIC MOVEMENT VOLUME (VPH)

LANE CONTROL

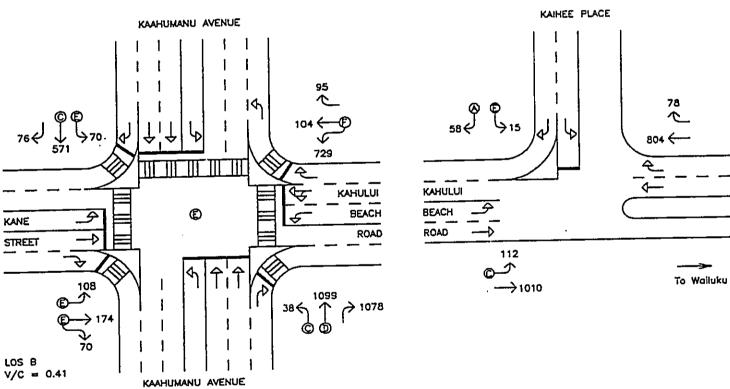
LEVEL OF SERVICE (MULTI-LANE HWY)

VOLUME-TO-CAPACITY RATIO

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

CAPACITY ANALYSIS (UNSIGNALIZED CONDITION)

DATE OF COUNT: SEPTEMBER 29, 1992 OCTOBER 1, 1992



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TRAFFIC IMPACT REPORT
MAUI COMMUNITY COLLEGE

EXISTING AM PEAK HOUR

EXHIBIT

intersection operates at an overall LOS "D" during the AM peak hour of traffic.

At the intersection of Kaahumanu Avenue and South Papa Avenue, the north bound double left-turn movement from South Papa Avenue to west bound Kaahumanu Avenue operates at LOS "E" during the AM peak hour of traffic. The west bound Kaahumanu Avenue through movement operates at LOS "D". The other traffic movements at the intersection of Kaahumanu Avenue and South Papa Avenue operate at satisfactory LOS. The intersection operates at an overall LOS "B" during the AM peak hour of traffic.

The north bound Wakea Avenue left/through movement operates at LOS "D" at the intersection with Kaahumanu Avenue during the AM peak hour of traffic. The opposing south bound traffic on Wakea Avenue from MCC is very light. The other movements at the intersection of Kaahumanu Avenue and Wakea Avenue operate at satisfactory LOS. The intersection operates at an overall LOS "C" during the AM peak hour of traffic.

The north bound approach of Kane Street at the intersection with Kaahumanu Avenue operates at LOS "E" during the AM peak hour of traffic. The south bound left-turn movement on Kahului Beach Road to east bound Kaahumanu Avenue operates at LOS "F". The east bound left-turn movement of Kaahumanu Avenue to north bound Kahului Beach Road operates at LOS "D". The east bound through movement also operates at LOS "D". The other movements at the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street operate at satisfactory LOS. The intersection operates at an overall LOS "E" during the AM peak hour of traffic. Kaahumanu Avenue, east of Kahului Beach Road, carries 3,585 vph during the AM peak hour, 1,370 vph east bound and 2,215 vph west bound.

At the intersection of Kahului Beach Road and Kaihee Place, the east bound left-turn and right-turn movements out of Kaihee Place to Kahului Beach Road operate at LOS "E" and LOS "A", respectively. The north bound left-turn movement from Kahului Beach Road to west bound Kaihee Place operates at LOS "C". Kaihee Place, just west of Kahului Beach Road, carries 280 vehicles, 201 vehicles entering and 79 vehicles exiting during the AM peak hour of traffic.

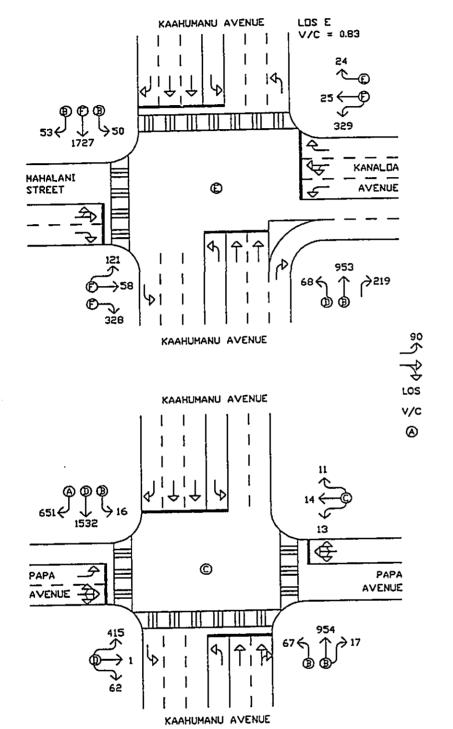
# 4. Existing PM Peak Hour Traffic Analysis

The PM peak hour of traffic in the study area occurs between 4:00 and 5:00 PM. The existing PM peak hour traffic volumes and operating Levels of Service are shown in Exhibits 5 and 6.

Kaahumanu Avenue, west of Mahalani Street/Kanaloa Avenue, carries 2,928 vph during the PM peak hour; 1,830 vph east bound and 1,098 vph west bound. East bound traffic on Kaahumanu Avenue operates at LOS "C" and at a v/c ratio of 0.55 during the PM peak hour of traffic.

The north bound approach of Mahalani Street and the south bound approach of Kanaloa Avenue, at the intersection with Kaahumanu Avenue, both operate at LOS "F". The east bound through movement on Kaahumanu Avenue at Mahalani Street/Kanaloa Avenue also operates at LOS "F". The intersection operates at capacity and at an overall LOS "E" during the PM peak hour of traffic.

At the intersection of Kaahumanu Avenue and South Papa Avenue, the north bound approach of South Papa Avenue and the east bound through movement on Kaahumanu Avenue both operate at LOS "D". The other traffic movements operate at satisfactory LOS during the PM peak hour of traffic. The intersection of Kaahumanu Avenue and South Papa Avenue operates at an overall LOS "C".





LEGEND

TRAFFIC MOVEMENT VOLUME (VPH)

LANE CONTROL

LEVEL OF SERVICE (MULTI-LANE HWY)

VOLUME-TO-CAPACITY RATIO

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

DATE OF COUNT: SEPTEMBER 30, 1992

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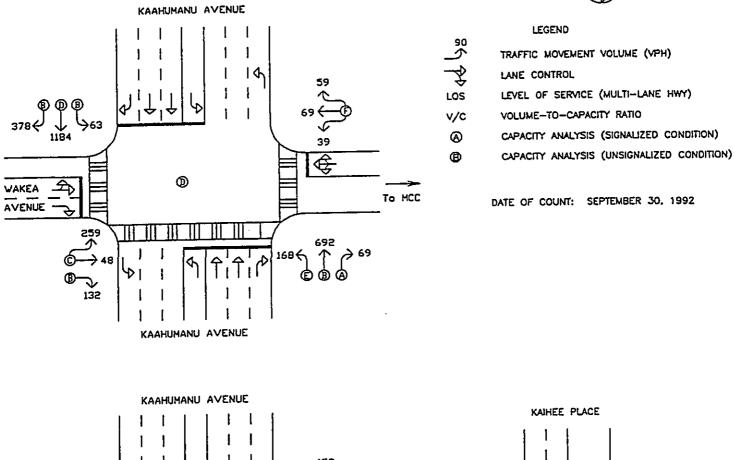
TRAFFIC IMPACT REPORT

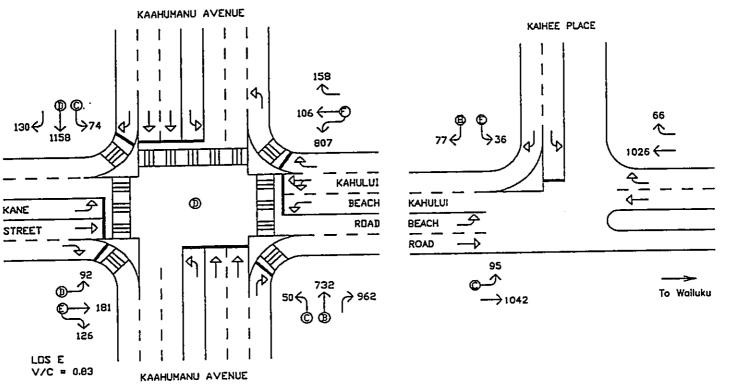
MAUI COMMUNITY COLLEGE

EXISTING PM PEAK HOUR

**EXHIBIT** 







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The south bound approach of Wakea Avenue at Kaahumanu Avenue operates at LOS "F" during the PM peak hour of traffic. The west bound left-turn movement from Kaahumanu Avenue to south bound Wakea Avenue operates at LOS "E", while the east bound through movement on Kaahumanu Avenue operates at LOS "D". The other traffic movements at the intersection of Kaahumanu Avenue and Wakea Avenue operate at satisfactory LOS. The intersection operates at an overall LOS "D" during the PM peak hour of traffic.

The south bound approach of Kahului Beach Road at the intersection with Kaahumanu Avenue operates at LOS "F" during the PM peak hour of traffic. The north bound approach on Kane Street at Kaahumanu Avenue operates at LOS "E". The intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street operates at an overall LOS "D" during the PM peak hour of traffic. With an intersection v/c of 0.978, the intersection operates at near capacity conditions.

Kaahumanu Avenue, just east of Kahului Beach Road/Kane Street, carries 3,837 vph during the PM peak hour; 2,093.vph west bound and 1,744 vph east bound. East bound traffic on Kaahumanu Avenue, east of Kahului Beach Road/Kane Street, operates at LOS "D" and at a v/c ratio of 0.63 during the PM peak hour of traffic.

At the intersection of Kahului Beach Road and Kaihee Place, the east bound left-turn and right-turn movements operate at LOS "E" and LOS "B", respectively. The north bound left-turn movement from Kahului Beach Road to west bound Kaihee Place operates at LOS "C". Kaihee Place, just west of Kahului Beach Road, carries 176 vehicles; 87 vehicles entering and 89 vehicles exiting during the PM peak hour of traffic.

#### IV. PROJECTED TRAFFIC

# A. Site-Generated Traffic

#### 1. Trip Generation Methodology

The trip generation methodology used in this study is based upon generally accepted techniques developed by the Institute of Transportation Engineers (ITE) and published in "Trip Generation, 5th Edition", 1991. The ITE trip rates are developed empirically, by correlating the vehicle trip generation data with various land use characteristics, such as vehicle trips per student.

# 2. Trip Generation Characteristics

The proposed Maui Community College expansion is expected to accommodate an increase of 2,200 students over the 1992 enrollment of 2,800 students. The "Ultimate Site Plan", included in the "Maui Community College Master Plan Report", prepared by Gima, Yoshimori & Associates, AIA, Inc., dated December 20, 1990, contains provisions for additional classroom space at the 5,000 FTE level. For the purpose of this study, the trip generation characteristics are based upon a total enrollment of 5,000 students. The projected increase in student enrollment is expected to generate 353 vehicles during the AM peak hour; 342 vehicles entering and 11 vehicles exiting. During the projected PM peak hour of traffic, the increase in student enrollment is expected to generate 264 vehicles; 198 vehicles entering and 66 vehicles exiting. The trip generation characteristics are summarized in Table 1.

TABLE 1. TRIP GENERATION SUMMARY

Project: Location: Independent	MCC Ex Kahului,			
Variable: Students			2,200	
Average Weekday Vehicle Trip Ends			ITE Rate 1.330	Trip Ends 2,927
Peak Hour of Adjacent Street	AM Peak Hour of Generator	Enter Exit Total	0.155 0.005 0.160	342 11 353
	PM Peak Hour of Generator	Enter Exit Total	0.090 0.030 0.120	198 66 264

# 3. Traffic Assignment

The vehicular traffic generated by the proposed project is assigned to the street system in the vicinity of the project. The residential population of major cities and towns on the island of Maui, obtained from "The State of Hawaii Data Book, 1990", by the State Department of Business, Economic Development and Tourism, was used as the basis for estimating traffic assignment. The traffic generated by the proposed project during the peak hours was distributed to the roadway system proportionately to the population of the cities and towns of the island of Maui. The directional distribution of existing traffic to and from MCC, obtained by the manual traffic counts taken in September of 1992, seem to correlate to the population distribution of the island of Maui.

# B. External Traffic

The State Department of Transportation and the County of Maui have developed the "Wailuku-Kahului Plan", dated May 1991, as a supplement to

the "Island-Wide Plan", dated May 1991, both prepared by Austin, Tsutsumi & Associates, Inc. The travel forecasts developed by this study indicate a reduction in traffic demand on Kaahumanu Avenue for the Year 2000 from the present. The travel forecasts are based upon land use forecasts and highway improvements for the Year 2000 resulting from regional developments expected to occur during this time frame in the Kahului/ Wailuku area. For example, developers of projects such as Maui Lani and the Wailuku and Piihana Project Districts include roadway improvements within their respective development plans. These roadway improvements would provide alternative access to Wailuku and Kahului and relieve much of the traffic congestion on Kaahumanu Avenue in the project vicinity. The roadway improvements cited in the report include:

- The extension of Mahalani Street to Waiale Road.
- 2. The construction of Maui Lani Parkway, a new roadway between Kaahumanu Avenue and Kuihelani Highway.
- 3. The extension of Kamehameha Avenue through the new Maui Lani Parkway.
- 4. The extension of Onehee Avenue to the new Maui Lani Parkway.
- 5. The extension and widening of Waiale Road to Honoapiilani Highway.
- 6. Roadway widening of Waiale Road between the Kahekili Highway Extension and the Mahalani Street Extension.
- 7. The Puunene Bypass Road between Mokulele Highway and Kuihelani Highway, connecting to the Maui Lani Parkway.

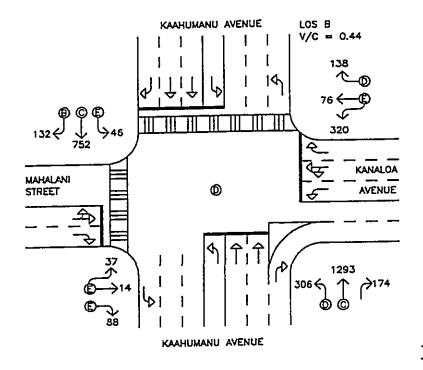
The proposed Maui Central Park is a development that would be located adjacent to the MCC campus. The park would provide recreational space and serve to separate the individual integrity of Wailuku and Kahului. The construction phasing of the proposed Maui Central Park is not

determined at this writing. For the purpose of this study, the trips generated by the proposed park are excluded from the traffic projections.

The proposed Maui Community Arts and Cultural Center would be located immediately north of the MCC campus. The Maui Community Arts and Cultural Center would include a 1,150-seat theater, a 200-seat support theater, a restaurant, and other theater-related facilities. Vehicular access would be provided by a new road connecting Kahului Beach Road and Kanaloa Avenue, immediately north of MCC. An exclusive left-turn lane on north bound Kahului Beach Road, at the new access road, is proposed. The new road would also intersect an extension of South Papa Avenue along the westerly boundary of MCC. Based on the "Maui Community College Master Plan Report", the new road would eliminate the need for the present Kaihee Place access. This study assumes that access to MCC's north parking lot is via the proposed new road. Based on a report titled "Traffic Impact Study, Maui Community Arts and Cultural Center", dated April 7, 1989, prepared by Parsons Brinkherhoff Quade and Douglas, Inc., vehicular trips generated by the proposed Maui Community Arts and Cultural Center would primarily occur during the off-peak periods of traffic on Kaahumanu Avenue. Therefore, these trips are excluded from the peak period traffic analysis.

# C. Total Traffic Volumes Without Project

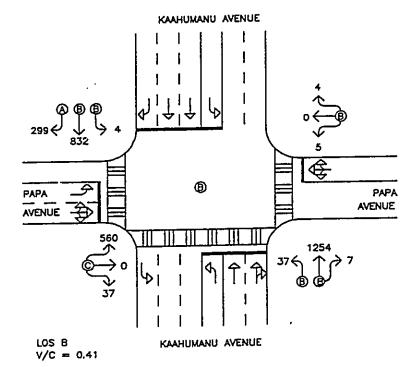
The Year 2000 traffic projections, without the proposed expansion, are shown on Exhibits 7 through 10. The "Wailuku-Kahului Plan" was used as the basis in projecting existing 1992 AM and PM traffic volumes to the Year 2000. Tables 2 and 3 show the operating overall LOS and v/c ratios of the intersections in the study area for the projected AM and PM peak hours of traffic, without the proposed expansion, respectively. Kaahumanu Avenue, west of Mahalani Street/Kanaloa Avenue, is expected to carry 2,398 vph during the projected AM peak hour of traffic; 930 vph east bound and 1,468 vph west bound. West bound traffic on Kaahumanu Avenue, west of





LOS V/C **(A)** 

LEGEND TRAFFIC MOVEMENT VOLUME (VPH) LANE CONTROL LEVEL OF SERVICE (MULTI-LANE HWY) VOLUME-TO-CAPACITY RATIO CAPACITY ANALYSIS (SIGNALIZED CONDITION)

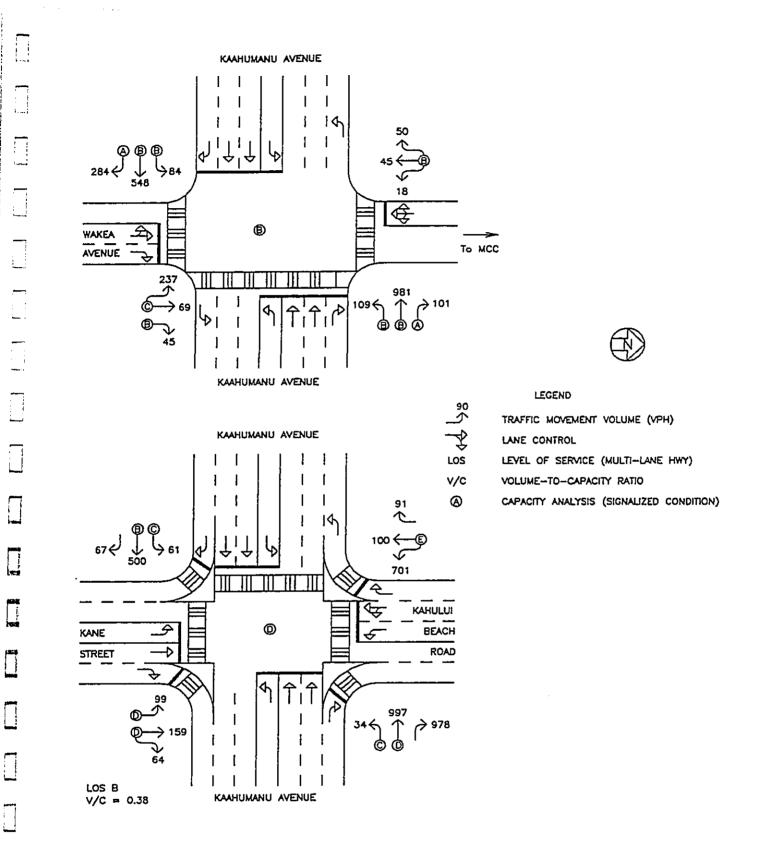


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TRAFFIC IMPACT REPORT MAUI COMMUNITY COLLEGE YEAR 2000 AM PEAK HOUR WITHOUT PROJECT

**EXHIBIT** 

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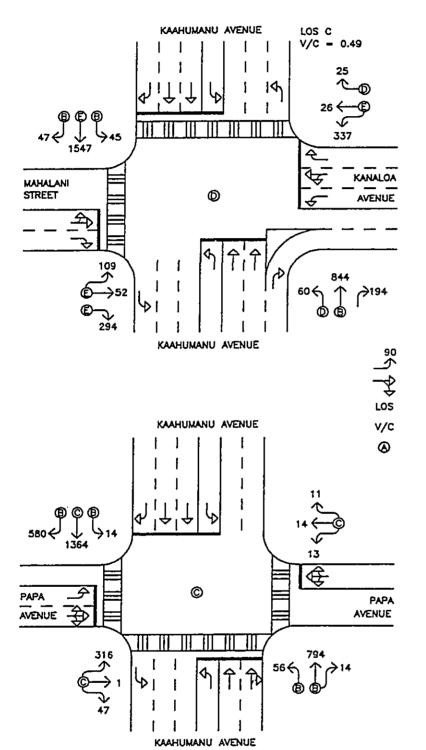
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TRAFFIC IMPACT REPORT

MAUI COMMUNITY COLLEGE

YEAR 2000 AM PEAK HOUR
WITHOUT PROJECT

EXHIBIT





LEGEND

TRAFFIC MOVEMENT VOLUME (VPH)

LANE CONTROL

LEVEL OF SERVICE (MULTI-LANE HWY)

VOLUME-TO-CAPACITY RATIO

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

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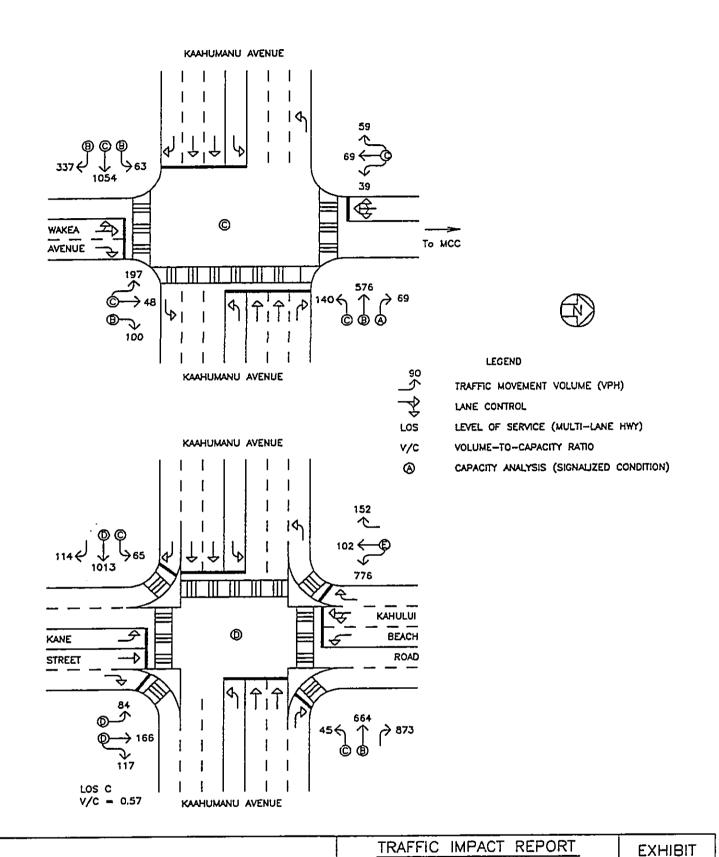
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MAUI COMMUNITY COLLEGE

YEAR 2000 PM PEAK HOUR

WITHOUT PROJECT

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TRAFFIC IMPACT REPORT
MAUI COMMUNITY COLLEGE
YEAR 2000 PM. PEAK HOUR
WITHOUT PROJECT

TABLE 2. YEAR 2000 AM PEAK HOUR INTERSECTION V/C RATIO AND OPERATING LOS WITHOUT THE PROPOSED PROJECT

KAAHUMANU AVENUE	V/C	LOS
MAHALANI STREET/ KANALOA AVENUE	0.805	D
SOUTH PAPA AVENUE	0.772	В
WAKEA AVENUE	0.712	В
KAHULUI BEACH ROAD/ KANE STREET	0.907	D

TABLE 3. YEAR 2000 PM PEAK HOUR INTERSECTION V/C RATIO AND OPERATING LOS WITHOUT THE PROPOSED PROJECT

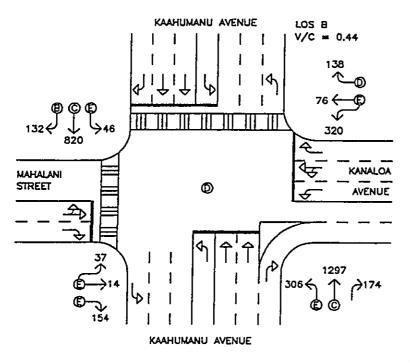
KAAHUMANU AVENUE	V/C	LOS
MAHALANI STREET/ KANALOA AVENUE	0.955	D
SOUTH PAPA AVENUE	0.748	С
WAKEA AVENUE	0.791	В
KAHULUI BEACH ROAD/ KANE STREET	0.891	D

Mahalani Street/Kanaloa Avenue, would operate at LOS "B" and a v/c ratio of 0.44. Kaahumanu Avenue, east of Kahului Beach Road, is expected to carry 3,274 vph, 1,265 vph east bound and 2,009 vph west bound. East bound traffic on Kaahumanu Avenue, east of Kahului Beach Road/Kane Street, would operate at LOS "B" and at a v/c ratio of 0.38 during the projected AM peak hour of traffic.

During the projected PM peak hour of traffic, Kaahumanu Avenue, west of Mahalani Street/Kanaloa Avenue, is expected to carry 2,617 vph; 1,639 vph east bound and 978 vph west bound. East bound traffic on Kaahumanu Avenue, west of Mahalani Street/Kanaloa Avenue, would operate at LOS "C" and at a v/c ratio of 0.49. East of Kahului Beach Road/Kane Street, Kaahumanu Avenue is expected to carry 3,488 vph during the projected PM peak hour of traffic; 1,906 vph east bound and 1,582 vph west bound. Kaahumanu Avenue, east of Kahului Beach Road/Kane Street, would operate at LOS "C", and at a v/c ratio of 0.57 for east bound traffic during the projected PM peak hour.

# D. Total Traffic With Project

Exhibits 11 through 14 show the Year 2000 traffic projections with the proposed MCC expansion. The intersection capacity analysis is shown under existing geometric and signalized conditions. Tables 4 and 5 show the overall intersection LOS at the intersections in the study area for the projected AM and PM peak hours of traffic, respectively. The traffic impact analysis of the site-generated traffic is discussed in the next section.





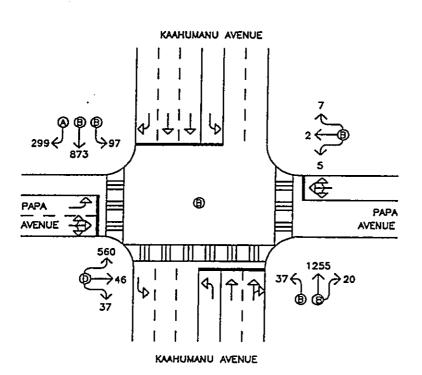
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TRAFFIC MOVEMENT VOLUME (VPH)

LEGEND

LEVEL OF SERVICE (MULTI-LANE HWY)
VOLUME-TO-CAPACITY RATIO

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

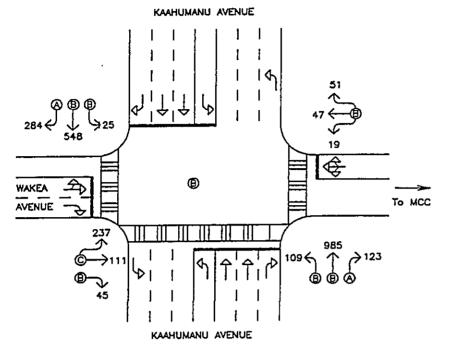


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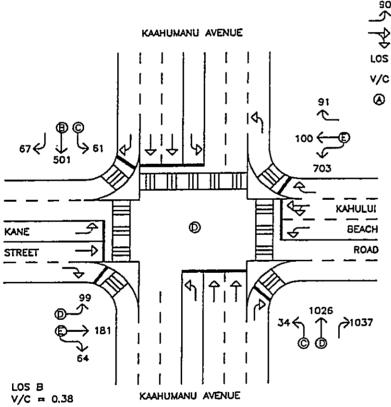
TRAFFIC IMPACT REPORT
MAUI COMMUNITY COLLEGE
YEAR 2000 AM PEAK HOUR
WITH PROJECT

11

**EXHIBIT** 







KAAHUMANU AVENUE

TRAFFIC MOVEMENT VOLUME (VPH) LANE CONTROL LEVEL OF SERVICE (MULTI-LANE HWY)

VOLUME-TO-CAPACITY RATIO

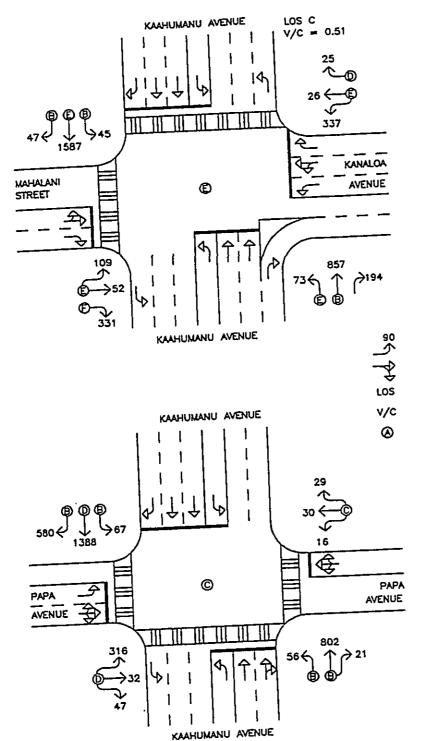
LEGEND

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

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TRAFFIC IMPACT REPORT MAUI COMMUNITY COLLEGE YEAR 2000 AM PEAK HOUR WITH PROJECT

**EXHIBIT** 





LEGEND

TRAFFIC MOVEMENT VOLUME (VPH)
LANE CONTROL

LEVEL OF SERVICE (MULTI-LANE HWY)

VOLUME-TO-CAPACITY RATIO

CAPACITY ANALYSIS (SIGNALIZED CONDITION)

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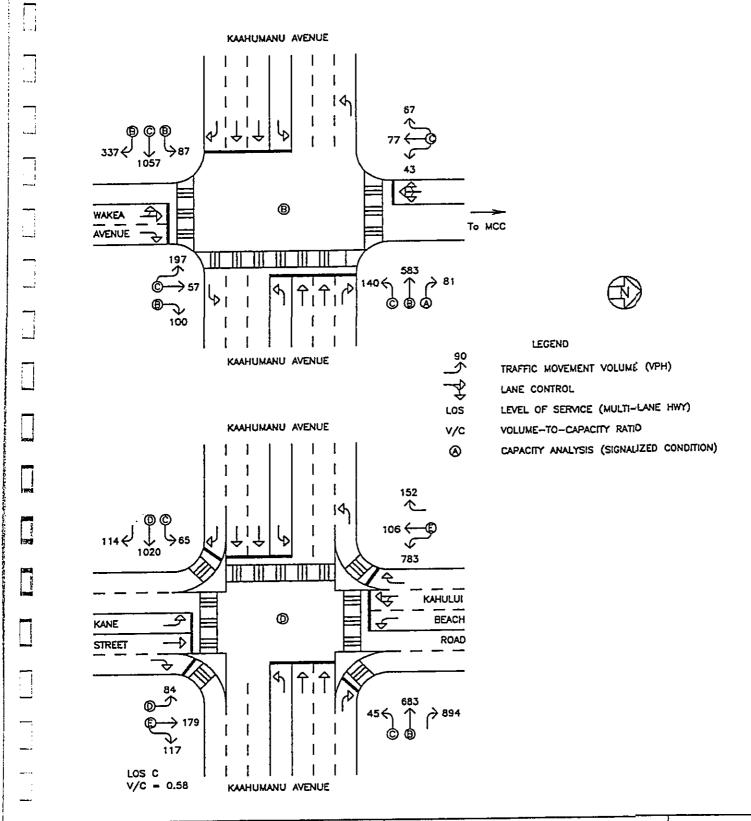
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MAUI COMMUNITY COLLEGE

YEAR 2000 PM PEAK HOUR
WITH PROJECT

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**EXHIBIT** 



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HONOLULU, HAWAII

TRAFFIC IMPACT REPORT

MAUI COMMUNITY COLLEGE

YEAR 2000 PM PEAK HOUR
WITH PROJECT

TRAFFIC IMPACT REPORT

MAUI COMMUNITY COLLEGE

YEAR 2000 PM PEAK HOUR

WITH PROJECT

TABLE 4. YEAR 2000 AM PEAK HOUR INTERSECTION V/C RATIO AND OPERATING LOS WITH THE PROPOSED PROJECT

KAAHUMANU AVENUE	V/C	LOS
MAHALANI STREET/ KANALOA AVENUE	0.824	D
SOUTH PAPA AVENUE	0.860	В
WAKEA AVENUE	0.763	В
KAHULUI BEACH ROAD/ KANE STREET	0.922	D

TABLE 5. YEAR 2000 PM PEAK HOUR INTERSECTION V/C RATIO AND OPERATING LOS WITH THE PROPOSED PROJECT

KAAHUMANU AVENUE	V/C	LOS
MAHALANI STREET/ KANALOA AVENUE	1.038	E
SOUTH PAPA AVENUE	0.790	С
WAKEA AVENUE	0.839	В
KAHULUI BEACH ROAD/ KANE STREET	0.923	D

#### V. TRAFFIC IMPACT ANALYSIS

#### A. AM Peak Hour Traffic With Project

Kaahumanu Avenue, west of Kanaloa Avenue/Mahalani Street, would carry 2,470 vph during the projected AM peak hour with the proposed project; 1,472 vph west bound and 998 vph east bound. West bound traffic on Kaahumanu Avenue would operate at LOS "B" and at a v/c ratio of 0.44 during the projected AM peak hour of traffic.

The left-turn and through movements on the Kanaloa Avenue approach at Kaahumanu Avenue would operate at LOS "E" during the projected AM peak hour of traffic. The other left-turn movements at the intersection of Kaahumanu Avenue and Mahalani Street/Kanaloa Avenue would also operate at LOS "E". the intersection would continue to operate at an overall LOS "D" during the projected AM peak hour of traffic. The total number of vehicles entering this intersection would increase by approximately 4% over projected traffic demands without the proposed project.

At the intersection of Kaahumanu Avenue and South Papa Avenue, the north bound approach of South Papa Avenue would operate at LOS "D" during the projected AM peak hour of traffic. The other traffic movements at this intersection would continue to operate at satisfactory LOS, similar to projected traffic conditions without the proposed project. The intersection would operate at an overall LOS "B" during the projected AM peak hour of traffic.

The Wakea Avenue north bound left/through movement would operate at LOS "C" at the intersection with Kaahumanu Avenue during the projected AM peak hour of traffic with the proposed project. The other movements at the intersection of Kaahumanu Avenue and Wakea Avenue would operate at satisfactory LOS. The intersection would operate at an overall LOS "B" during the projected AM peak hour of traffic. Vehicular traffic entering the

MCC main entrance at the intersection of Kaahumanu Avenue and Wakea Avenue is expected to increase by 105 vph during the projected AM peak hour of traffic with the proposed project.

The north bound approach of Kane Street at the intersection with Kaahumanu Avenue would operate at LOS "E" during the projected AM peak hour of traffic. The south bound left-turn movement on Kahului Beach Road to east bound Kaahumanu Avenue would operate at LOS "E". The west bound approach on Kaahumanu Avenue would operate at LOS "D". The other traffic movements at the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street would operate at satisfactory LOS. The intersection would operate at an overall LOS "D" during the projected AM peak hour of traffic.

Kaahumanu Avenue, east of Kahului Beach Road, would carry 3,365 vph during the projected AM peak hour; 1,268 vph east bound and 2,097 vph west bound. East bound traffic on Kaahumanu Avenue, east of Kahului Beach Road, would operate at LOS "B" and at a v/c ratio of 0.38 during the AM peak hour of traffic. The total number of vehicles entering the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street is expected to increase by approximately 3% over projected conditions without the proposed project.

# B. PM Peak Hour Traffic With Project

Kaahumanu Avenue, west of Kanaloa Avenue/Mahalani Street, would carry 2,670 vph during the projected PM peak hour with the proposed project; 991 vph west bound and 1,679 vph east bound. East bound traffic on Kaahumanu Avenue would operate at LOS "B" and at a v/c ratio of 0.51 during the projected PM peak hour of traffic.

The north bound right-turn movement from Mahalani Street to east bound Kaahumanu Avenue would operate at LOS "F" during the projected

PM peak hour of traffic with the proposed project. The left-turn and through movements on the Kanaloa Avenue approach at Kaahumanu Avenue would continue to operate at LOS "E". The Kaahumanu Avenue west bound left-turn movement to south bound Mahalani Street also would operate at LOS "E". The intersection would operate at an overall LOS "E" during the projected PM peak hour of traffic. The total number of vehicles entering this intersection would increase by approximately 3% over projected traffic demands without the proposed project.

At the intersection of Kaahumanu Avenue and South Papa Avenue, the north bound approach of South Papa Avenue would operate at LOS "D" during the projected PM peak hour of traffic. The other traffic movements at this intersection would continue to operate at satisfactory LOS, similar to projected traffic conditions without the proposed project. The intersection would operate at an overall LOS "B" during the projected PM peak hour of traffic.

The north bound Wakea Avenue left/through movement would operate at LOS "C" at the intersection with Kaahumanu Avenue during the projected PM peak hour of traffic with the proposed project. The south bound approach from MCC would continue to operate at LOS "C". The other traffic movements at the intersection of Kaahumanu Avenue and Wakea Avenue also would operate at satisfactory LOS. The intersection operates at an overall LOS "B" during the projected PM peak hour of traffic. Vehicular traffic entering the MCC main entrance at the intersection of Kaahumanu Avenue and Wakea Avenue is expected to increase by 45 vehicles during the projected PM peak hour of traffic with the proposed project.

The north bound approach of Kane Street at the intersection with Kaahumanu Avenue would operate at LOS "E" during the projected PM peak hour of traffic. The south bound left-turn movement on Kahului Beach Road to east bound Kaahumanu Avenue also would operate at LOS "E". The

other traffic movements at the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street would operate at satisfactory LOS. The intersection would continue to operate at an overall LOS "D" during the projected AM peak hour of traffic.

Kaahumanu Avenue, east of Kahului Beach Road, would carry 3,542 vph during the projected PM peak hour; 1,920 vph east bound and 728 vph west bound. East bound traffic on Kaahumanu Avenue, east of Kahului Beach Road, would operate at LOS "C" and at a v/c ratio of 0.58 during the PM peak hour of traffic. The total number of vehicles entering the intersection of Kaahumanu Avenue and Kahului Beach Road/Kane Street is expected to increase by approximately 2% over projected conditions without the proposed project.

# VI. PROPOSED ROADWAY IMPROVEMENTS AND RECOMMENDATIONS

# A. Improvements to Mitigate Existing Roadway Deficiencies

- Provide exclusive left-turn, through, and right-turn lanes on the north bound Mahalani Street approach at Kaahumanu Avenue.
- 2. Construct an exclusive left-turn lane, an optional left/through lane, and an exclusive right-turn lane on the north bound approach of Wakea Avenue at Kaahumanu Avenue.

#### B. Site Access Recommendations

- Restrict access from Kaihee Place and utilize the new road connecting Kanaloa Avenue to Kahului Beach Road as the access to the faculty/student parking lot located north of the campus.
- Provide an exclusive right-turn lane and an optional left/through lane on the south bound approach exiting the MCC campus at the intersection of Kaahumanu Avenue and South Papa Avenue.

- Provide an exclusive right-turn lane and an optional left/through lane on the south bound approach exiting the MCC campus at the intersection of Kaahumanu Avenue and Wakea Avenue.
- 4. Each of the campus parking lot driveways on the South Papa Avenue Extension should have exclusive left-turn and right-turn lanes for vehicles exiting the parking areas.
- Left-turn lanes to the parking lot driveways should be constructed to maintain through traffic flow on the new roadway and the South Papa Avenue Extension.

#### VII. CONCLUSION

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Based upon the "Wailuku-Kahului Plan", the proposed roadway improvements and new roadways included in the development plans of other projects in the Kahului-Wailuku area are expected to 2 reduce the traffic demand on Kaahumanu Avenue. The projected AM and PM peak hour traffic volumes on Kaahumanu Avenue, west of Mahalani Street/Kanaloa Avenue, with the proposed MCC expansion, would increase by 3% and 2%, respectively, over projected traffic conditions without the proposed project. East of Kahului Beach Road/Kane Street, the AM and PM peak hour traffic projections on Kaahumanu Avenue, with the proposed MCC expansion, would each increase by approximately 2% over projected traffic conditions without the proposed project.

With the implementation of roadway improvements and new roadways discussed in this report, the proposed Maui Community College expansion should not have any significant impact on traffic in the vicinity of the project.

# APPENDICES

# APPENDIX A LEVEL OF SERVICE DEFINITIONS

ATA

# LEVEL OF SERVICE DEFINITIONS

# 1. LEVEL-OF-SERVICE CRITERIA FOR MULTILANE HIGHWAY

Level of Service (LOS) criteria for multilane highways are defined in terms of density. Density is a measure which quantifies the proximity to other vehicles in the traffic stream. It expresses the degree of maneuverability within the traffic stream.

**Level of service** criteria depend on the design speed of the highway element being studied. A "highway element" can be an isolated geometric element, such as a curve or grade having a reduced design speed, or a series of such geometric elements that dominate the operation of a longer segment of highway.

Level of Service A describes completely free-flow conditions. The operation of vehicles is virtually unaffected by the presence of other vehicles, and operations are constrained only by the geometric features of the highway and driver preferences. Vehicles are spaced at an average of 440 feet, or 22 car-lengths, at a maximum density of 12 pc/mi/ln. The ability to maneuver within the traffic stream is high. Minor disruptions to flow are easily absorbed at this level without causing significant delays or queuing.

**Level of Service B** is also indicative of free flow, although the presence of other vehicles begins to be noticeable. Average travel speeds are somewhat diminished from LOS A. Vehicles are spaced at an average of approximately 264 feet, or 13 car-lengths, at a maximum density of 20 pc/mi/ln. Minor disruptions are still easily absorbed at this level, although local deterioration in LOS will be more obvious.

Level of Service C represents a range in which the influence of traffic density on operations becomes marked. The ability to maneuver within the traffic stream, and to select an operating speed, is now clearly affected by the presence of other vehicles. The average spacing of vehicles is reduced to approximately 175 feet, or 9 car-lengths, at a maximum density of 30 pc/mi/ln. Minor disruptions may be expected to cause serious local deterioration in service, and queues may form behind any significant traffic disruption. Severe or long-term disruptions may cause the facility to operate at LOS F.

**Level of Service D** borders on unstable flow. Speeds and ability to maneuver are severely restricted because of traffic congestion. The average spacing of vehicles is 125 feet, or 6 car-lengths, at a maximum density of 42 pc/mi/ln. Only the most

minor of disruptions can be absorbed without the formation of extensive queues and the deterioration of service to LOS F.

Level of Service E represents operations at or near capacity, and is quite unstable. At capacity, vehicles are spaced at only 80 feet, or 4 car-lengths, at a maximum density of 67 pc/mi/ln. This is the minimum spacing at which uniform flow can be maintained, and effectively defines a traffic stream with no usable gaps. Thus, disruptions cannot be damped or dissipated, and any disruption, no matter how minor, will cause queues to form and service to deteriorate to LOS F.

Level of Service F represents forced or breakdown flow. It occurs at a point where vehicles arrive either at a rate greater than that at which they are discharged, or at a point on a planned facility where forecasted demand exceeds the computed capacity. While operations at such points (and on immediately downstream sections) will appear to be at capacity or better, queues will form behind these breakdowns. Operations within queues are highly unstable, with vehicles experiencing short spurts of movement followed by stoppages. Densities are higher than 67 pc/mi/ln. Note that the term "LOS F" may be used to characterize both the point of the breakdown and the operating conditions within the queue. It must be remembered, however, that it is the point of breakdown that causes the queue to form, and that operations within the queue are generally not related to defects along the highway segment over which the queue extends.

# 2. LEVELS OF SERVICE CRITERIA FOR TWO-LANE HIGHWAYS

The highest quality of traffic service occurs when motorists are able to drive at their desired speed, representative of Level of Service A. Almost no platoons of three or more vehicles are observed. Drivers would be delayed no more than 30 percent of the time by slow-moving vehicles. A maximum flow rate of 420 pcph, total in both directions, may be achieved under ideal conditions.

Level of Service B characterizes the region of traffic flow where drivers are delayed up to 45 percent of the time on the average. Service flow rates of 750 pcph, total in both directions, can be achieved under ideal conditions. Above this flow rate, the number of platoons forming in the traffic stream begins to increase dramatically.

Further increases in flow characterize Level of Service C, resulting in noticeable increases in platoon formation, platoon size, and frequency of passing impediment. At high volume levels, chaining of platoons and significant reductions in passing capacity begin to occur. While traffic flow is stable, it is becoming susceptible to congestion due to turning traffic and slow-moving vehicles. Percent time delays

are up to 60 percent. A service flow rate of up to 1,200 pcph, total in both directions, can be accommodated under ideal conditions.

Unstable traffic flow is approached as traffic flows enter Level of Service D. The two opposing traffic streams essentially begin to operate separately at higher volume levels. Mean platoon sizes of 5 to 10 vehicles are common, although speeds of 50 mph can still be maintained under ideal conditions. The fraction of no passing zones along the roadway section usually has little influence on passing. Turning vehicles and/or roadside distractions cause major shockwaves in the traffic stream. The percentage of time motorists are delayed approaches 75 percent. Maximum service flow rates of 1,800 pcph, total in both directions, can be maintained under ideal conditions. This is the highest flow rate that can be maintained for any length of time over an extended section of level terrain without a high probability of breakdown.

Level of Service E is defined as traffic flow conditions on two-lane highways having a percent time delay of greater than 75 percent. Passing is virtually impossible under Level of Service E conditions, and platooning becomes intense when slower vehicles or other interruptions are encountered.

The highest volume attainable under Level of Service E defines the capacity of the highway. Under ideal conditions, capacity is 2,800 pcph, total in both directions. Operating conditions at capacity are unstable and difficult to predict. Traffic operations are seldom observed near capacity on rural highways, primarily because of a lack of demand.

As with other highway types, Level of Service F represents heavily congested flow with traffic demand exceeding capacity. Volumes are lower than capacity. Level of Service E is seldom attained over extended sections on level terrain as more than a transient condition; most often, perturbations in traffic flow as Level E is approached cause a rapid transition to Level of Service F.

# 3. LEVEL OF SERVICE OF SIGNALIZED INTERSECTIONS

Level of service for signalized intersections is defined in terms of delay. Delay is a measure of driver discomfort, frustration, fuel consumption and lost travel time. specifically, level-of-service criteria are stated in terms of the average stopped delay per vehicle for a 15-minute analysis period. The criteria are given in Table A-1.

Table A-1. Level-of Service Criteria for Signalized Intersections

Level of Service	Stopped Delay for Vehicle (SEC)
A B C D E F	5.0 5.1 to 15.0 15.1 to 25.0 25.1 to 40.0 40.1 to 60.0 60.0

Delay is a complex measure, and is dependent on a number of variables, including the quality of progression, the cycle length, the green ratio, and the v/c ratio for the lane group or approach in question.

**Level-of-service** A describes operations with very low delay, i.e., less than 5.0 seconds per vehicle. This occurs when progression is extremely favorable, and most vehicles arrive during the green phase. Most vehicles do not stop at all. Short cycle lengths may also contribute to low delay.

**Level-of-service B** describes operations with delay in the range of 5.1 to 15.0 seconds per vehicle. This generally occurs with good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.

**Level-of-service C** describes operations with delay in the range of 15.1 to 25.0 seconds per vehicle. These higher delays may result from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear in this level. The number of vehicles stopping is significant at this level, although many still pass through the intersection without stopping.

**Level-of-service D** describes operations with delay in the range of 25.1 to 40.0 seconds per vehicle. At level D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high v/c ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.

**Level-of-service** E describes operations with delay in the range of 40.1 to 60.0 seconds per vehicle. This is considered to be the limit of acceptable delay. These high delay values generally indicate poor progression, long cycle lengths and high v/c ratios. Individual cycle failures are frequent occurrences.

**Level-of-service** F describes operations with delay in excess of 60.0 seconds per vehicle. This is considered to be unacceptable to most drivers. This condition often occurs with oversaturation, i.e., when arrival flow rates exceed the capacity of the intersection. It may also occur at high v/c ratios below 1.00 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing causes to such delay levels.

### 4. LEVEL OF SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS

Level of Service definitions for unsignalized intersections is determined by the reserve or unused capacity of a lane. The potential capacity is determined by the size and frequency in gaps in conflicting traffic that can accommodate the side street demand. The reserve capacity is equal to the potential capacity minus the traffic demand. A lower Level of Service translates into longer side street delay. The Levels of Service criteria are shown in the following table:

Table A-2. Level-of-Service Criteria for Unsignalized Intersections

Reserve Capacity	Level of Service	Expected Delay to Minor Street Traffic
≥ 400	Α	Little or no delay
300-399	8	Short traffic delays
200-299	C	Average traffic delays
100-199	O	Long traffic delays
0- 9	ш	Very long traffic delays
< 0	H.	Extreme traffic delays

# A P P E N D I X B EXISTING TRAFFIC COUNT DATA

SITE CODE : Hajor St. : : Hinor St. : : Weather : :	KAAHUHAHU A		NALOA AVS.			•	LLEGS EXPA s: Vehicle						PAGS: 1 ZILE: XARHAHAM DATS: 9/30/92
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6:45	15	20	34	39	231	71	14	3	7	35	: 46	:0	620
HR TOTAL	31	25	60	72	363	114	18	6	13	55	235	13	1010
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7:30	. 22	21	99	46	377	89	25	3	11	45	233	:€	978
7:45	45	26	66	45	379	83	25	6	5	45	244	15	984
IR TOTAL	124	63	316	183	1403	282	87	15	38	138	844	48	3541
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#### PEAK PERIOD ANALYSIS FOR THE PERIOD: 6:30 AM - 8:30 AM

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North	7:15 AM	0.92	135	74	312	521	26	14	60
East	7:15 AX	0.93	197	1460	345	2003	16	73	17
South	7:30 AM	0.74	110	23	57	190	58	12	30
West	7:15 AM	<b>e.85</b>	147	839	51	1037	14	81	•
			Entire	Inters	ection				
North	7:15 AX	0.92	135	74	312	521	25	14	69
<b>East</b>		0.99	197	1460	346	2003	10	73	17
South		0.75	98	16	41	153	63	10	26
Yest		0.85	147	839	51	1037	14	81	5

#### MADI COMMUNITY COLLEGE EIFAMSION

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East	3:45 PM	0.90	223	948	99	1270	18	75	8
South	4:00 PX	0.81	315	65	139	519	6:	13	27
West	4:15 PM	0.71	53	1727	50	1830	3	94	3
		•	Entire	Inters	ection				
North	4:15 PK	0.86	24	25	329	378	ş	7	87
Rast		0.88	219	953	69	1240	16	37	5
South		0.79	323	58	121	507	€₹	::	24
West		0.71	53	1727	59	1839	3	64	3

1.24	COMMUNICAL	COLLEGE	EXPLYSION

SITE CODE : 1
Major St. : KAAHUHANU AVZNUS
Hinor St. : PAPA AVENUE / K.C.C
Weather : DRY, SUNRY

Primary Movements: Vehicles

PAGE: 1 FILE: KAAPAPPX DATE: 9/29/91

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4:15	2	5	2	7	236	7	12	1	99	121	348	5	845
4:30	3	4	0	2	265	21	15	ð	105	193	115	3	1945
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South	3:45 PM	0.93	55	Ļ	428	487	::	1	85
West	4:15 PM	9.88	653	1572	17	2747	29	70	:
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North	4:00 PH	0.79	11	14	13	38	29	37	3;
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South		0.95	62	1	415	473	13	Ş	27
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Minor St.	: KAABUHARU	EUE													PAGE: K
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	North	3:45	PH	0.71 0.89		61	71	44	176		35	40	25		
	East South	3:30	אם שם	0.89 0.88		43	771	156 286	980		4 28	73	17		
		4:15	5ñ	0.99		392	1221	51	1674		23				
	West					7	ntare	ction							
	Nest	-			Ent	ile 1									
			DV	a 67											
	North	4:00	PX	0.67 6.93		59	63	39	157		35				
		4:00	PX	0.67 0.93 0.84		59	63		157 929 439		7	41 74 11	23 18 55		

Weather	: KANZ STREE : DRY, SUNNY				Primar	y Movement	s: Vehic	les						FILE: KAAEANEN DATE: 9/29/92
Time	;	ion Sc	uth		roz Wes	5 <b>t</b>		From S:	orti		 ?	 roz Iss	 :	
Beg10 					TERU			T TER				THRU		fotal
3:30	33	44	23		269			€ 27				219	 :s	1142
3:45		47			243	13	3	5 27	:33		209		29	1935
IR TOTAL	58	91	48	81	512	<b>42</b>		72 54	377		438		33	2237
4:00 PM	36	39	30	23	263	16	4	5 36	. 31		203	176	13	10.00
4:15	35	49	16		299	14	;		197		275	• • •	13	1963 1155
4:30	23	46	26		298	22	4		225		253		:: ::	1200
4:45	33	47	20	33		22		28			231		8	1123
IR TOTAL	126	151	52	139	1158	74	11	3 105			962		58	4376
5:00 PH	20	41	17	25	259	29	33	39	210		284	147	ç	1032
AY TOTAL	204	313	157	236	1929	144	26	3 159	::::4		1504	1385	 97	7845
			PEAR	PERIOD ANALY	SIS FOR	THE PERI	99: 3:3	0 PX -	8:15 FX					
	FROH	STA: PEAK I	OUR	PSAK ER Pactor	R	Vo	: Left	Total		 Right	ምት ቀክ	taf*		
	South	3:30	 yç	0.96		 129 179		402						
	West	4:15		0.95		132 1154	\$6			32 10		23 6		
	Rorth	4:15	PH	0.95		145 115	323	1054		13		5 75		
	East	3:30	PX	0.92		916 792	69	1777		52		4		
					Snt	ire Inters	ection							
	South	4:00	PK	6.95	:	26 191	32	395		32	45	13		
	äest			0.94		30 1159	74	1362		10	95	:		
	North			0.94	3	58 106	307	1071		15	10	7		
	Zast			0.99	9	52 /32	50	1744		33	42	1		

Location : Maji COM College Makea Avenue
Location : Kaihee PL. S N. Wakea Avenue
Weather : DRY. STREE FILE: AFELCOAR DATE: 9/29/92 Operator : EK --------- CERIEROC -----DAT: TUESDAY ----- 780 -----TIME **?** !! 1.7 an BEGIN \_\_\_\_\_\_ 58 12:30 57 23 37 12:15 46 30 11:10 . 8 273 £: 3: 102 177 26 12:45 20 44 24 1:00 : 1 24 1:15 1:30 11 123 3: 8 87 1:45 11 42 2:00 85 :7 69 2:15 24 2:30 25 ::: 55 46 164 2:45 28 44 29 2:00 21 3:15 33 3:30 33 54 107 24 3:45 24 4:00 #: 21 24 4:15 23 22 4:30 57 35 20 39 19 4:45 30 10 20 5:00 50 29 5:15 34 20 14 5:30 53 128 48 69 15 5:45 37 23 14 6:00 12 34 22 6:15 9 18 6:30 33 55 81 6:45 27 12 14 7:00 :7 13 7:15 11 7:30 27 20 :5 54 7:45 16 3:00 23 13 3:15 15 8:30 29 21 3 23 67 8:45 0 9.00 9:15 0 9:30 ð 0 9:45 10:00 10:15 27 10:30 7.) 23 13:45 50 11 36 : 5 11:00 19 22 11:15 3; 0 13 11:30 174 54 3 3 18 11:45 46 112 3 1453 153 558 895 84 TOTALS 1657 1055 DAY TOTALS 38.4 51.5 65.5 SPLIT :

5:15

121

8.63

11:00

54

68.0

12:00

177

0.45

PEAK HOUR

VOLUNE

F.H.?.

11:00

110

0.50

11:22

: 74

FAGE: :

12:00

279

3.51

MANI COMMUNITY COLLEGE TRAFFIC STUDY

PAGE: 2 FILE: MARICONN

SITE CODE : MAUI COM COLLEGE
Location : RAINEE PL. 9 N. WAKSA AVENUE
Weather : DRT, SUNNY
Operator : ER

DATE: 9/30/92

Operator : :	23 													
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7:15	25		•		46		*		71		,			
7:30	15		7		49		*		£4		•	_	•	-
7:45	21	63	t	2	57	162	•	•	78	225		•		ì
3:60	19		,		43		•		67					
8:15	12		•		34		•		45		•			_
8:30	5		+		37		*		42		-			į
8145	20	55	•	*	49	159	•	•	53	124	•	•		-
9:00	10 14		•		22		•		32 33		,			
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andat c	207		·		498		ŧ		702		•			_
TOTALS	204	504	-		1.0	498				732				_
DAY TOTALS	45 *	204	•		70.9	476				79.6				<b>-</b>
SPLIT 3	29.1		•		10.7		•							
			,		7.10		,		7:15		,			÷
SEWN HORS	7:15		,		7:15		•		288		•			
P.H.F.	79		•		201		•		ð.50		*		,	≱. • }
u u v	9.79		•		3.89				0.70					!

# APPENDIX C

### CAPACITY ANALYSIS CALCULATIONS

EXISTING PEAK HOUR TRAFFIC ANALYSIS

19 **	85 HCM:MULTILAN	NE HIGH	WAYS ******	*****	*****	*****	*****	****
	ANALYSTTIME OF ANALYST DATE OF ANALYST	 SIS	. PGP . 7:15- . 9/29/	8:15 AM 92		MAHALA	NI ST.	
A)	ADJUSTMENT FAC	CTORS						
	PERCENTAGE OF PERCENTAGE OF	BUSES. RECREA	TIONAL	 VEHICLE	0 s 0	(TYPICAI	L - 200	#/HP)
	PEAK HOUR FACT DRIVER POPULATE LANE WIDTH (FT	ror rion fa r)	CTOR	• • • • • • • • • • • • • • • • • • •	1	(WEEKDA	Y/COHMU	rer)
	DISTANCE (FT) TYPE OF MULTII	FROM R	OADWAY :	EDGE	6	URBAN, I	DIVIDED	
В)	DISTANCE (FT)	FROM R	OADWAY :	EDGE	6	URBAN, I	DIVIDED	
В)	DISTANCE (FT) TYPE OF MULTII	FROM R	OADWAY : GHWAY E B	EDGE	6		DIVIDED f	f E
в)	DISTANCE (FT) TYPE OF MULTII  CORRECTION FAC	FROM RLANE HI	OADWAY : GHWAY E	EDGE E R	6 SUB	f W	f P	
·	DISTANCE (FT) TYPE OF MULTII  CORRECTION FAC	FROM RANE HI	OADWAY : GHWAY	EDGE E R	6 SUB	f W	f P	E
	**	FACILITY SECT: ANALYST TIME OF ANALYS DATE OF ANALYS OTHER INFORMAT  A) ADJUSTMENT FACE PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED PEAK HOUR FACE DRIVER POPULAT LANE WIDTH (FI	FACILITY SECTION  ANALYST  TIME OF ANALYSIS  DATE OF ANALYSIS  OTHER INFORMATION  A) ADJUSTMENT FACTORS  PERCENTAGE OF TRUCKS  PERCENTAGE OF BUSES.  PERCENTAGE OF RECREA  DESIGN SPEED (MPH)  PEAK HOUR FACTOR  DRIVER POPULATION FA  LANE WIDTH (FT)	FACILITY SECTION KAAHU ANALYST	FACILITY SECTION KAAHUMANU AVANALYST	FACILITY SECTION KAAHUMANU AVE W. OF ANALYST PGP TIME OF ANALYSIS 7:15-8:15 AM DATE OF ANALYSIS 9/29/92 OTHER INFORMATION KAAMAHA. EXISTING  A) ADJUSTMENT FACTORS  PERCENTAGE OF TRUCKS Ø PERCENTAGE OF BUSES Ø PERCENTAGE OF RECREATIONAL VEHICLES Ø DESIGN SPEED (MPH) 50 PEAK HOUR FACTOR 1 DRIVER POPULATION FACTOR 1	ANALYST	FACILITY SECTION KAAHUMANU AVE W. OF MAHALANI ST. ANALYST PGP TIME OF ANALYSIS 7:15-8:15 AM DATE OF ANALYSIS 9/29/92 OTHER INFORMATION KAAMAHA. EXISTING  A) ADJUSTMENT FACTORS  PERCENTAGE OF TRUCKS Ø (TYPICAL - 200 PERCENTAGE OF BUSES Ø PERCENTAGE OF RECREATIONAL VEHICLES. Ø DESIGN SPEED (MPH) 50 PEAK HOUR FACTOR

1985 HCM: MULTIL	ANE HIG	HWAYS	*****	*****	*****	*****	*****
FACILITY SECTION ANALYST TIME OF ANALYDATE OF ANALY OTHER INFORMA	KSIS	PGP 7:15 9/29	5-8:15 0/92		ANE ST		
A) ADJUSTMENT FA	CTORS						
PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED PEAK HOUR FAC	BUSES. RECREA (MPH)	TIONAL	VEHICL	0 Es 0	(TYPIC#	L - 200	7 #/HP)
DRIVER POPULA LANE WIDTH (F OBSTRUCTIONS. DISTANCE (FT) TYPE OF MULTI	TION FA T) FROM R LANE HI	CTOR	······································	1 11 NO		Y/COMML	•
TERRAIN TYPE	E T	E B	E R	f HV	f w	 f p	 f E
LEVEL	1.7	1.5	1.6	1.00		1.00	 0.90
C) OPERATIONAL AN	ALYSIS	RESULT	S ****				
NO. OF LANES INPUT VOLUME V/C RATIO LEVEL OF SERVI COMPUTED CAPAC SPEED (mph) DENSITY (pcpmp	CE	php1).	. 1370 41 . B . 785				

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1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION...KAAHUMANU AVENUE/(MAHALANI RD. / KANALOA AVE.)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....09/30/92
TIME.....7:15 - 8:15 AM
COMMENT.....1992 EXISTING TRAFFIC VOLUMES (KAAMAHAM)
______
                                         GEOMETRY
          VOLUMES
                                                NB
                          EB
11.0 L
11.0 T
11.0 T
                                      WB
          WB NB
                    SB :
                                                           10.5
                                                 10.5
                                       11.0
                                            LT
                   312 : L
                                                     LT
         346
               41
                                                           10.5
LT
     51
                                                 10.5
                                       11.0 R
                   74 : T
135 : T
               16
       1460
                                                           10.5
    839
TH
                                                 12.0
                                                      R
                                       11.0
         ø
               98
                                                           12.0
                   60 : R
    147
RT
                                                 12.0
                                       10.5
                            10.5
               65
     30
           Ø
                                                           12.0
                                                 12.0
RR
                                       12.0
                             12.0
                                                 12.0
                                      12.0
                            12.0
ADJUSTMENT FACTORS
                                                     ARR. TYPE
                                            PED. BUT.
                  ADJ PKG BUSES PHF PEDS
           n√
(%)
2
            ΗV
     GRADE
                                            Y/N min T
                 Y/N Nm Nb
     (%)
                                                 15.8
                                             Y
                                        15
                  N 0
N 0
                                0.86
      0.00
            2.00
                                       Ø
EB
                                                 15.8
                 N 0
N 0
N 0
                                0.85
           2.00
                                        0
      0.00
WB
                                                  21.8
                                             И
                           Ø
                                    0
20
                                0.95
            1.00
      0.00
                                            Y
ИВ
                                                 21.8
                           0 0.84
            1.00
      0.00
SB
                                           CYCLE LENGTH = 180.0
                      SIGNAL SETTINGS
                                          PH-1 PH-2 PH-3 PH-4
                     PH-3 PH-4
               PH-2
        PH-1
                                                 X
                                 NB LT
                      \mathbf{x}
EB LT
          X
                                     TH
    TH
                                     RT
    RT
                                     PD
    PD
                                 SB LT
                      Х
WB LT
                                     TH
    TH
                                           X
                                     RT
    RT
                                     PD
                                                      0.0
                                                            0.0
    PD
                                                18.0
                                          27.0
                                 GREEN
                          0.0
         5.0 15.0
3.0 (3.0
                    94.0
                                                            0.0
 GREEN
                                                      0.0
                                               5.0
                    5.0 0.0
                                 AETFOM
                                          5.0
 YELLOW
                     LEVEL OF SERVICE
                     -----
                           DELAY LOS
                                                       APP. LOS
                                            APP. DELAY
     LANE GRP.
              V/C
                                              20.4
                             52.8
                                      E
                    0.561
              0.732
     L
                              19.3
                                       C
                     0.533
              0.617
        Т
                                       В
                              14.2
                     0.533.
              0.198
                                                           Đ
                                              27.7
                                       E
                     0.678
                              48.6
              0.925
 WB
                                       C
                     0.633
                              23.0
              0.915
                                                           E
        Т
                                              48.0
                                       E
                              48.4
                     0.111
              0.365
        LT
                                       Ε
                              47.4
                     0.111
              0.243
        R
                                              80.0
                              89.7
                     0.161
              0.951
 SB
                             84.2
44.8
                     0.161
        LT
              0.989
                                      Ε
                    0.161
              0.433
        R
                            -------
 INTERSECTION: Delay = 33.1 (sec/veh) V/C = 0.857
                                                    Los = D
```

ANA DA! TII	ALYST TE ME	. <b></b> .		- 8:1	5 AM ING	TRAFF	IC VO	LUME	:S (1	KAAPAE	(11A				
		v	OLUMES					- <b></b> -			OMETR				-
	E		B NI		SB :		EB			WB	OHLIA	NB		SB	
LT TH	934		5 736 6 6		5 : 0 :	L	11.0			11.0	L	11.0		11.6	
ìT	336		8 48	_	4:	T	11.0		'R	11.0 12.0	LTR			12.0	
R	90		6 10		<i>ø</i> :		10.5		I.	12.0		12.0 12.0		12.6	
					:		12.0			12.0		12.0		12.0	
					:		12.0			12.0		12.0		12.0	
	<b>-</b>					ADJUS	 TMEN	 T FA							•
		ADE	HV	ADJ	PKG	BUSI	ES	PHF		EDS	PED	. BUT.	ARR	. TYPE	,
_		<del>ኔ</del> )	(%)	Y/N	Nm	N			_		Y/N	min !			•
B B		.00 .00	2.00 2.00	N	0	9		0.85		Ø	N	10.9	<del>)</del>	5	
В		.00	1.00	N N		6		0.99 0.76		20	Y	10.9		5	
В		.00	0.00	N	õ	è		0.70 0.92		10 20	Y Y	22.1 22.1		3 3	
													- 		,
		PH-1	L PH-	2 5	12 3-H°	GNAL PH-		INGS				LE LE		90.0	
В	LT	x	- X	_	11-3	FH-	· • <u>•</u>	NB	LT	PH-	-1 F K	H-2	PH-3	PH-4	
	TH		X						TH		K				
	RT		x						RT	3					•
В	PD LT	x	X X						PD	2					Ī
_	TH	Λ	X					SB	LT TH	2					1
	RT		X						RT	3					•
	PD	_	X						PD	-	•				
REI	FOM FW	5.0		_	0.0	0.		GREE		31.	0	0.0	0.0	0.0	
		3.0	5.0	o 	0.0 	ø. 	ø 	YELI	OM	5.	0	0.0	0.0	0.0	
						LEVEL	OF S	ERVI	CE						
_		GRP.			G/C		DELAY		Los	; A	PP. D	ELAY	APP.	LOS	_
3	I		0.028		.567		6.5		В		7.			В	•
	F		0.776 0.468		.478 .478		8.3 5.0		В						
3	I		0.165		. 567		7.1		A B		32.	3	•	D	-
		'R	1.060	9 0	. 478		33.0		D		J	J	i	J	***
3	L		0.945		.367		40.4		E		48.	6	j	Ξ	
		TR	1.045 0.065		.367 .367		56.0 11.9		E			_			-
	T						77 0		В		11.9	_	I	-	

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1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION..KAAHUMANU AVENUE/(WAKEA AVE. / M.C.C)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....09/30/92
TIME......7:15 - 8:15 AM
COMMENT.....1992 EXISTING TRAFFIC VOLUMES (KAAWAKAM)
GEOMETRY
         VOLUMES
                             EB WB
11.0 L 11.0
11.0 T 11.0
11.0 T 11.0
10.5 R 10.5
                                                 NB
                            EB
          WB NB
                    SB :
                                                  10.5 LTR 10.5
     EB
                    18 : L
        131
              311
     84
                                                            12.0
                                                  10.5
              69
                     45 : T
    615
        1178
                                                            12.0
TH
                                                  12.0
                     50 : T
               59
         101
RT
    319
                                                            12.0
                                                  12.0
               35
                     15 : R
          25
     90
                                                            12.0
                                                  12.0
                                        12.0
                             12.0
12.0
                                                  12.0
                                       12.0
                       ADJUSTMENT FACTORS
ADJ PKG BUSES PHF PEDS
                                             PED. BUT. ARR. TYPE
            ΗV
               GRADE
                          Nb Y/N
0 0.86 15 Y
0 0.90 0 N
0 0.94 15 Y
0 0.71 15 Y
                                                  min T
                                             Y/N
      (%)
            ( % )
                                                  10.6
     0.00 2.00 N
0.00 2.00 N
0.00 1.00 N
                      0
                   N
                                                           5.
EB
                                                   10.6
WB
                                                  24.4
                       0
NB
                                                  24.4
     CYCLE LENGTH = 90.0
                      SIGNAL SETTINGS
                                          PH-1 PH-2 PH-3 PH-4
                    PH-3 PH-4
                                  NB LT
        PH-1
               PH-2
                                           X
         X
               X
EB LT
                                            X
                                     TH
                 X
    TH
                                            X
                                     RT
                 X
    RT
                                     PD
                 Х
    PD
                                  SB LT
                                            X
       · X
WB LT
                                     TH
    TH
                                     RT
                 Х
    RT
                                     PD
                 X
                                                 0.0
                                                             0.0
    PD
                                                       0.0
                                                            0.0
                    0.0
                     0.0 0.0 GREEN
0.0 0.0 YELLOW
                                           32.0
          5.0
               40.0
GREEN
                                                0.0
                                                       0.0
                                           5.0
               5.0
          3.0
                     ______
                      LEVEL OF SERVICE
                                                        APP. LOS
                           DELAY LOS
                                            APP. DELAY
               V/C
     LANE GRP.
                    0.556
                                                5.7
                                       В
                              10.9
              0.462
     L
EB
                                       В
                               5.2
              0.517
                     0.467
       Т
                               5.1
                                       В
                     0.467
              0.444
       R
                                               15.1
                                       В
                     0.556
                               8.7
              0.323
 WB
                                       C
                     0.467
                               16.5
              0.947
        T
                                       Α
                                4.2
              0.139
                     0.467
       R
                                               26.8
                               27.8
              0.884
                     0.378
        LT
 NB
                               11.5
15.4
                     0.378
              0.053
                                               15.4
                                       C
                    0.378
             0.528
                              _____
                Delay = 13.5 (sec/veh) V/C = 0.875 LOS = B
 INTERSECTION:
```

REA NAI ATI	A TYPI Lyst. E	E		92 8:1	5 AM						<b>)</b>			
		vo	LUMES		:					GEOM	ETRY			
	EB	WB		:	SB :		EB		WB			NB	_	SB
ľ	70	38			29 :		11.		11		L	11.0	L	11.0
ŀ	571	1099			24 :		11.		11		TR	11.0 12.0	LT R	11.0 11.0
r	76	0			95 :		11.		11 12			12.0	I.	12.0
R	55	. 0	95		90 :	ĸ	10. 12.		12			12.0		12.0
					:		12.		12			12.0		12.0
							JUSTME	NT FA PHF	CTORS PED:	c	חשמ	BUT.	APP	TYPE
		ADE	HV		PKG	8	USES Nb	Pnr	PED.		/ N	min T	******	
_	-	€ ) OO	(%)	Y/N N	Nm O			0.86	1		Y	16.4		5
B B		.00 .00	2.00 2.00	N	ø		ø	0.84			N	16.4		5
В		.00	1.00	N	ő		Ø	0.95	1		Y	21.8		3
В		.00	2.00	N	0		0	0.84	1	5	Y	21.8		3
						 		TTYCS			CYCI	E LEN	 стн =	140.0
			nu-	,	s PH-3		AL SET PH-4	TINGS		PH-1			PH-3	PH-4
-	T 171	PH-1 X	PH-:	2	rn-3		E11-4	NB	LT			X		
В	LT TH	Λ	x						TH			X		
	RT		X						RT			X		
	PD		X						PD					
В	LT	X	X					SB	LT	X				
	TH		X						TH	X				
	RT		X						RT PD	x				
	PD		X = 7 X	a	0.0		0.0	GRE		40.0	20	0.0	0.0	0.0
RE	EN LOW	5.0 3.0			0.0		0.0	YEL		5.0		5.0	0.0	0.0
				<b>-</b> -										
					~ . ~	LE	VEL OF		ICE LOS	2 5	P. DI	T.AY	APP	. Los
_		E GRP.	. V/C	0	G/C 0.47	a	DEL	.9	D		11.		*	В
В		L T	0.59 0.53		0.42			.3	В		_ = -			
		R	0.04		0.42			.3	В					
В		L	0.11		0.47			. 3	C		28.6	5		D
_		T	0.99	3	0.42	1		. 1	D			_		-
В		Ĺ	0.48	7	0.15			. 2	E		42.8	3		E
		TR	0.74		0.15			1.1	Z		102	2		F
В		L	1.17		0.30		146		F E		103.	<u>c</u>		-
		LT	0.99	1	0.30	Ø	56	. 7	C E					

`\_

juna juna	19 **	85 HCM:MULT	ILANE HI	GHWAYS	*****	*****	*****	******	******
7 m		FACILITY STANALYST TIME OF AND DATE OF AND OTHER INFOR	ALYSIS	4:00	0-5:00 I	PM	F MAHAL	ANI ST.	
2	A)	ADJUSTMENT	FACTORS						
7.		PERCENTAGE PERCENTAGE PERCENTAGE	OF BUSES OF RECRE	S  ATIONAL	VEHTCI.	Ø	(TYPIC)	AL - 200	0 #/HP)
		DESIGN SPEE PEAK HOUR F DRIVER POPU	ACTOR LATION F	ACTOR.	• • • • • • •	1	/ WEIGHT	Чү/сомы	
		OBSTRUCTION:	(FT) S	• • • • • •	• • • • • • •	11	/ WEEKD!	117COMME	TER)
		DISTURCE (E.	TI EROM F	KOADWAY.	EDGE	_			
	•	TYPE OF MUL!	T) FROM F	KOADWAY.	EDGE	_	SURBAN,	DIVIDED	1
	B)	TYPE OF MUL	FACTORS	ROADWAY IGHWAY.	EDGE	f HV	URBAN,  f	DIVIDED	 f E
	B)	TYPE OF MUL	FACTORS	E B	E R	f HV	f W	 f p	 f E
	B)	TYPE OF MUL	FACTORS E T T T T T T T T T T T T T T T T T T	E B 1.5	E R	f HV	f W	 f p	 f E

*****	HIGHW		****	**************************************	, x * * * * * * * * * * * * * * * * * *	******	*****
FACILITY SECTION ANALYST TIME OF ANALYSING OF ANALYSING OTHER INFORMATION		4:00-5: 9/29/92	00 PH		, ,,		
A) ADJUSTMENT FACT	rors				TYPICAL		
PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED (DESIGN SPEED (DESIGN FACTOR FA	RECREATION FACTOR ROME ROME ROME HIS	CTOR		50 1 1 ( 11	WEEKDAY IRBAN, D		ER)
B) CORRECTION THE	 E	 E	E_	f HV	f W	f	f E
TERRAIN TYPE	T	B 	R 				
			_	_			
LEVEL	1.7	1.5	1.6	1.00	o.97	1.00	0.90

en:

Fr.

151

```
1985 HCM: SIGNALIZED INTERSECTIONS
 SUMMARY REPORT
              ************
 INTERSECTION.. KAAHUMANU AVENUE/(MAHALANI RD. / KANALOA AVE.)
 AREA TYPE....CBD
 ANALYST.....PGP
 DATE.....09/29/92
 TIME.....4:15 - 5:15 PM
 COMMENT.....1992 EXISTING TRAFFIC VOLUMES (KAAMAHPM)
 VOLUMES
                                       GEOMETRY
      EB
                    SB :
           WB NB
                            EB
                                     WB NB
11.0 LT 10.
                                                          SB
 LT
                                     11.0 LT
11.0 R
     50
           68
               121
                    329 : L
                            11.0 T 11.0
11.0 T 11.0
10.5 10.5
12.0 12.0
                            11.0 L
                                                    ŗ_
                                                10.5
                                                          10.5
 TH 1727
          953
               58
                    25 : T
                                                10.5
                                                     LT
                                                          10.5
          0
 RT
      53
               328
                    24 : T
                                                12.0
                                                          10.5
 RR
      20
              130
                    15 : R
                            10.5
                                                12.0
                                                          12.0
                      •
                                                12.0
                                                          12.0
                            12.0
                                     12.0
                                               12.0
                       ADJUSTMENT FACTORS
                ADJ PKG BUSES PHF PEDS
     GRADE HV
                                           PED. BUT. ARR. TYPE
      (%)
            (%)
                 Y/N Nm Nb
                                           Y/N
                                                min T
EB
      0.00
            2.00
                n
N
                     Ø
                              0.89
0.88
                           0
                                       15
                                           Y
                                                15.8
WB
                      ő
      0.00 2.00
                           0
                                      Ø
Ø
                                           N
                                                15.8
                                                         5
     0.00 1.00 N 0 0
0.00 1.00 N 0 0
NR
                              0.79
0.86
                                          N
                                                21.8
SB
                                       20
                                          Y
                                               21.8
     SIGNAL SETTINGS CYCLE LENGTH = 180.0
        PH-1
              PH-2
                    PH-3 PH-4
                                        PH-1 PH-2 PH-3 PH-4
EB LT
         X
              X
                                NB LT
                                               X
   TH
                X
                                   TН
                                                Х
   RT
                                   RT
   PD
                X
                                   PD
WB LT
                X
                                SB LT
   TH
                                   TH
                                          Х
   RТ
                                   RT
                                          X
   PD
                                   PD
                                         X
GREEN
         5.0 102.0
3.0 5.0
                  0.0
                        0.0
0.0
                              GREEN
                                        28.0
                                            27.0
                                                   0.0 0.0
0.0 0.0
YELLOW
                    0.0
                              YELLOW
                                        5.0
                                            5.0
                    ______
                    LEVEL OF SERVICE
    LANE GRP.
            V/C
                         DELAY LOS
                                         APP. DELAY
                                                     APP. LOS
EB
            0.226
      L
                   0.622
                            11.7
                                   В
      T'
                                           59.9
             1.133
                   0.578
                            62.3
                                    F
      R
            0.051
                   0.578
                             5.0
                                    B
WB
      L
            0.731
                   0.622
                            39.3
                                           11.7
                                                        Ħ
      T
            0.633
                   0.578
                            9.9
                                    В
NB
      LT
                            75.7
            0.949
                   0.161
                                    F
                                          123.1
      R
            1.193
                   0.161
                           166.0
SB
            0.789
                   0.167
                            65.4
                                    F
                                            65.6
      LT 0.901 0.167
R 0.051 0.167
      LT
                            67.0
                  0.167
                           40.7
                                   E
______
                            -------
INTERSECTION:
           Delay = 53.8 (sec/veh) V/C = 1.084 LOS = E
```

	YST	• • • •	. a 9	: /29/9	2 5:00 ISTI	PH NG T	RAF	FIC VO	LUMES	(KAAPA	PPH)		·	
OMM'	ENT									G	EOMETRY		,	SB
	77.0		WB BW	MES NB		: B:		EB		WB 11.0	L	NB 11.0	LTR	11.0
T	EB 16		67	415		.3 :		11.	_	11.0	LTR	11.0		12.0 12.0
Ĥ	1532	_	54	1		4:	_	11.		12.0		12.0 12.0		12.0
T	651		17 6	62 10	•	0:		10.		12.0 12.0		12.0		12.0
R	150		0			:		12. 12.		12.0		12.0	_	12.0
						:								
								JUSTME	NT FAC	TORS	PED.		ARR.	TYPE
	GR	ADE	1	·V		PKG		USES Nb	Pnr	120	Y/N	min T		5
		ઢ )	•	š)	Α\Ν	N m O		0	0.87	0	N	10.9 10.9		5
B B		.00		.00	N N	ø		Ø	0.90	20	Y Y	22.1		3
NB		.00 .00	1	.00	N	Ø		Ø	0.96	10 10	Ÿ	22.1		3
NB SB		.00		.00	N	0		0	0.79					120.0
							IGN	AL SE	TTINGS		CY PH-1	CLE LEN PH-2	PH-3	PH-4
		10.	H-1	PH-	. 2	PH-3		PH-4	NB	LT	X	–		
EB	LT	_	·x	3					ИБ	TH	X			
	TH			2						RT	X			
	RT			2	K. K					PD	X X			
	PD		x		X.				SB	LT TH	X			
WB	LT TH		••		X					RT	X			
	RT				X X					PD		0.0	0.0	0.0
	ΡD		5.0			ø.	0	0.0	GRE	EN	32.0 5.0	0.0	0.0	0.0
	<b>LLOW</b> EEN		3.0		.0	ø.	0	0.0	YEI	TOM				
 X 121							 T.	EVEL (	OF SER	VICE		הפואט	API	e. Los
				V/	C	G/		D1	ELAY	POS	APP.	DELAY	24.4	c
		ИĒ (	GRP.	0.0		0.6	67		5.3	В	2	2.4		
EB	3	L T		0.9		Ø.6	00		26.3 10.6	D E				
		R		0.7	742	0.6			10.0	В		6.4		В
WE	3	L		0.4		0.6	500		6.2	В	_	2 0		D
		TR	•		595 578		283		29.7	D	3	2.0		-
NE	3	L LT	סו		720		283 283		33.9 24.8	D C	2	4.8		C

.1

SUM	B5 HCM	REPORT		INTERSE	CTIONS							
INT ARE ANA	ERSEC! A TYPI LYST.	rion E	Kaahum <i>i</i> CBD PGP		UE/(WAKE	A AVE	. / H.	C.C)	****	****	<i>, * * * * *</i>	****
TIM	ΙΕ	<i></i> .		5:00 PM	TRAFFIC	VOLUMI	ES (KA	.AWAKPH	)			
		vo	 Lumes	:				GEOM	 ETRY			
LT TH RT RR	EB 63 1184 378 90	WB 168 692 69	NB 259 48 132	SB: 59: 69: 39: 15:	T 11 T 11	.0 I .0 I .0 I .5 F	L 1 F 1 R 1	В	LT R	NB 10.5 10.5 12.0 12.0 12.0	LTR	SB 10.5 12.0 12.0 12.0 12.0
					ADJUSTM	בבבבב מים ישואה						
	GRA (%	; )	( % )	ADJ PKG Y/N Nm	BUSES Nb	PHF	PE:	DS I		BUT. min T		TYPE
eb Wb	0. 0.			N 0	0 0	0.87 0.93	7 1	0 1 15 }		10.6		5 5
NB SB	ø.	00	1.00	N 0	0	0.84 0.67		15 3 15 3	?	24.4		3. 3
		_		sı	GNAL SET	TINGS			CYCLI	E LEN	GTH =	90.0
EB	LT TH	PH-1 X	PH-2 X X	PH-3	PH-4	NВ		PH-1 X X			PH-3	PH-4
ИВ	RT PD LT	x	X X X			SB	RT PD LT	X X X				
	TH RT PD		X X X				TH RT PD	X X				
FREE YELI		5.0 3.0	37.0 5.0	0.0 0.0	0.0 0.0	GRE. YEL	EN LOW	35.0 5.0	0. 0.		0.0 0.0	0.0 0.0
			· • • • • • • •		LEVEL OF	SERV	ICE					
EB	L T		V/C 0.174 1.060	0.522 0.433	8 34	.7 .6	Los B D		. DEI 28.4	AY	APP.	
ИB	R L T		0.591 0.855 0.579	0.433 0.522 0.433	40 6	. 1	B E B		12.1		E	3
NB	R L' R	T	0.094 0.768 0.183	0.433 0.411 0.411	4 19 10	. 1	A C B		17.4		c	:
B		TR	1.087	0.411	85		F		85.6			

INTERSECTION: Delay = 26.0 (sec/veh) V/C = 1.052 LOS = D

```
1985 HCM: SIGNALIZED INTERSECTIONS
  SUMMARY REPORT
  INTERSECTION.. KAAHUMANU AVENUE/(KANE RD. / BEACH RD.)
  AREA TYPE....CBD
  ANALYST.....PGP
  DATE.....09/29/92
  TIME.....4:00 - 5:00 P.M
  COMMENT.....1992 EXISTING TRAFFIC VOLUMES (KAAKANPM)
              VOLUMES
                                                   GEOMETRY
        EB
              WB
                          SB :
                    NB
                                    EB
                                                 WB
                                                            ' NB
                                                                         SB
        74
              50
                    92
                         807 : L
                                    11.0
                                           L
                                                11.0
                                                             11.0
                                                                    Ļ
  ŤН
                                                                         11.0
      1158
             732
                   181
                         106 : T
                                    11.0
                                           Ţ
                                                 11.0
                                                        TR
                                                             11.0
                                                                   LT
                                                                         11.0
  ŔT
       130
             0
                   126
                         158 : T
                                    11.0
                                           T
                                                 11.0
                                                             12.0
                                                                    R
                                                                         11.0
               0
                   95
                          90
                             : R
                                    10.5
                                                12.0
                                                             12.0
                                                                         12.0
                                    12.0
                                                12.0
                                                             12.0
                                                                         12.0
                                    12.0
                                                12.0
                                                             12.0
                                                                         12.0
                               ADJUSTMENT FACTORS
       GRADE
                HV
                       ADJ PKG BUSES
                                       PHF
                                             PEDS
                                                       PED. BUT.
                                                                   ARR. TYPE
        (%)
                (왕)
                       Y/N Nm
                                  Nb
                                                      X \setminus N
                                                            min T
 EB
        0.00
               2.00
                       N
                            0
                                  0
                                        0.94
                                                 15
                                                       Y
                                                             16.4
                                                                        5
 WB
        0.00
               2.00
                       N
                             0
                                   0
                                        0.94
                                                 Ø
                                                             16.4
 NB
                                                                        5
        0.00
               1.00
                       N
                             0
                                   Ø
                                        0.95
                                                 15
                                                       Y
                                                             21.8
 SB
        0.00
               2.00
                            0
                                   0
                                        0.94
                                                 15
                                                             21.8
                           SIGNAL SETTINGS
                                                       CYCLE LENGTH = 120.0
           PH-1
                  PH-2
                         PH-3
                               PH-4
                                                   PH-1
                                                        PH-2 PH-3
 EB
     LT
             X
                  X
                                         NB
                                           LT
                                                            X
     TH
                    X
                                             TH
                                                            X
     RT
                    X
                                             RT
                                                            X
     PD
                    X
                                             PD
                                                            X
 WB
    LT
             Х
                                         SB
                                           LТ
                                                    Х
     TН
                                             TH
                                                    Х
    RT
                    Х
                                            RT
                                                    Х
    PD
                    Х
                                            PD
GREEN
            5.0
                          0.0
                  44.0
                                 0.0
                                        GREEN
                                                  35.0
                                                         18.0
                                                                         0.0
                                                                  0.0
YELLOW
            3.0
                  5.0
                          0.0
                                 0.0
                                       YELLOW
                                                   5.0
                                                         5.0
                                                                  0.0
                                                                         0.0
                         ______
                             LEVEL OF SERVICE
     LANE GRP.
                 V/C
                         G/C
                                   DELAY
                                         Los
                                                    APP. DELAY
                                                                  APP. LOS
EB
                0.234
        L
                         0.450
                                    15.6
                                             C
                                                       31.2
        T
                1.027
                         0.383
                                    33.6
                                              D
                0.161
                         0.383
                                     7.4
                                              В
        L
WB
                0.326
                        0.450
                                    16.9
                                              C
                                                       12.6
                                                                      В
        T
                0.686
                        0.383
                                    12.3
                                              Е
NB
        L
                0.391
                        0.167
                                    34.4
                                              ח
                                                       43.6
                                                                      Ε
        TR
                        0.167
                0.876
                                    47.5
                                              Ε
SB
        L
                0.942
                        0.308
                                   51.1
                                              Ε
                                                       68.8
                1.113
                                    89.2
                        0.308
                                              F
        R
                0.177
                        0.308
                                   19.6
                                              C
INTERSECTION:
                 Delay = 38.5 (sec/veh)
                                            V/C = 0.978
```

## APPENDIX C

# CAPACITY ANALYSIS CALCULATIONS

YEAR 2000 PEAK HOUR TRAFFIC ANALYSIS WITHOUT PROJECT

### FACILITY SECTION.... KAAHUMANU AVE W. OF MAHALANI ST ANALYST..... PGP TIME OF ANALYSIS.... AM PEAK HOUR DATE OF ANALYSIS.... PROJECTED OTHER INFORMATION.... KAAMAHOA. WOP\ A) ADJUSTMENT FACTORS DESIGN SPEED (MPH)..... 50 OBSTRUCTIONS..... NO DISTANCE (FT) FROM ROADWAY EDGE..... 6 TYPE OF MULTILANE HIGHWAY....... SUBURBAN, DIVIDED B) CORRECTION FACTORS -----E E E f f f f TERRAIN TYPE T B R HV W P E E LEVEL 1.7 1.5 1.6 1.00 0.97 1.00 0.90 C) OPERATIONAL ANALYSIS RESULTS NO. OF LANES..... 2 INPUT VOLUME..... 1468 COMPUTED CAPACITY (pcphpl).. 841 DENSITY (pcpmpl)..... 22

1985 HCM: MULTILANE HIGHWAYS

19 **	85 HCM: MULTILAN	E HIGH	WAYS	*****	****	****	*****	****
	FACILITY SECTI ANALYST TIME OF ANALYS DATE OF ANALYS OTHER INFORMAT	IS	. PGP . AM PE. . PROJE	AK HOUR CTED		NE ST		
A)	ADJUSTMENT FAC	TORS						
	PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED ( PEAK HOUR FACT	BUSES. RECREA MPH)	TIONAL	VEHICLE	0 S 0 50	(TYPICA	L - 200	#/HP)
	DRIVER POPULAT	ION FA	CTOR		1	(WEEKDA	Y/COMMU'	TER)
	OBSTRUCTIONS DISTANCE (FT)				ио			
	TYPE OF MULTIL					URBAN, 1	DIVIDED	
B)		ANE HI				URBAN,	DIVIDED	
B)	CORRECTION FAC	ANE HI	GHWAY  E	E	sub	 f	 f	
B)	TYPE OF MULTIL  CORRECTION FAC  TERRAIN TYPE	TORS	E B	E R	SUB	f W	f p	E
B)	CORRECTION FAC	TORS	E B	E R	SUB	f W	f p	E
	TYPE OF MULTIL  CORRECTION FAC  TERRAIN TYPE	TORS TORS TORS TORS TORS	E B  1.5	E R	SUB	f W	f p	E

SUMMARY REPORT INTERSECTION.. KAAHUMANU AVENUE/(MAHALANI RD. / KANALOA AVE.) AREA TYPE....CBD ANALYST.....PGP DATE.....PROJECTED TIME.....AM PEAK HOUR COMMENT.....2000 WOP; KAAMAHOA VOLUMES GEOMETRY FB WB NB SB : EB WB SB 11.0 L 11.0 T 11.0 11.0 LT46 306 37 320 : L LT 10.5 L 10.5 TH 752 1293 14 76 : T R 10.5 LT 10.5 RT Ø 11.0 T 11.0 132 88 12.0 R 138 : T 10.5 RR 30 0 65 60 : R 10.5 10.5 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 ADJUSTMENT FACTORS
ADJ PKG BUSES PHF PEDS GRADE HV PED. BUT. ARR. TYPE (%) ( % ) Y/N Nm Nb Y/N min T 0 0 EΒ 0.00 2.00 n N 15 0.86 0 15.8 Y 0.00 WB Ø Ø 2.00 0 0.85 N 15.8 5 N N 0.95 NB 0.00 1.00 0 N 21.8 3 SB 0.00 1.00 Ø 0 0.84 20 Y 21.8 \_\_\_\_\_\_ SIGNAL SETTINGS CYCLE LENGTH = 180.0 PH-1 PH-2 PH-3 PH-4 PH-1 PH-2 PH-3 PH-4 EB LT X NB LT ΤH Х TН Х RT X RT PD X PD WB LT X X SB LT Х THХ THRTRT X PD PD X 5.0 15.0 79.0 0.0 3.0 3.0 5.0 0.0 GREEN GREEN YELLOW 39.0 0.0 21.0 0.0 YELLOW 5.0 5.0 0.0 LEVEL OF SERVICE LANE GRP. V/C G/C DELAY LOS APP. DELAY APP. LOS L 0.660 0.478 47.6 E 16.0 T 0.656 0.450 15.1 Ç R 0.205 0.450 9.1 В WB 0.800 0.594 34.3 D 24.1 C 0.550 · T 0.933 21.8 C NB LT 0.284 0.128 46.1 E 45.8 E 45.1 R 0.148 0.128 E 52.5 SB 0.690 0.228 Ε 47.1 0.228 LT 0.718 45.6 Ε R 37.6 0.318 0.228 D \_\_\_\_\_\_\_ INTERSECTION: Delay = 25.7 (sec/veh) V/C = 0.815 LOS = D

1985 HCM: SIGNALIZED INTERSECTIONS

·\_\_\_,

AN DA TI	ALYSI TE		.KAAHU .CBD .PGP .PROJE .AM PEI	CTED	t D		PA AVEN	UE				****	* * * * *	*
			OLUMES		.=					OMETRY				_
LT	E		B NE		B :		EB		WB	OHETR	NB		C.D.	
TH	83	_	7 560		5 :		11.0	L	11.0	L	11.	Ø LTF	SB 11.0	2
RT	29		-		0:	_	11.0	T	11.0	LTR	11.		12.0	
RR	9		7 37 7 10		4 :		11.0	TR	12.0		12.		12.0	
	_	•	, 10		0:		10.5		12.0		12.		12.0	
					:		12.0		12.0		12.		12.0	
							12.0	<b>_</b>	12.0		12.	0	12.0	
	_					ADJUS	MENT F	ACTOR	5					
		RADE	HV	ADJ 1	PKG	BUSES	PH		EDS	PED.	Ditm			
3 D		<b>%</b> )	(名)	Y/N	Nm	Nb				Y/N	min		. TYPE	i
B B		.00	2.00	N	0	0	0.8	5	Ø	N	10	_	_	
iB		.00	2.00	N	0	0	0.9	9	20	Ÿ	10		5 5	
B		.00	1.00 0.00	N		0	0.7	_	10	Ÿ	22.		3	
		. 00 		N	0	Ø	0.9	2	20	Y	22.		3	
					ST	 SNAT 6	ETTINGS							
		PH-1	PH-2	PH	-3	PH-4	ETTING	5		CYCI	LE LE	NGTH =		
В	LT	X	X			T 11 - 4	NB	7 m	PH-		1-2	PH-3	PH-4	
	TH		x				МВ	LT TH	X					•
	RT		x					RT	X					
В	PD		X			•		PD	X					:
<b>H</b>	LT	X	X				SB	LT	X					
_	m • •		X					TH	x					-
	HT							TH						_
_	RT		X					RT						
_	RT PD	E A	X X	-				RT PD	X					ļ
REE	RT PD IN	5.0	X X 41.0		. ø	0.0	GRE	RT PD EN	Х 31.	ø ø	. Ø	0.0	a.a	-
REE	RT PD	5.0 3.0	X X		. Ø . Ø	0.0 0.0		RT PD	х	0 0	.ø .ø	0.0 0.0	Ø.ø Ø.ø	
REE	RT PD IN		X X 41.0		. 0	0.0	YEL	RT PD EN LOW	Х 31.	0 0		0.0 0.0	0.0 0.0	1
REE	RT PD IN IOW		X X 41.0 5.0		. 0  L	Ø.Ø EVEL (	YEL  F SERV	RT PD EN LOW 	31. 5.	0 0 0 0 	. 0 			
REE ELI	RT PD IN IOW	3.0 GRP.	X X 41.0 5.0 V/C	Ø:  G:	. Ø  L	Ø.Ø EVEL (	YEL  F SERV LAY	RT PD EN LOW  ICE LOS	31. 5.	0 0 0 0 	. 0 	0.0 	0.0 	-
REE ELI	RT PD IN OW LANE	3.0 GRP.	X X 41.0 5.0	0 G, 0.5	.0 L 'C 567	Ø.Ø EVEL (	YEL F SERV LAY 6.5	RT PD EN LOW  ICE LOS B	31. 5.	0 0 0 0 	. 0 	0.0 	0.0	-
REE ELI 	RT PD IN OW LANE	3.0 GRP.	X X 41.0 5.0 V/C 0.022 0.691 0.397	Ø:  G:	.0 L 'C 567	Ø.Ø EVEL (	YEL F SERV LAY 6.5 7.5	RT PD EN LOW  ICE LOS B B	31. 5.	0 0 0 0 	. 0 	0.0 	0.0 	-
REE	RT PD IN OW LANE I R	3.0 GRP.	X X 41.0 5.0 V/C 0.022 0.691	G / O · 5 O · 4	.0 L C 567 78	0.0  EVEL O	YEL F SERV LAY 6.5 7.5 4.7	RT PD EN LOW  ICE LOS B B A	31. 5.	0 0 0 0  PP. DE: 6.9	. 0 	0.0 APP.	0.0 LOS B	
REE ELI 	RT PD IN OW LANE I R L	3.0 GRP.	X X 41.0 5.0 V/C 0.022 0.691 0.397 0.112 0.881	6, 0, 5 0, 4	. 0 L . C . 67 . 78 . 78	0.0  EVEL O	YEL F SERV LAY 6.5 7.5 4.7 6.9	RT PD EN LOW  ICE LOS B B A B	31. 5.	0 0 0 0 	. 0 	0.0 APP.	0.0 	
REE ELI 	RT PD IN OW LANE I T L	3.0 GRP.	X X 41.0 5.0 V/C 0.022 0.691 0.397 0.112 0.881 0.719	G, 0.5 0.4 0.4	.0 C 567 78 78 67	0.0  EVEL C DE	YEL F SERV LAY 6.5 7.5 4.7 6.9 2.6	RT PD EN LOW  ICE LOS B A B B	31. 5.	0 0 0 0 PP. DE: 6.9	. 0 	0.0 APP.	LOS B	-
REE ELI	RT PD IN OW LANE T L	3.0 GRP.	X X 41.0 5.0 V/C 0.022 0.691 0.397 0.112 0.881	G / 0 · 5 0 · 4 0 · 5 0 · 4	.0 .0 .67 .78 .78 .78 .67 .67	0.0  EVEL C DE	YEL F SERV LAY 6.5 7.5 4.7 6.9	RT PD EN LOW  ICE LOS B B A B	31. 5.	0 0 0 0  PP. DE: 6.9	. 0 	0.0 APP.	LOS B	

1985 HCM: SIGNALIZED INTERSECTIONS SUMMARY REPORT INTERSECTION.. KAAHUMANU AVENUE/(WAKEA AVE. / M.C.C) AREA TYPE....CBD ANALYST.....PGP DATE.....PROJECTED TIME.....AM PEAK HOUR COMMENT.....2000 WOP KAAWAKOA VOLUMES GEOMETRY WB EB EB WB NB SB : NB 18 : L 11.0 L 45 : T 11.0 T 50 : T 11.0 T SB 11.0 L 11.0 T LT 84 109 237 11.0 LT10.5 LTR 10.5 TH 548 981 69 11.0 10.5 12.0 RT 284 101 45 11.0 12.0 12.0 RR 90 25 35 10.5 12.0 12.0 : 12.0 12.0 12.0 12.0 12.0 12.0 12.0 ADJUSTMENT FACTORS GRADE HV ADJ PKG BUSES PHF PEDS PED. BUT. ARR. TYPE Y/N Nm Nb N 0 0 (%) 0.86 15 0.90 0 0.94 15 0.71 15 (%) Y/N min T 0.00 2.00 N 0 0 0.00 2.00 N 0 0 0.00 1.00 N 0 0 0.00 0.00 N 0 0 EB Y 10.6 WB N 10.6 5 NB Y 24.4 SB 24.4 3 -----SIGNAL SETTINGS CYCLE LENGTH = 90.0 PH-1 PH-2 PH-3 PH-4 PH-1 PH-2 PH-3 NB LT EB LT X X X TH Х THX RT X RTX PD PD X WB LT X X SB LT THХ TH RT Х RT PD X PD 0.0 0.0 GREEN 0.0 0.0 YELLOW GREEN 5.0 40.0 32.0 0.0 0.0 5.0 0.0 0.0 0.0 0.0 YELLOW 3.0 5.0 LEVEL OF SERVICE LANE GRP. V/C DELAY LOS APP. DELAY APP. LOS EB 0.354 0.556 L 9.1 В 5.4 T 0.461 0.467 5.0 В R 0.376 0.467 A 4.8 WB 0.249 0.556 8.1 В 8.4 В T 0.788 0.467 8.7 В R 0.139 0.467 4.2 Α NB LΤ 0.703 0.378 18.1 C 17.9 С R 0.022 0.378 11.4 В LTR 0.395 0.378 13.6 В 13.6 В INTERSECTION: Delay = 8.7 (sec/veh) V/C = 0.712 LOS = B

H 500 997 159 100: T 11.0 T 11.0 TR 11.0 LT 11.0 LT 11.0 R 50 0 64 91: T 11.0 T 11.0 T 11.0 12.0 R 11.0 R 50 0 50 70: R 10.5 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0	RE NA AT IM	A TYI LYST E	PE	• • • •	CBD	CTED AK H	our			ANE	RD.		BEACH	RD	.) 		<b></b>	
GRADE HV ADJ PKG BUSES PHF PEDS PED. BUT. ARR. TYPE  (%) (%) Y/N Nm Nb Y/N min T  B 0.00 2.00 N 0 0 0.86 15 Y 16.4 5  B 0.00 2.00 N 0 0 0.84 0 N 16.4 5  B 0.00 1.00 N 0 0 0.84 15 Y 21.8 3  B 0.00 2.00 N 0 0 0.84 15 Y 21.8 3  B 0.00 2.00 N 0 0 0.84 15 Y 21.8 3  B 0.00 2.00 N 0 0 0.84 15 Y 21.8 3  SIGNAL SETTINGS CYCLE LENGTH = 140.0  PH-1 PH-2 PH-3 PH-4 PH-1 PH-2 PH-3 PH-4  B LT X X RT X PD  X RT X X RT X PD  B LT X X S SB LT X  TH X X RT X PD  X REEN S.0 52.0 0.0 0.0 GREEN 43.0 22.0 0.0 0.0  ELEVEL OF SERVICE  LANE GRP. V/C G/C DELAY LOS APP. DELAY APP. LOS  B L 0.426 0.443 22.4 C 11.3 B  T 0.509 0.386 10.1 E  R 0.040 0.386 8.2 B  B L 0.101 0.443 17.3 C 29.2 D  T 0.984 0.386 29.6 D  T 0.986 0.321 59.7 E 54.4 E  LT 0.973 0.321 51.1 E  R 0.060 0.321 51.1 E  R 0.060 0.321 51.1 E	T H T R	6: 506 6:	1 9 : 7	WB 34 997 0	N 9 15 6	<b>9</b> 9 1	701 100 91	:	T T	11. 11. 10. 12.	Ø 9 5 Ø	T	11 11 11 12 12	.0	L	NB 11.0 11.0 12.0 12.0	LT R	11.0 11.0 11.0 12.0 12.0
PH-1	B B B	(	(%) 0.00 0.00 0.00		(%) 2.00 2.00 1.00	Y/Y N N N		N m 0 0 0	BUS	ES b 0 0	P 0. 0.	HF 86 84 95	PED 1	5 0 5	Y/N Y N Y	min 16. 16. 21.	T 4 4 8	5 5 3
REEN 5.0 52.0 0.0 0.0 GREEN 43.0 22.0 0.0 0.0 CELLOW 3.0 5.0 0.0 0.0 YELLOW 5.0 5.0 0.0 0.0 CELLOW 3.0 5.0 0.0 0.0 YELLOW 5.0 5.0 0.0 0.0 0.0 CELLOW 5.0 5.0 0.0 0.0 0.0 0.0 0.0 CELLOW 5.0 5.0 0.0 0.0 0.0 0.0 CELLOW 5.0 5.0 0.0 0.0 0.0 0.0 0.0 0.0 CELLOW 5.0 5.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	iB IB	TH RT PD LT TH RT	. PI	x			PH				N	B I	TH PD T TH RT		-1 X X X	PH-2 X X		
LANE GRP. V/C G/C DELAY LOS APP. DELAY APP. LOS  B L 0.426 0.443 22.4 C 11.3 B  T 0.509 0.386 10.1 E  R 0.040 0.386 8.2 B  NB L 0.101 0.443 17.3 C 29.2 D  T 0.984 0.386 29.6 D  IB L 0.409 0.171 39.9 D 39.6 D  TR 0.686 0.171 39.4 D  BB L 0.966 0.321 59.7 E 54.4 E  LT 0.973 0.321 51.1 E  R 0.060 0.321 21.2 C		EN			52	0			-	-		REEN	ī	43	.0			
TB L 0.101 0.443 17.3 C 29.2 D T 0.984 0.386 29.6 D TB L 0.409 0.171 39.9 D 39.6 D TR 0.686 0.171 39.4 D TB L 0.966 0.321 59.7 E 54.4 E LT 0.973 0.321 51.1 E R 0.060 0.321 21.2 C	B	LAì	L T	RP.	0.4 0.5	26 99	Ø.	443 386	3	DEL 22 10	AY . 4 . 1	RVI	LOS C E	•			APP.	
TR 0.686 0.171 39.4 D  BB L 0.966 0.321 59.7 E 54.4 E  LT 0.973 0.321 51.1 E  R 0.060 0.321 21.2 C			L T		0.10 0.9	)1 34	Ø. Ø.	443 386	3	17 29	.3		C D					
INTERSECTION: Delay = 33.8 (sec/veh) V/C = 0.891 LOS = D			TR L LT		0.68 0.96 0.9	36 56 73	0. 0. 0.	17: 32: 32:	L L	39 59 51	. 4		E E		54	. 4		E
$\cdot$	nT)	ERSEC	TIOL		]	ela;	y =	3	33.8	(sec	:/ve	 h)	V/C	= (	0.891	L	OS = D	

	* *	******	*****	****	*****	*****	*****	****	*****
		FACILITY SECT ANALYST TIME OF ANALY DATE OF ANALY OTHER INFORMA	SIS	PGP PM PI PROJI	EAK HOU:	R	' MAHAL	ANI ST	
<u></u>	A)	ADJUSTMENT FA							
		PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED	TRUCKS BUSES RECREA (MPH).	TIONAL	VEHICLE	0 0 .s. 0		L - 200	#/HP)
		PEAK HOUR FACT	ror rion f <i>i</i>	ACTOR		1	(WEEKDA	Ү/СОММИ	TER)
		LANE WIDTH (FT OBSTRUCTIONS DISTANCE (FT) TYPE OF MULTII	FROM F	ROADWAY	EDGE	NO	URBAN.	DIVIDED	
	в)	CORRECTION FAC					·		
7254 		TERRAIN TYPE	E	E	R	f HV		f p	f E
		LEVEL		1.5	1.6	1.00	0.97	1.00	0.90
	C)	OPERATIONAL AN							
		NO. OF LANES INPUT VOLUME V/C RATIO			. 1639				
		LEVEL OF SERVI COMPUTED CAPAC SPEED (mph) DENSITY (pcpmp	CE ITY (p	cphpl).	. C . 939 . 40				
<b></b>									

1985 HCM: MULTILANE HIGHWAYS

19 **	85 HCM:MULTILA	*****	****	****	******	*****	*****	*****
	FACILITY SECT ANALYST TIME OF ANALY DATE OF ANALY OTHER INFORMA	sis	. PGP . PM PE . PROJE	AK HOUF	2	ANE ST		
A)	ADJUSTMENT FAC	CTORS						
•	PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED PEAK HOUR FACTORIVER POPULATIONS	BUSES. RECREA (MPH) POR PION FA	TIONAL CTOR	VEHICLE	0 SS 0 50 1 1	(WEEKDA		
•	OBSTRUCTIONS. DISTANCE (FT) TYPE OF MULTIN	FROM R	OADWAY	EDGE	6	URBAN, 1	DIVIDED	
•	DISTANCE (FT)	FROM R LANE HI	OADWAY GHWAY	EDGE	6 sub	URBAN,		
•	DISTANCE (FT) TYPE OF MULTI	FROM R LANE HI CTORS	OADWAY GHWAY  E	EDGE	6 sub		 f	 f
•	DISTANCE (FT) TYPE OF MULTI  CORRECTION FAC	FROM RANE HI	OADWAY GHWAY E B	EDGE E R	f		f p	 f E
В)	DISTANCE (FT) TYPE OF MULTI  CORRECTION FACTOR TERRAIN TYPE LEVEL OPERATIONAL AN	TORS  T  T  1.7	OADWAY GHWAY E B 	EDGE E R	f	f W	f p	 f E

INTE	ERSECT	IONK	AAHUMAN BD	A U	VENU	E/	(HAHAL	AN.	r RD	. /	KAN	ALUA A	VE.,		
ANAI	LYST	P	GP												
DATI	B	P	ROJECTE	D.											
TMI	R	P	M PEAK	HOU	R	ua:	D								
COM	MENT	2	000 WOI	;	AAMA	.ne									- <del></del> -
			UMES		:						G	EOMETR	Y		
	EB		NB	5	: A		EB				В		NB	_	SB
LT	45	60		33	37 :	L	1 1	0	L	1	1.0	LT	10.5	L	10
TH	1547	844	52	•	6 .	T	11.	0	T	1		R		ГŢ	
RT	47	Ø	194	2	5 :	T	11.	0	T			)		R	12
		ø	130	1	.5 :	R	10.					5	12.0		12
1/1/	20	_	•		:		12.					)	12.0		12
					:		12.	0		1	2.0	) <b></b>	12.0		
						חב. ממ	JUSTME	יינו יינו	FAC	TORS	<b></b>				
	GR <i>I</i>	n P	HV	ד.ת ב	PKG			- • • •	PHE	PE	DS	PED	. BUT.	ARR	. TY
			%) ·	7 / N	Nm	-	ממ					V/N	min	ጥ	
E D	0.	s) (	.00	N				Ø	.89		15	Y	15.	8	5
EB WB	ø.	00 2	.00	N	0		0	0	.88		0	n N	15.	8	5 3
NB	ø.	00 1	. 00 . 00 . 00	N	0		9	-							3
SB	Ø.		.00	N	0		0	0	.86		20		21.		
								 	TCC			CY	CLE LE	NGTH =	180
			D				AL SE	LTI	MGS		1	PH-1	PH-2	PH-3	PH
			PH-2		Fn-3		E11-4		NB	LT			X		
EB	LT	X	X X							TH			X		
	TH		x							RT			X		
	RT		X							PD					
***	PD LT	x	X						SB	LT		X			
WB	TH	Δ.	X							$\mathbf{TH}$		X			
	RT									RT		X			
	PD									PD		X			6
GRE	EN	5.0	95.0		0.0		0.0		GRE	EN		31.0		0.0 0.0	(
	TOM	3.0	5.0		0.0		0.0		YEL	LOW		5.0	5.0		`
							EVEL O		ERV	TCE					
	T B M	E GRP.	V/C		G/C		DE	LA	7	LO.	S		DELAY	APP	L.L
			0.167		0.58			3.3		В		4	7.7		E
EB		L T	1.089		0.53			9.4		E					
		R	0.045		0.53			6.0	9	В					_
WB		L	0.645		0.58		3	1.8		D		10	0.2		В
11 15		T	0.601		0.53			8.8		В					E
NB		LT	0.751		0.18			1.		Ē		4:	В.6		Ľ
		R	0.338	}	0.18			1.		E			o 1		E
SB		L	0.735		0.18			9.		E		5	8.1		
		LT	0.840		0.18			7.		E					
		R	0.051		0.18	3	3	9.	Ļ	D				<b></b>	

SUN *** ARI ANI DAT	MMARY RE****** FERSECTI EA TYPE. ALYST FE	ON	KAAHUMA CBD PGP PROJECT		******** UE/PAPA	A AVENU	* * * * * E	**************************************	V अर्थ तथ थे अथ श	******	* * * * * *	****
		vo	LUMES					GEC	METRY			
LT TH RT RR		WB 56 794 14	NB 316 1 47	SB: 13: 14: 11: 0:	L 1 T 1 R 1	B 1.0 1 1.0 5 1.0 5 2.0 2	T TR	WB 11.0 11.0 12.0 12.0 12.0	L	NB 11.0 11.0 12.0 12.0 12.0		SB 11.0 12.0 12.0 12.0 12.0
					ADJUST	MENT FA	CTORS	<b>-</b>				
eb Wb Nb Sb	GRAD: (%) 0.00 0.00 0.00	2 2	(%)	ADJ PKG Y/N Nm N 0 N 0 N 0 N 0	BUSES Nb 0 0		' Pi	EDS	Y/N N Y	BUT. min 1 10.9 10.9 22.1 22.1	; ;	TYPE 5 5 3
					CNAT C							
ЕВ	LT TH RT PD	PH-1 X	PH-2 X X X X		IGNAL SI PH-4	ETTINGS NB	LT TH RT	PH- X X X	1 PI		GTH = PH-3	
WB	LT TH RT PD	х	X X X			SB	PD LT TH RT PD	X X X				
GREE YELI		5.0 3.0	64.0 5.0	0.0 0.0	0.0 0.0	GRE Yel		38.6 5.6		).Ø ).Ø	0.0 0.0	0.0 0.0
EB	LANE G L T R	RP.	0.043 0.962	G/C 0.617 0.550	2	6.9 4.5	LOS B C	AF	PP. DE 21.4			Los C
WB	L		0.694 0.393	0.550 0.617		1.4	B B	•	7.3		1	3
NB	TR L		0.538 0.379	0.550		7.1	В					
	LTR		0.379	0.333 0.333	_	3.5 4.6	C		24.1		(	
SB	LTR		0.136	0.333		1.2	č		21.2		C	31
INTE	RSECTIO	 N:	Del	ay = 1	7.7 (se	c/veh)	V/(	C = 0.	748	LOS	 S = C	,

	ERSECTIO A TYPE			. טא	AVEN	UE,	(WAKEA	AVE.	/ м.	C.C	)			
ANA	LYST	F	PGP											
	E				מנו									
COM	E MENT	2	2000 WC	P:	KAAW.	AK	ð P							
		-	LUMES						7.1		EOMETR	NB		SB
	EB	WB	NB		SB :	Ψ.	EB 11.			1B 1.0	LT	10.5	LTR	
LT	63	140	197 48		39 : 69 :			-			R			12.0
TH RT	1054 337	69			59 :		11.0		. 1	1.0	•	12.0		12.0
RR	90		50		15 :		10.		₹ 1	0.5		12.0		12.0
KK	20				:		12.0		1	2.0		12.0		12.0
					:		12.0	9	1	2.0		12.0		12.0
						 ια	JUSTME	 44 TV	CTORS	. <b></b> .				
	GRADE	7	нv	ADJ	PKG		BUSES	PHI		DS	PED	. BUT.	ARR.	TYPE
	(%)			Y/N			Nb				Y/N	min T		_
EB	ò.óe		2.00	N	0		0	0.8	7 3	0	N	10.6		5
WB	0.00	) 2	2.00	N	Ø					15	Y	10.6		5
NB	0.00		1.00	N N	0		0	0.84			Ä	24.4		3
SB	0.00		0.00	N	0		0	0.6		15		24.4		
			. <b></b>				NAL SET					CLE LEN		90.0
	E	PH-1	PH-2	!			PH-4		•	P		PH-2	PH-3	PH-4
EB	LT	x	X					ИВ			X			
	TH		X						TH		X			
	RT		X						RT PD		X X			
	PD		X					SB			X			
WB	LT	X	X X				•	36	TH		X			
	TH RT		X						RT		x			
	PD		X						PD					
GRE		5.0	37.0	)	0.0		0.0	GRI	EEN	3	5.0	0.0	0.0	0.0
	LOW	3.0	5.6		0.0		0.0	YE	LLOW		5.0	0.0	0.0	0.0
							EVEL OF		TCE					
	LANE G	RP.	V/C		G/C		DEL.		LOS	3	APP.	DELAY	APP.	Los
EB	L		0.154	ļ	0.52			. 5	В		14	. 9		В
	T		0.943		0.43	3	17		С					
	R		0.507	•	0.43	3.	5	. 9	в			_		_
WB	L		0.707		0.52		22		С		8	. 6		В
	T		0.482		0.43			. 6	В					
	R		0.094		0.43			. 6	A			-		В
	ĽТ		0.634		0.41		15		C		14	. 5		5
NB						1	70	. 6	В					
NB SB	R LTF		0.113 0.649		0.41 0.41		16		c		16	. 2		C

,

AN DA TI	ALYST. TE		.PGP .PROJEC .PM PEA	TED K HOUR OP; KAAK			D. /	BEACH R	D.)	****	****	****
			OLUMES	:		<b>-</b>		 G)	EOMETR	 Y		
LT TH RT RR	EB 65 1013 114 55	5 6 6	/B NB 0 84 4 166 0 117 0 95	SB : 776 : 102 : 152 : 90 : :	T T	EB 11.0 11.0 10.5 12.0 12.0	L T	WB 11.0 11.0 11.0 12.0 12.0	L TR	NE 11.0 12.0 12.0 12.0	D LT D R	SB 11.0 11.0 12.0 12.0 12.0
					ADJ	USTMENT	PAC	TORS				
EB WB NB SB	( ° 0 . 0 .	ADE . 00 . 00 . 00	HV (%) 2.00 2.00 1.00 2.00	ADJ PKG Y/N Nm N Ø N Ø N Ø	BU	SES Nb 0 0 0 0	PHF .94 .94 .95	PEDS 15 0 15 15	PED. Y/N Y N Y Y	BUT. min 16. 16. 21.	T 4 4 8	5 5 3 3
		•		sı	GNAI	L SETTI	 NGS		 cyc		 NGTH =	120.0
EB WB	LT TH RT PD LT TH	PH-1 X	X X X X X X	PH-3		<b>i-4</b>	NB L T R P SB L	'H 'T 'D	-1 P	H-2 X X X X X	PH-3	PH-4
	RT PD		X X				R	T 3				
GREE YELI	en	5.0 3.0	40.0	0.0 0.0			P REEN ELLO	39.		3.0 5.0	Ø.0 Ø.0	0.0 0.0
					EVE	L OF SE	RVIC	 E				
EB	LANE L T R	GRP.	V/C 0.203 1.039 0.139	G/C 0.417 0.350 0.350		DELAY 17.1 37.7		LOS A C D	PP. DE 35.1			Los D
WB	L		0.252	0.417		8.1 17.6		B C	13.6			В
NB	T L TF		0.681 0.357 0.774	0.350 0.167 0.167		13.3 34.1 38.8		C B D D	37.3			D
SB	L L1		0.817	0.342 0.342		34.5 44.1		D E	38.4		i	D

.

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# APPENDIX C

## CAPACITY ANALYSIS CALCULATIONS

YEAR 2000 PEAK HOUR TRAFFIC ANALYSIS WITH PROJECT

•	19 **	85 HCM: MULTILA	NE HIGH	HWAYS	****	******	*****	****	****
		FACILITY SECT ANALYST TIME OF ANALY DATE OF ANALY OTHER INFORMA	SIS	PGP AM PE PROJE	AK HOUF	R	МАНАLA	NI ST	
	A)	ADJUSTMENT FA	CTORS						
		PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED	BUSES. RECREA	TIONAL	 VEHICLE	0 S 0	(TYPICA	L - 290	#/HP)
		PEAK HOUR FACTORIVER POPULA LANE WIDTH (FOUR TRANS POPULA DISTANCE (FT) TYPE OF MULTI:	TOR TION FAT) FROM F	ACTOR	EDGE	1 11 NO	•	Y/COMMU	TER)
	B)	CORRECTION FAC	CTORS						
		TERRAIN TYPE		E B		f HV		ţ p	f E
		LEVEL	1.7			1.00	0.97	1.00	0.90
	C)	OPERATIONAL A							
		NO. OF LANES. INPUT VOLUME. V/C RATIO LEVEL OF SERVI COMPUTED CAPAC SPEED (mph) DENSITY (pcpm)	ICE	cphpl).	. 1472 44 . B . 843				

198	35 HCM: MULTILAN	E HIGHW	1AY5 *******	******	*****	****	*****	****
	FACILITY SECTI ANALYST TIME OF ANALYS DATE OF ANALYS OTHER INFORMAT	IS	PGP AM PE! PROJEC	AK HOUR	E E. KAN	E ST		
A)	PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED ( PEAK HOUR FACT DRIVER POPULAT LANE WIDTH (FT	TRUCKS. BUSES. RECREAT MPH) OR	TIONAL V	/EHICLES	0 5 50 1 1 (		200 //COMMUT	
	DISTANCE (FT) TYPE OF MULTII	FROM RO	OADWAY 1	EDGE	6 svau	RBAN, 1	DIVIDED	
в)	DISTANCE (FT) TYPE OF MULTII	FROM RO	OADWAY 1	EDGE	6 subu			
в)	DISTANCE (FT) TYPE OF MULTII	FROM RO	GHWAY	EDGE E R	SUBU	rban, i	DIVIDED f p	f 5
B)	DISTANCE (FT) TYPE OF MULTII  CORRECTION FAC	FROM RO	SHWAY	E R	svau	f w	f p	Ξ
-•	DISTANCE (FT) TYPE OF MULTII  CORRECTION FAC  TERRAIN TYPE  LEVEL	TORS TORS TORS TORS TORS	E B 	E R	SUBU	f w	f p	Ξ

	41 M L L N '1'		. 2000 W	K HO		u:n									
			LUMES					<b></b> -							
	EB			:	SB:	7	SB		IJ	B B	METRY	Y NB			SB
LT	46				20 :			L		1.0	LТ		5	L	10.
TH			7 14		76 :		11.0			1.0		_	5		10.
RT	132	6	154	1:	38 :	T :	11.0	${f T}$		1.0		12.		R	10.
RR	30	6	65	(	60 :	R 1	10.5			0.5		12.			12.
					:	3	12.0			2.0		12.			12.0
					:	1	2.0		1	2.0		12.			12.0
						ADJUSI	MENT	FAC	TORS						
	GR.	ADE	ΗV	ADJ	PKG			PHF	PE	DS	PED.	BUT		ARR.	TYP
	(	ኔ )	(%)	Y/N	Nm	Nb					Y/N	min			
EB		.00	2.00	N	0	0	0	.86		15.	Y	15	. 8		5
WB			2.00	N	0	0 0 0	0.	.85		0	N	15			5
NB			1.00	N	Ø			.95		0	N	21	. 8		3
SB	0	.00	1.00	N		_		.84	:	20	Y	21	. 8		3
						GNAL S					CYC	TE L	מריי. מאכייו	 H =	 180.0
		PH-1	PH-2	Т									1110 11		
EB			4 44 6	-	'H-3	PH-4				PH-	1 P	H-2	PH-	- 3	_ PH ~ 4
	LT	X		•	X	PH-4			LT	PH-	1 P	H-2 X	PH-	- 3	PH-4
	TH	X		•	X X	PH-4		NB		PH-	1 P	_	PH-	- 3	2H-4
	TH RT	x		•	X X X	PH-4		NB	LT	PH-	1 P	X	PH-	-3	2H-4
	TH RT PD			•	X X X	PH-4	1	NB	LT TH RT PD		1 P	X	PH-	-3	PH-4
WB	TH RT PD LT	x	x	•	X X X X	PH-4	1	NB SB	LT TH RT PD LT	х	1 P	X	PH-	-3	2H-4
WB	TH RT PD LT TH			•	X X X	PH-4	1	NB SB	LT TH RT PD LT TH	X	1 P	X	PH-	-3	PH-4
WB	TH RT PD LT TH RT		x	•	X X X X	PH-4	1	NB SB	LT TH RT PD LT TH RT	X X X	1 P	X	PH-	- 3	2H-4
	TH RT PD LT TH RT PD	X	x x		X X X X		s	NB SB	LT TH RT PD LT TH RT PD	X X X	-	X X X			
GRE:	TH RT PD LT TH RT PD EN	X 5.0	X X 15.0	7	x x x x x	0.0	) S	NB SB SREE!	LT TH RT PD LT TH RT PD N	X X X X 39.6	a 2	x x x	ø.	. 0	0.0
GRE:	TH RT PD LT TH RT PD EN	X	X X 15.0	7	X X X X		) S	NB SB	LT TH RT PD LT TH RT PD N	X X X	a 2	X X X		. 0	9H-4 0.0
WB GRE YEL	TH RT PD LT TH RT PD EN LOW	X 5.0 3.0	15.0 3.0	7	X X X X X X X 7 7	0.0 0.0  LEVEL	S G Y OF SE	NB SB SREE! FELL	LT TH TRT CT TH TH TH O CE	X X X X 39.6	3 2 3	X X X	0. 0.	. 0	0.0
GRE YEL:	TH RT PD LT TH RT PD EN LOW LANE	5.0 3.0 GRP.	15.0 3.0 	7	X X X X X 9.0 5.0	0.0 0.0 LEVEL	G G Y OF SE ELAY	NB SB SREE! FELL	LT TH RT PD LT TH RT PD O CE LOS	X X X X 39.6	0 2 0	X X X 1.0 5.0	0. 0.	. 0 . 0 	0.0 0.0 LOS
GRE YEL:	TH RT PD LT TH RT PD EN LOW LANE	5.0 3.0 GRP.	15.0 3.0  V/C 0.660	7  0	X X X X X 9.0 5.0  G/C .478	0.0 0.0  LEVEL	GY OF SE ELAY 47.6	NB SB SREE! FELL	LT TH TRT PLT TH PD NO CE CE S E	X X X X 39.6	3 2 3	X X X 1.0 5.0	0. 0.	. 0 . 0 	Ø.Ø Ø.Ø
GRE YEL:	TH RT PD LT TH RT PD EN LOW LANE	5.0 3.0 GRP.	X X 15.0 3.0  V/C 0.660 0.715	7  0 0	X X X X X 2 9.0 5.0 	0.0 0.0  LEVEL	G Y OF SE ELAY 47.6 15.9	NB SB SREE! FELL	LTHTTPLHTTRPD W S C L E C	X X X X 39.6	0 2 0	X X X 1.0 5.0	0. 0.	. 0 . 0 	0.0 0.0 LOS
GRE: YEL: 	TH RT PD LT RT PD EN LOW LANE	5.0 3.0 GRP.	X X 15.0 3.0  V/C 0.660 0.715 0.205	7  0 0 0	X X X X X X 2 9.0 5.0  G/C 8.450 .450	0.0 0.0  LEVEL D	GY OF SE ELAY 47.6 15.9 9.1	NB SB SREE! FELL	LTHTDTHTDW-EOECB	X X X X 39.6	9 2 9 PP. DI	X X X 5.0 	0. 0.	.0 .0 .1PP.	0.0 0.0 Los
GRE: YEL: 	TH RT PD LT RT PD EN LOW LANE	5.0 3.0 GRP.	X X 15.0 3.0  V/C 0.660 0.715 0.205 0.869	7  0 0 0 0	X X X X X Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y	0.0 0.0  LEVEL D	G Y OF SE ELAY 47.6 15.9 9.1 43.6	NB SB SREE! FELL	LTHTDTHTD W S C L C B E	X X X X 39.6	0 2 0	X X X 5.0 	0. 0.	. 0 . 0 	0.0 0.0 Los
GRE: YEL: EB	TH RT PD LT RT PD EN LOW LANE	5.0 3.0 GRP.	X X 15.0 3.0  V/C 0.660 0.715 0.205 0.869 0.936	7  0 0 0 0 0	X X X X X 2 9.00 5.00 	Ø. Ø Ø. Ø  LEVEL D	G Y OF SE ELAY 47.6 15.9 9.1 43.6 22.0	NB SB SREE! FELL	THTDTHTD W S CBEC	X X X X 39.6	PP. DI	X X X 5.0 ELAY	0. 0.	.0 .0 .1PP.	0.0 0.0 LOS
GRE: YEL: 	TH RT PD LT RT PD EN LOW LANE	5.0 3.0 GRP.	X X 15.0 3.0 3.0 0.660 0.715 0.205 0.869 0.936 0.284	7  0 0 0 0 0	X X X X X 9.0 9.0 6.478 0.4594 0.5598 0.128	0.0 0.0  LEVEL D	GY OF SE ELAY 47.6 15.9 9.1 43.6 22.0 46.1	NB SB SREE! FELL	THTOTHTO W S C BECE	X X X X 39.6	9 2 9 PP. DI	X X X 5.0 ELAY	0. 0.	.0 .0 .1PP.	0.0 0.0 LOS
GRE. YEL: EB	TH RT PD LT TH RT PD EN LOW LANE T	5.0 3.0 GRP.	X X 15.0 3.0 3.0 0.660 0.715 0.205 0.869 0.936 0.284 0.566	7  00000000000000000000000000000000	X X X X X Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y	0.0 0.0  LEVEL D	GY OF SE ELAY 47.6 15.9 43.0 46.1 50.4	NB SB SREE! FELL	THTOTHTO W S C BECEE	X X X X 39.6	PP. DI 16.6 26.6	X X X 5.0 ELAY	0. 0.	. Ø . Ø . LPP. C	0.0 0.0 LOS
GRE. YEL: EB	TH RT PD LT TH RT PD EN LANE LANE LANE LANE LANE LANE LANE L	5.0 3.0 GRP.	X X 15.0 3.0 3.0 0.660 0.715 0.205 0.869 0.936 0.284	7  00000000000000000000000000000000	X X X X X 9.0 9.0 6.478 0.4594 0.5598 0.128	0.0 0.0  LEVEL D	GY OF SE ELAY 47.6 15.9 9.1 43.6 22.0 46.1	NB SB SREE! FELL	THTOTHTO W S C BECE	X X X X 39.6	PP. DI	X X X 5.0 ELAY	0. 0.	.0 .0 .1PP.	0.0 0.0 LOS

UMMA NTER AREA ANALY	RY REI	***** ON K.	LIZED  ***** AAHUMA BD GP ROJECT M PEAK 000 WE	**** NU A ED	venue	*** /P <sup>P</sup>	***	***** VENUE	* * * * *	***		* * * * * * * * * * * * * * * * * * *			
		VOL	UMES		:	-	EB		1	G √iB	EOMETRY	NB	" <b></b>	SB 11.0	·
LT TH RT RR	EB 97 873 299 90	WB 37 1255 20 7	NB 560 46 37 10	\$		r T	11 11 11 10 12	.0 .0 .5	R	11.0 11.0 12.0 12.0 12.0	LTR	11.0 11.0 12.0 12.0 12.0	LTR	12.0 12.0 12.0 12.0 12.0	
						ADJ	USTM	ENT F	ACTOR	S EDS	PED	. BUT.	ARR.	TYPE	
EB WB NB	GRAI (% 0.0 0.0	) 20 00 00	HV (%) 2.00 2.00 1.00	ADJ Y/N N N N	PKG	вu	SES Nb 0 0	PH 0.8 0.9 0.7	5 9 6	0 20 10 20	Y/N N Y Y	min T 10.9 10.9 22.1 22.1		5 . 5 . 3 3	
SB	 	00 ∹	0.00 		_	GN	AL SE	TTING	s		CY PH-1	CLE LEN PH-2	GTH = PH-3	90.0 PH-4	
EB	LT TH	PH-1 X	PH- X 2	_ [	PH-3	,	PH-4	NE			X X X X	• • •			
WB	RT PD LT TH	x	3	ζ .				S			X X				
GRE	RT PD EN	5.6	a 41	Х . Ø	0.0		ø.0 ø.0		REEN ELLOW		31.0 5.0	0.0 0.0	0.0 0.0	0.0 0.0	
	TOM	3.6	ə 5 	.0 	0.0 			OF SE	 RVICE				 * 10 T	. Los	-
EB		E GRP L	0.5	40	G/G Ø.56	7	ב	12.6 7.8	I	os B B	APP.	DELAY 7.6	REI	В	_
22		T R	Ø.7 Ø.3	97	0.4	18		4.7		A B	1	2.9		В	
WB		L TR	0.1 0.8	92	0.50	78		13.1		B C	2	5.6		ם	-
NB		L LTR	0.7 0.8	396	0.3	57		28.3 11.9		D B	1	1.9		8	-
SB		LTR	0.0		0.3  ay =	-		sec/v	 eh)		= 0.86	 io I	.os =	В	;

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1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION.. KAAHUMANU AVENUE/(WAKEA AVE. / M.C.C)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....PROJECTED
TIME.....AM PEAK HOUR
COMMENT.....2000 WP; KAAWAK1A
GEOMETRY
          VOLUMES
                     SB: EB WB

19: L 11.0 L 11.0

47: T 11.0 T 11.0

51: T 11.0 T 11.0

15: R 10.5 R 10.5
                                                                 SB
                                          WB
                                                     NB
           WB NB
     EB
                                                      10.5 LTR 10.5
                                                 LT
                237
    125
          109
LT
                                                                 12.0
                                                      10.5
         985
               111
TH
    548
                                                                 12.0
                                                      12.0
                45
    284
          123
RT
                                                                 12.0
                                                      12.0
                35
          25
RR
                                                                 12.0
                                                      12.0
                              12.0
                                                      :2.ଚ
                                           12.0
                               12.0
                         ADJUSTMENT FACTORS
                                                           ARR. TYPE
                   ADJ PKG BUSES PHF PEDS
                                                 PED. SUT.
             VH
( $ )
     GRADE
                                                Y/N min T
                   Y/N Nm Nb
      (8)
                                                      10.6
                                                 Y
                                   0.86
                                            15
                        0
                               0
                   N
WB 0.00 2.00 N 0 0 0.90 0 N 10.6
NB 0.00 1.00 N 0 0 0.94 15 Y 24.4
SB 0.00 0.00 N 0 0 0.71 15 Y 24.4
      0.00
             2.00
SB
                                                 CYCLE LENGTH = 90.0
                        SIGNAL SETTINGS
                                              PH-1 PH-2 PH-3 PH-4
                PH-2
                      PH-3 PH-4
         PH-1
                                               Х
                                     NB LT
                 X
EB LT
         X
                                                X
                                        TH
    TH
                                        RT
                  Х
    RT
                                                X
Z
                                        ΒĐ
                  Х
    PD
                                     SB LT
WB LT
                  X
                                        TH
                                                X
                  X
    TH
                                        RT
                  X
    RT
                                        PD
                  X
    PD
                                  GREEN 32.0
YELLOW 5.0
                                                               0.0
0.0
                                               0.0
                     0.0 0.0
0.0 0.0
          5.0
                40.0
GREEN
                5.0
          3.0
YELLOW
                          LEVEL OF SERVICE
                                                             APP. LOS
                            DELAY LOS
                                                APP. DELAY
                       G/C
     LANE GRP. V/C
                                                                 В
                                                    5.1
                                         B
                     0.556
                                12.7
               0.531
EB
       L
                                          В
                                  5.0
                      0.467
               0.461
        T
                                  4.8
                                          A
               0.376
                       0.467
                                                                 3
                                                    8.4
                                          B
                      Ø.556
                                  8.1
               0.249
WB
                       0.467
                                  8.8
                                          В
               0.791
        T
                                           Α
                                  4.3
               0.180
                       0.467
        R
                                20.8
11.4
14.2
                                                   20.6
               9.782
                       ø.378
ΝВ
        LT
                       0.378
               0.022
        LTR
                                          2
                      0.378
              0.453
SB
                             Delay = 9.4 (sec/veh) V/C = 0.763 LOS = B
INTERSECTION:
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1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION..KAAHUMANU AVENUE/(KANE RD. / BEACH RD.)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....PROJECTED
TIME.... AM PEAK HOUR
COMMENT.....2000 WP; KAAKANIA
______
                                          GEOMETRY
           VOLUMES
                                                               SB
                                         WB
                              EB
                      SB :
           WB NB
34 99
                                                               11.0
                                                    11.0
     EB
                               11.0 L
                                         11.0
                     703 : L
                                                               11.0
                                         11.0 TR
                                                          LT
LT
     61
                                                    11.0
                               11.0 T
11.0 T
                     100 : T
               181
                                                               11.0
     501 1026
                                                    12.0
ጥዝ
                                         11.0
                     91 : T
                                                               12.0
                64
          Ø
      67
                                                    12.0
RТ
                                         12.0
                      70 : R
                               10.5
                50
                                                               12.0
      50
                                                    12.0
RR
                                          12.0
                               12.0
                         :
                                                    12.0
                                          12.0
                               12.0
                         -----
                          ADJUSTMENT FACTORS
                                               PED. BUT.
                                                           ARR. TYPE
                   ADJ PKG BUSES PHF PEDS
              ΗV
                                               Y/N min T
      GRADE
                             Nb
                   Y/N Nm
             (%)
                                                     16.4
      (%)
                                               Y
                                   0.86
                        Ø
                              0
                   N
       0.00
             2.00
                                                               5
                                                      16.4
EB
                                                N
                                          Ø
                                 0.84
                         0
                              Ø ·
      0.00
             2.00
                    N
                                                      21.8
WB
                                           15
                                                Y
                                 0.95
                         0
                              0
             1.00
                    N
                                                      21.8
       0.00
NВ
                                           15
                                  0.84
                   N
                       Ø
             2.00
       0.00
SB
                                                CYCLE LENGTH = 140.0
                       SIGNAL SETTINGS
                                                              PH-4
                                                   PH-2 PH-3
                      PH-3 PH-4
                PH-2
          PH-1
                                                     X
                                    NB LT
                  X
           X
 EB LT
                                       TH
                  X
    TH
                                       RT
                  X
    RT
                                       PD
    PD
                                    SB
                                       LT
            X
 WB
    LT
                                               Х
                                        TH
                  X
    ТH
                                       RT
                                               X
                  X
    RT
                                               X
                                       PD
                                                                 0.0
                                                           0.0
    PD
                                                    22.0
                                             43.0
                                    GREEN
                              0.0
                        0.0
                                                                 9.0
                52.0
           5.0
                                                           0.0
                                              5.0
                                                    5.0
 GREEN
                       0.0 0.0
                                   AETFOM
                5.0
           3.0
 YELLOW
                       LEVEL OF SERVICE
                       _____
                                                           APP. LOS
                                               APP. DELAY
                                        LOS
                V/C
      LANE GRP.
                                                 11.4
                                          С
                                23.6
                       0.443
               0.463
       L
 EB
                                          В
                                 10.1
                       0.386
                0.510
        Т
                                         В
                                  8.2
                       0.386
                0.040
                                                                D
        R
                                                  33.2
                                          C
                                 17.3
                       0.443
                0.101
 WB
                                 33.7
                                          D
                       0.386
                1.012
                                                                E
        T
                                                  42.2
                                          D
                                 39.9
                       0.171
                0.409
 NB
        L
                                          Ξ
                                 43.3
                       0.171
                0.773
         TR
                                                  54.9
                                . 60.3
                                          F
                0.969
                       0.321
        L
 SB
                                 51.5
                       Ø.321
                0.975
         LT
                                 21.2
                       0.321
                0.060
         R
  INTERSECTION: Delay = 35.8 (sec/veh) V/C = 0.922
                                                        LOS = D
```

**** E A	HCM: MULTILANE ACILITY SECTION NALYST IME OF ANALYST ATE OF ANALYST OTHER INFORMAT	N	KAAHUMA PGP PM PEAK	HOUR	w******	******* AHALANI	**************************************	<b>स्त्र</b> में में
!	ADJUSTMENT FACTOR OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED (PEAK HOUR FACTORIVER POPULATIONS DISTANCE (FT)	TRUCKS. BUSES. RECREAT MPH) OR	TOR.	EHICLES	0 50 1 1 (	WEEKDAY	- 200 #	
B)	CORRECTION FA	CTORS  E	 E	 E	f	 £ W	 f p	f E
	TERRAIN TYPE	T	В	R 	HV 			9.90
	LEVEL	1.7	1.5	1.6	1.00	ø.97	1.00	Ø. <del>.</del> .
C)	OPERATIONAL F ************************************	VICE	pcphpl	. 2 . 1679 51 C 962 40				

198	35 HCM: MULTILAN	E HIGH	NAYS	*****	****	*****	****	****
	FACILITY SECTION ANALYSTTIME OF ANALYST DATE OF ANALYST OTHER INFORMATION	SIS	. PGP . PM PE <i>l</i> . PROJEC	AK HOUR	E E. KAN	ie st		
A)	ADJUSTMENT FAC	TORS						
	PERCENTAGE OF PERCENTAGE OF PERCENTAGE OF DESIGN SPEED	BUSES. RECREA! MPH)	rional \	VEHICLES	0 s 0 50	TYPICAI	. – 200	#/HP)
	PEAK HOUR FACT	TION FA	CTOR		1	(WEEKDA)	COMMU'	rer)
	LANE WIDTH (FOODSTRUCTIONS.) DISTANCE (FT)				NO		i	
	терамен (РФ)	FROM R	UADWAY J	EDGE:				
	TYPE OF MULTI	LANE HI	GHWAY	• • • • • •	subi	URBAN, I	DIVIDED	
B.	TYPE OF MULTI	LANE HI	GHWAY		subi	URBAN, I	OIVIDED	
<b>B</b> )	TYPE OF MULTI	LANE HI	GHWAY		SUBI		 f	 £
B)	TYPE OF MULTI	LANE HI	GHWAY		SUBI	f w		 £
B)	CORRECTION FAC	CTORS	GHWAY	E R	SUBI	f w	f p	f E
	CORRECTION FAC	ETORS E T 1.7	GHWAY  E B 1.5	E R	SUBI	f w	f p	f E
	TYPE OF MULTING CORRECTION FACTORY TERRAIN TYPE LEVEL OPERATIONAL ASTRONOMY ************************************	ETORS T T 1.7	GHWAY  E B 1.5  RESULT	E R	SUBI	f w	f p	f E
	TYPE OF MULTING CORRECTION FACTORIST TERRAIN TYPE LEVEL OPERATIONAL AND	TORS  T  T  1.7  NALYSIS	GHWAY  E B 1.5  RESULT	E R	SUBI	f w	f p	f E
	TYPE OF MULTING CORRECTION FACTORY TERRAIN TYPE LEVEL OPERATIONAL ASTRONOMY ************************************	CTORS  T  T  1.7  NALYSIS  ******	GHWAY  E B 1.5  RESULT ******	E R	SUBI	f w	f p	f E

```
1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION.. KAAHUMANU AVENUE/(MAHALANI RD. / KANALOA AVE.)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....PROJECTED
TIME.....PM PEAK HOUR
COMMENT.....2000 WP; KAAMAH1P
                                         GEOMETRY
          VOLUMES
                            EB WB
11.0 L 11.0
11.0 T 11.0
11.0 T 11.0
                      . .
                                                             SB
                    SB :
                                                  NB
          WB NB
     EB
                                                             10.5
                                              LT 10.5 L
                    337 : L
          73
               109
LT
     45
                                                   10.5
                                                         LT
                                                             10.5
                    26 : T
               52
         857
TH
   1587
                                                             10.5
                                                   12.0
                     25 : T
              331
    47
         0
RT
                     15 : R 10.5
                                                             12.0
                                                   12.0
           0
              130
     20
RR
                             12.0
                                        12.0
                                                   12.0
                                                             12.0
                                                             12.0
                                                 12.0
                                        12.0
                             12.0
                       ADJUSTMENT FACTORS
ADJ PKG BUSES PHF PEDS
                                                        ARR. TYPE
                                              PED. BUT.
             ΗV
     GRADE
                                             Y/N
                                                   min T
            (%)
                  Y/N Nm Nb
      ( % )
                                         15
                                                   15.8
                      0
                                 0.89
                                              Y
            2.00
                            0
                  N
      0.00
EB
                                              Ŋ
                  N
                                         0
                                                   15.8
                                 0.88
WB
      0.00
            2.00
                                                         y 3
                                              N
                                                   21.8
                             0
                                 0.79
                       0
           1.00
                  N
      0.00
                                                   21.8 8 3
NB
                                         20
                                 0.86
                  N
                       0
                             0
           1.00
     0.00
SB
                      ____
                       SIGNAL SETTINGS
           _____
                                              CYCLE LENGTH = 180.0
                                           PH-1 PH-2 PH-3 PH-4
                     PH-3 PH-4
               PH-2
        PH-1
                                                   X
                                  NB LT
                X
          X
EB LT
                                      ΤH
                X
   TH
                                     RT
                X
   RT
                                      PD
                X
   PD
                                  SB LT
                X
WB LT
                                      TH
   TH
                                            X
                                     RT
   RT
                                     PD
                                            X
   PD
                                                            0.0
                    0.0
0.0
                          0.0 GREEN
0.0 YELLOW
                                           31.0
                                                31.0
                                                        0.0
         5.0 95.0
3.0 5.0
GREEN
                                                        0.0 0.0
                                           5.0 5.0
YELLOW
                         LEVEL OF SERVICE
                                             APP. DELAY
                                                         APP. LOS
                      G/C
                           DELAY LOS
             V/C
    LANE GRP.
                                                            Ε
                                       В
                                               56.1
                    0.583
                               13.3
             0.171
EB
     L
                               58.1
                                       E
                     0.539
              1.117
       Ŧ
                     Ø.539
                               6.0
                                       В
             0.045
       R
                                                             В
                                               14.1
                     0.583
                               50.9
                                       E
             0.785
WB
                               11.1
                                      В
             0.610
                    0.539
       T
                                                30.9
                                       E
                     0.183
                              51.3
             0.751
       LT
NB
                              104.7
                                       F
                    0.183
       R
              1.064
                                                58.1
                              59.5
                                       E
              0.735
                    0.183
SB
       L
                                       E
             0.840 0.183
                               57.8
       LT
                               39.1
                                       D
             0.051 0.183
       R
      _____
INTERSECTION: Delay = 47.5 (sec/veh) V/C = 1.038 LOS = E
```

•

REA NAL ATE	TYPE YST		CBD	red K Ho	UR		PAPA AVI						
					-				(	GEOMETRY			SB
			LUMES NB		SB	} 1	EB		WB		ИВ	LTR	11.0
_	EB	WB 56			16		11.0		11.0		11.0 11.0	222	12.0
T	67 1388	802			30		11.0		11.9		12.0		12.0
H	580	21			29		11.0		12.0 12.0		12.0		12.0
R	150	6			Ø	.R	10.5		12.		12.0		12.0
111						:	12.0		12.		12.0		12.0
					<b>_</b>	: 	12.0	, 					
						A	DJUSTHEN	T FAC	TORS	577	BUT.	ARR	TYPE
	an.	מחת	нν	AD	J PK		BUSES	PHF	PEDS	Y/N	min T	HILL.	
		ADE %)	(%)	Y/1			Ир		0		10.9		5
EB	ò		2.00	N		0	Ø	0.87	0 20		10.9		5
WB	ő		2.00	N		Ø	Ø	0.90	10		22.1		3
NB	ø	.00	1.00	N		0	0	0.96 0.79	10		22.1		3
SB	Ø	.00	0.00	N		0	0						120 0
						SIG	NAL SET	TINGS			LE LENC	9TH = PH-3	120.0 PH-4
		PH-	1 PH-	-2	PH-		PH-4				H-2	-11-3	
	LT	X		ĸ –				NB	LT	X X			
EB	TH	•-		X					TH	x			
	RT			X					RT PD	X			
	PD			X				CB.	LT	ž.			
WB	LT	X		X				SB	TH	X			
	TH			X					RT	X			
	RT			X					PD				
	PD			X		. Ø	0.0	GRE		38.0	0.0	0.0	0.0
GRE		5.				.0	0.0		LOW	5.0	0.0	0.0	0.0
AEI	LOW	3.	0 5	.0									
							TEAET OF	SERV	ICE	200	DELAY	APP	. LOS
	Ţ. A 1	NE GRP	. V/			/C_	DEI		LOS B	22	. 8		C
EB		L	0.2			617		7.8	Ď				
		T	0.9	79		550		5.9	B				_
		R	0.6			550		1.4 0.2	В	7	. 4		В
WB		L	0.3			617		7.2	3				_
		TR	0.9			550		3.9	č	25	. 2		D
		_		1/2/0	ω.	333							
ΝВ		L LTR	0.4	558		333		6.2	D		2.3		С

```
1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION.. KAAHUMANU AVENUE/(WAKEA AVE. / M.C.C)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....PROJECTED
TIME.....PM PEAK HOUR
COMMENT.....2000 WP; KAAWAK1P
______
                     GEOMETRY

SB: EB WB

43: L 11.0 L 11.0 LT

77: T 11.0 T 11.0 P

67: T 11.0 T 11.0

15: R 10.5 R 10.5
           VOLUMES
                                                      NB
                                                                    SB
     EB
           WB NB
                                                                   10.5
                                                              LTR
                 197
                                                        10.5
    87
           140
LT
                                                                    12.0
                                                        10.5
TH
   1057
           583
                 57
                                                                    12.0
                                                        12.0
           81
                 100
RT
    337
                                                                    12.0
                 50
                                                        12.0
      90
           20
RR
                                12.0
                                             12.0
                                                        12.0
                                                                    12.0
                          :
                                                        12.0
                                                                    12.0
                                12.0
                                            12.0
                          ______
             ADJUSTMENT FACTORS

HV ADJ PKG BUSES PHF PEDS

(%) Y/N Nm Nb
                                                  PED. BUT. ARR. TYPE
      GRADE
                                                   Y/N min T
      ( % )
                                                        10.6
10.6
                   N 0 0
N 0 0
N 0 0
N 0 0
                               0
                   N
                                              0
                                                   N
                                    0.87
            2.00
EB
      0.00
                                                                   5
                                              15
             2.00
                                     0.93
                                     0.93 15 1
0.84 15 Y
0.67 15 Y
                                                    Y
                                                        24.4
24.4
      0.00
WB
            1.00
      0.00
NB
                                    0.67
            0.00
SB
      0.00
            SIGNAL SETTINGS CYCLE LENGTH = 90.0
PH-3 PH-4 PH-1 PH-2 PH-3 PH-4
                       PH-3 PH-4
         PH-1
                PH-2
                                      NB LT
                                                X
                 X
EB LT
           X
                                                  X
                                          TH
    TH
                  X
                  X
                                          РT
                                                  X
   RT
                                          PD
                                                  Х
                  X
   PD
                                      SB LT
WB LT
           X
                  Х
                                          TH
    TH
                  X
                  X
                                          RT
   RT
                  X
                                         PD
   PD
                            0.0 GREEN
0.0 YELLOW
                                               35.0 0.0 0.0 0.0
5.0 0.0 0.0 0.0
         5.0 37.0 0.0 0.0
3.0 5.0 0.0 0.0
                      0.0
GREEN
YELLOW
LEVEL OF SERVICE
LANE GRP. V/C G/C DELAY LOS
                                                               APP. LOS
                                                  APP. DELAY
                                                                   В
               0.214
                      0.522
                                 8.9
                                          3
                                                  15.0
EB
       Τ.
                       0.433
                                           С
               0.946
                                  17.5
        Т
                                          В
       R
               0.507
                       0.433
                                   5.9
                                                                   8
                                                     8.6
WB
               0.712
                       0.522
                                  23.3
                                          С
       L
                                  5.7
               0.488
                       0.433
                                           3
        Т
        R
               0.117
                       0.433
                                  4.6
                                           A
                                  16.3 C
10.6 B
19.9 C
                                                    15.4
NB
       LT
               0.675
                      0.411
               0.111
                       0.411
        R
                      0.411
       LTR
                                                    19.9
SB
               0.754
              Delay = 13.7 (sec/veh) V/C = 0.839 LOS = B
INTERSECTION:
```

-

All Maria

77 15

4

		CTIO		KAAHUM.	ANU A	VEN	UE/(	KANE	RD	. /	BE	ACH :	RD.	}			•		
				PROJEC															
				PM PEAT															
COI	IMENT		• • • •	2000 W	P; KA	AKA	NIP												
				LUMES										METRY					
	F	В		NB	9	я :		EB				WB			ı Ne			SB	
LT				84				11	. 0	L		11.0	o .	I.			ī.	11	
				179				11	. Ø	T		11.6	ō	TR	11	. 0	LT	11	
RT	11	4	0	117	15	2 :	T	11	. 0	T		11.0					R		
R	5	5	0	95	9	0:	R	10	. 5			12.0						12	.0
						:		12	. 0			12.0				2.0			. 0
						:		12	. 0			12.0	2		12	. 0		12	. 0
							T.OA	JSTM	יים יים אים		CTO								
	G	RADE		HV	ADJ	PKG	D114	700		OHE	,	0000		PED.	Bü	TT.	ARR	TY	PE
		(%)	(	(%)	Y/N	Nm	1	1b			•		3	7/N	mi	n T	ARR		
B		0.00		2.00	N	0		0	0	.94		15		Y	1	6.4		5	
IB		0.00	2	2.00 2.00 1.00	N	0		0	0	.94		0		N	1	6.4		5	
IB		0.00	1	1.00	N	0		0	0	.95		15		Y	2	1.8		3 .	
B		0.00	, ž	2.00	N 	0		0	9	. 94		15		Y	2	1.8		3	
	- 7 - 7							SET								~~~	 3TH =		<b>-</b>
		Pl	- 	PH-2	. P	H-3	Pi										917 - PH-3		
B	LT	•	X	X							LT	_	-	_	x	•	•		•
	TH			X							TH				X				
	RT			X							RT				X				
	PD			X							PD				X				
IB	LT TH		Х	X X						5B	LT		X						
	RT			X							TH		X X						
	PD			X							PD		Λ.						
RE	EN	5	5.0	40.0		0.0	ø	.0	(	REE	7 N	3	9.0	1	8.0		0.0	0.	a
	LOW			5.0				.0	3	ELI	OW		5.0	, -	5.0		0.0		
	* * *	ID 07	ח	W 40		- ,~		LOF		RV.				<b>.</b> -					
В	LAI	L L	CP.	V/C 0.207		9/C .417		DEL	AY .1			S	AP	P. D		Y	APP.		Š
<b>D</b>		T T		1.046		350			. 2		O و			36.	כ			D	
		Ř		0.139		350			.1		2								
		Ľ		0.229		417			. 4		ā			13.	7			В	
В		T		0.701		350			. 5		B			- <b>-</b> ·				_	
В		L		0.357	0.	167		34			D			40.	o			D	
ВВ		TR		0.826		167		42			E								
В		TIL							_		_			40.0	2			_	
		L LT		0.825 0.980		342		35 46			D E			40.	0			Ε	

					FM NG T	RAFI	TC VOI	LUMES	( K.						
LT TH	EB 63 1184 378	VOL WB 168	UMES - NB 259 48 132	\$1 5° 6'	: 3 :	L T T R	EB 11.0 11.0 10.5 12.0 12.0	AFFR			gonat Le R	N ! 1	B . 5 . 6 . 6 . 6 . 6 . 6 . 6 . 6 . 6 . 6		S 10 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
 EB WB NB	(° 0 0	ર) ( .00 2	%) .00	N N	M M O	BU:	ISTIEN	FHF 0.87	P	EDS .) 15	A !! A\;		BUT. nin T 10.6 10.6 24.4	ta v	5 3 3
EB		PH-1 X	90 90 90 PH-2 X	 P	53 H-3	GNA P	L SETT H-4	INGS NB	TH RT	P	X X	PH CACT:	e len -2	GTH = PH-3	90 PH
WB	PD LT TH RT	X	X X X X X		٠			SB	PD LT TH RT PD		X X X				
GRE YEL	PD EN LOW	5.0 3.0	37.0 5.0		0.0 0.0		0.0 0.0 	YEL	en Low		5.0 5.0		.0 .ი 	Ø.0 Ø.0	0 3 
 EB		E GRP. L T	V/C 0.174 1.066	. 0	G/C ).52	LEV 2 3	EL OF DELA 3. 34.	SERV 7 6	ICE LO	05 2 0		. DE 23.4		APP	. LO
WB		R L T R	0.593 0.853 0.573 0.094	9 6	.43 .52 .43	2 3 3	40. 6. 4.	4 : 6		3 8 9 8 C		12.1			2
ne se		LT R LTR	0.768 0.133 1.08	3 6	0.41 0.41 0.41	1	19. 10. 85.	. 9	!	e F		85.6			<u> </u>
	ERSEC	TION:			<del>-</del>	26.0	(sec	/veh)		V/C :	= 1.0	52	L	)S = D	

```
1985 HCM: SIGNALIZED INTERSECTIONS
SUMMARY REPORT
INTERSECTION..KAAHUMANU AVENUE/(KANE RD. / BEACH RD.)
AREA TYPE....CBD
ANALYST.....PGP
DATE.....09/29/92
TIME.....4:00 - 5:00 P.M
COMMENT.....1992 EXISTING TRAFFIC VOLUMES (KAAKAMPM)
                                                 GEOMETRY
            VOLUMES
                           :
                                                                      SB
                                                          ΝB
                                              WB
                        SB :
                                  EB
            WB
                NB
      EB
                                                                      11.0
                                                          11.0
                                              11.0
                       807 : L
                                  11.0
                 92
            50
                                                                      11.0
LT
      74
                                                          11.0
                                                     TR
                                         T
                                              11.0
                                  11.0
                       106 : T
                 181
           732
    1158
                                                                 R
                                                                      11.0
TH
                                                          12.0
                                              11.0
                                         Ţ
                       158 : T
                                  11.0
            0
                 126
     130
                                                                      12.0
RT
                                                          12.0
                                              12.0
                                  10.5
                        90 : R
                 95
                                                                      12.0
      55
RR
                                              12.0
                                  12.0
                                                                      12.0
                                                          12.0
                                              12.0
                                  12.0
                           ADJUSTHENT FACTORS
                                                                 ARR. TYPE
                                                     PED. BUT.
                                           PEDS
                     ADJ PKG EUSES
                                     PHF
               ΗV
      GRADE
                                                          min T
                                                    3/7
                                Иb
                     Y/N Nm
              (왕)
                                                                     5
       (名)
                                                           16.4
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                                      0.94
                          Ø
                                 0
              2.00
                      N
       0.00
                                                                     5
EB
                                                           16.4
                                               0
                                                     N
                                      0.94
                           0
                                 0
                      N
              2.00
       0.00
                                                                     3
WB
                                                           21.8
                                               15
                                      0.95
                           Ø
                                 0
              1.00
                      N
       0.00
ΝВ
                                                           21.8
                                                     Y
                                      0.94
                                               15
                                Ø
                           Ø
              2.00
       0.00
SB
                                                    CYCLE LENGTH = 120.0
                          SIGNAL SETTINGS
                                                               PH-3
                                                                      PH-4
                                                        PH-2
                        PH-3 PH-4
                 PH-2
          PH-1
                                       NB
                                           LT
                   X
            X
    LT
EB
                                           TH
                   Х
    TH
                                           RT
                   X
    RT
                                           50
                    Х
    PD
                                        SB
                                           LT
    LT
            Х
WB
                                            TH
                                                    Х
                    X
     TH
                                                    X
                                           RT
                    X
    RT
                                            ЪD
                                                                        0.0
                    X
                                                                 0.0
    PD
                                                         18.0
                                                  35.0
                                        GREEN
                                0.0
                         0.0
                  44.0
           5.0
                                                                       0.0
 GREEN
                                                         5.0
                                                                 ହ.ଠ
                                                   5.0
                                       AETFOM
                               0.0
                 5.0
                         0.0
            3.0
 YELLOW
                         ______
                             LEVEL OF SERVICE
                                                                  APF. LOS
                                             LOS
                                                    APP. DELAY
                         G/C
                                 DELAY
                 V/C
      LANE GRP.
                                                       31.2
                                              С
                         0.450
                                   15.6
                 0.234
        L
                                              D
                                    33.6
                         0.383
                 1.027
         T
                                              ā
                         0.383
                                     7.4
                 0.161
         R
                                                       12.5
                                              C
                         0.450
                                    16.9
                 Ø.326
         \mathbf{L}
 WB
                                              Ε
                                    12.3
                 0.686
                         0.383
                                                                      Ξ
         Т
                                                       43.€
                                              D
                                    34.4
                         0.167
                 0.391
 NB
         L
                                              Ε
                                    47.5
                         Ø.167
                 0.876
         TR
                                    51.1
                                              Ε
                         0.308
                 0.942
                                    89.2
                         0.308
                 1.113
                                              C
                                    19.6
                         0.308
                 0.177
         Ŕ
                    Delay = 38.5 (sec/veh) V/C = 0.978
                                                              LOS = D
 INTERSECTION:
```

# Appendix C

Drainage Report

## PRELIMINARY GRADING AND DRAINAGE REPORT

FOR

MAUI COMMUNITY COLLEGE

BUILDINGS "J", "S" AND SITE IMPROVEMENTS

KAHULUI, MAUI, HAWAII

TMK: 3-8-7:40

PREPARED FOR

DEPARTMENT OF ACCOUNTING AND GENERAL SERVICES

DIVISION OF PUBLIC WORKS

STATE OF HAWAII

BY

AUSTIN, TSUTSUMI & ASSOCIATES, INC. ENGINEERS \* SURVEYORS

WAILUKU \* HILO \* HONOLULU, HAWAII SEPTEMBER 1992 REVISED SEPTEMBER 23, 1992

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III.	EXISTING CONDITIONS	2-3						
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IV.	V. GRADING AND DRAINAGE PLAN							
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APPEN	DIX							
	Hydrologic Calculations							

TED S. KAWAHIGASHI, P.E. GEORGE M. NEUFFER, P.E. KENNETH K. KUROKAWA, P.E. THOMAS S. OTAGURO IVAN K. NAKATSUKA, P.E. KEN K KUROKAWA PE vice President & Maul Branch Office Manager

#### PRELIMINARY GRADING AND DRAINAGE REPORT

FOR

#### MAUI COMMUNITY COLLEGE

#### BUILDINGS "J", "S" AND SITE IMPROVEMENTS

#### I. INTRODUCTION

The purpose of this report is to evaluate the existing site drainage conditions and to develop a preliminary grading and drainage plan for the proposed project.

#### II. PROPOSED PROJECT

#### A. LOCATION

The project site is located within the Maui Community College (MCC) campus in Kahului on the island of Maui, Hawaii. The MCC campus is located on the Makai side of Kaahumanu Avenue across the Kaahumanu Shopping Center. The campus site encompasses 61 acres and is designated by Tax Map Key Number 3-8-7, Parcel 40. (See Exhibits 1 & 2 for the location of the MCC campus.)

#### B. PROJECT

The proposed project consists of two new classroom buildings, a new parking lot area and a new storm drainage retention basin. (See Exhibit 2A for the location of the

proposed improvements.)

#### III. EXISTING CONDITIONS

#### A. TOPOGRAPHY AND SOIL CONDITIONS

The general slopes of the MCC campus ranges from 0.5 percent at the eastern part of the campus to 2 percent at the western end. The majority of the campus slopes toward the northeast to Kahului Beach Road and the remaining area slopes toward the east. Onsite elevations range from 8 feet to 50 feet MSL (mean sea level). The MCC property is presently covered with buildings, sidewalks, parking areas, tennis courts, cultivated agricultural areas, and open grassed and overgrown areas. The soil classification for the MCC area is Puuone Sand (PZUE) as described by the USDA Soil Conservation service ("Soil Survey of the Islands of Kauai, Oahu, Maui, Molokai and Lanai").

#### B. CLIMATE

Kahului is generally sunny and warm throughout the year. The annual rainfall is about 20 inches with a mean annual temperature of 75 degrees Fahrenheit.

#### C. DRAINAGE

Presently, the majority of the onsite runoff sheets flows across the campus towards Kahului Harbor and under the Kahului Beach Road via two off-site drainage ditch and drainline systems. The remaining on-site runoff drains towards the east. An existing 72" x 44" arch pipe, which collects approximately 16 acres of offsite drainage from Kaahumanu

Avenue, crosses the campus and outlets into onsite drainage ditch which connects to the existing offsite ditch and drainline system.

#### D. FLOOD ZONE

The Flood Insurance Rate Map (FIRM) for the area indicates that the majority of the campus is within Zone C, which is an area of minimal flooding. The remaining portion of the site lies within zone A4, which has a base flood elevation of approximately 16 feet MSL. (See Exhibit 3 for flood map).

#### IV. GRADING AND DRAINAGE PLAN

#### A. GRADING PLAN

The proposed grading plan will require embankment to bring the finish floor elevations of the proposed buildings to elevation 37 feet MSL for Building "J" and elevation 15 feet MSL for Building "S". The proposed parking area will also require embankment to bring the pavement surface to required finish grades which vary from elevation 12 feet MSL to 20 feet MSL. The site will be graded to dispose of the onsite storm runoff generated from the proposed improvements as shown in Exhibit 4 (Grading and Drainage Plan). An offsite storm drainage retention basin will be constructed in the area between the MCC campus and Kahului Beach Road to retain the difference in runoff between existing and proposed site conditions for the entire MCC campus. Erosion control measures will be incorporated during the construction period to minimize soil loss.

#### B. DRAINAGE PLAN

The proposed drainage plan for the Buildings "J", "S" and Site Improvements project will involve two underground drainage collection systems. Both systems will consist of inlets, catch basins, and underground piping which will discharge into the offsite storm drainage retention basin system. The retention basin system will consist of a 4 feet deep pond with a two feet of freeboard, approximately 140 feet wide and 900 feet long. The pond will retain the difference in runoff volume between the existing onsite runoff and the onsite runoff generated from the proposed improvements. Offsite runoff will continue to pass through the campus and will be directed through the retention basin before ultimate The ultimate discharge flow disposal into Kahului Harbor. into Kahului Harbor will be limited to not exceed existing discharge levels. The retention basin emergency spillway will be designed to control the discharge rate. See Exhibit 4 for the proposed drainage systems and retention basin system.

#### C. HYDROLOGY

The Rational Method as described in the "Storm Drainage Standards", May 1988, by the City and County of Honolulu was used to compute the storm water runoff quantity. Runoff calculations were based on a 10 - year, 50 - year and 100 - year storm recurrence intervals. The rainfall intensities were interpolated from the "Rainfall Frequency Atlas of the Hawaiian Islands", by the U.S. Department of Commerce, Weather

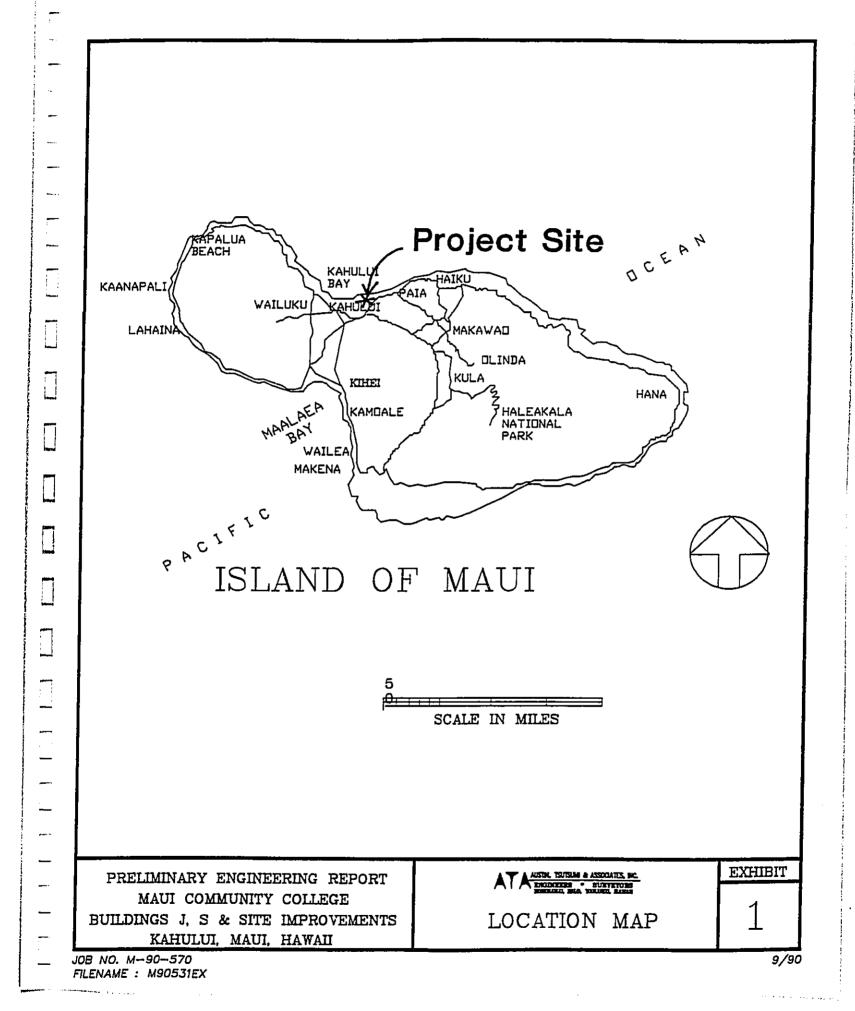
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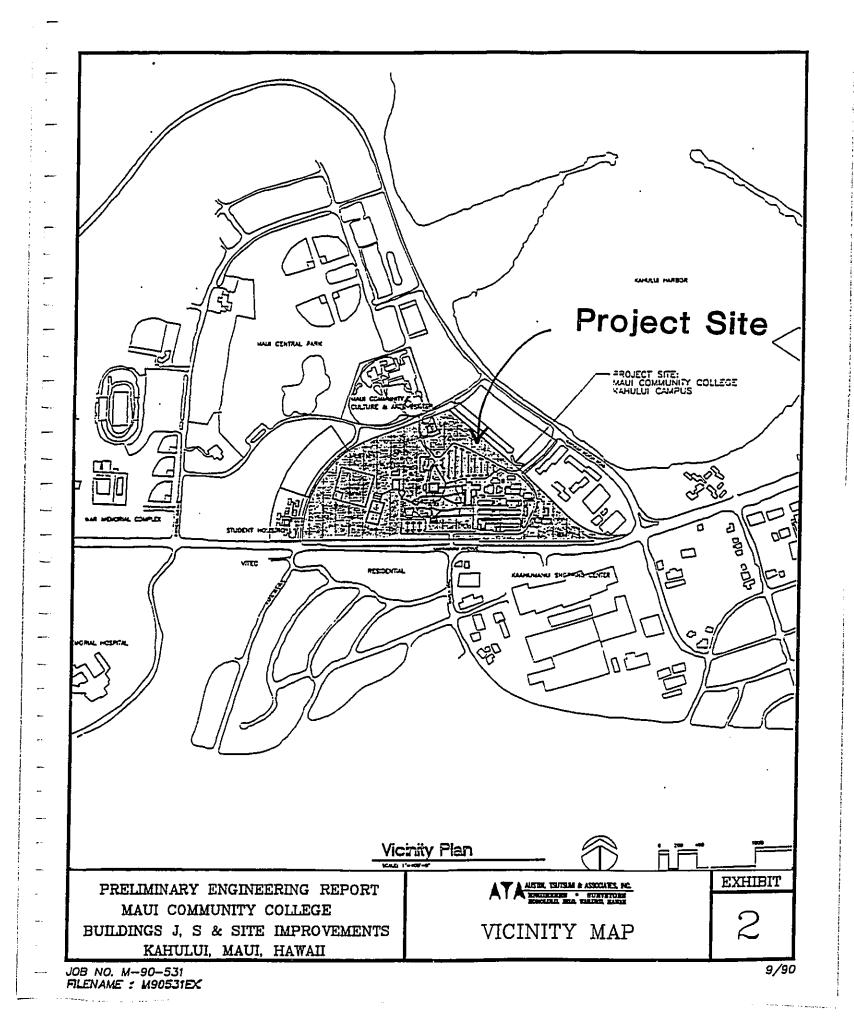
The existing onsite storm runoff for the entire MCC campus for the 10 year storm event is approximately 91 CFS. For the improved site conditions, the projected runoff for the entire MCC campus is 146 CFS. The existing offsite runoff is 26 CFS. The difference in runoff volume for the 10 year storm is 3.0 acre-feet which includes the offsite drainage area. The retention pond also detains the 100 year storm difference in runoff volume of 5.5 acre-feet. See Appendix for site drainage and retention basin calculations. See Exhibit 5 for Drainage Sub basins.

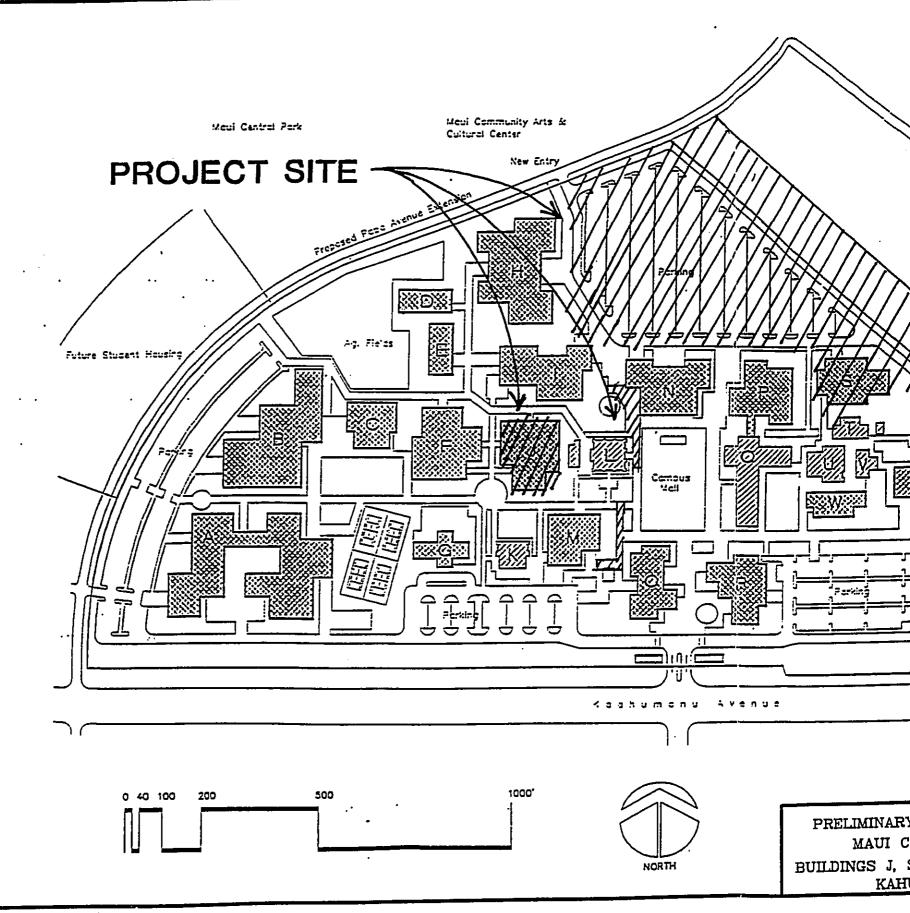
#### IV. CONCLUSION

The proposed grading and drainage design for this project will produce no adverse effect by storm runoff to adjacent and downstream properties. The two proposed buildings will be designed to be constructed within the campus area designated by the FIRM as Zone C. The proposed retention basin system will retain the increase of runoff generated from the proposed ultimate improvements and will regulate the ultimate discharge rate into Kahului Harbor to not exceed existing values. Soil loss will be minimized during the construction period by implemented appropriate erosion control measures. Dust will also be minimized during construction by the implementation of water sprinkling. All drainage improvements will conform to the County Standards and will be coordinated with the Department of Public Works, County of Maui.

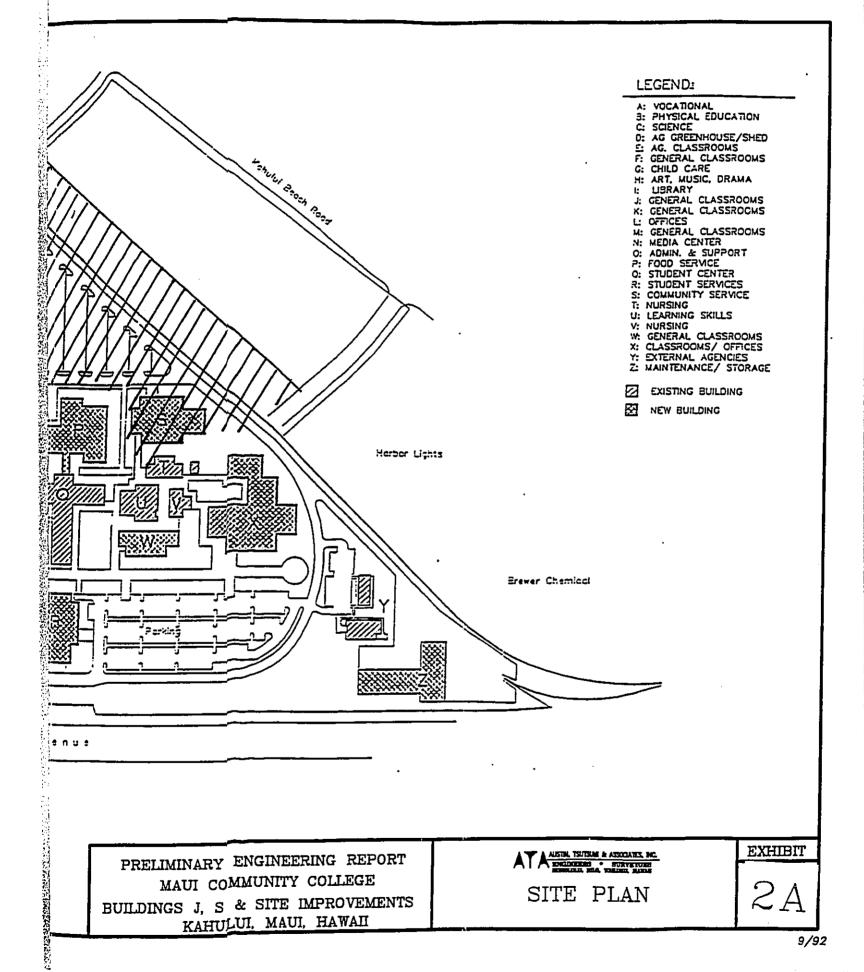
# **EXHIBITS**



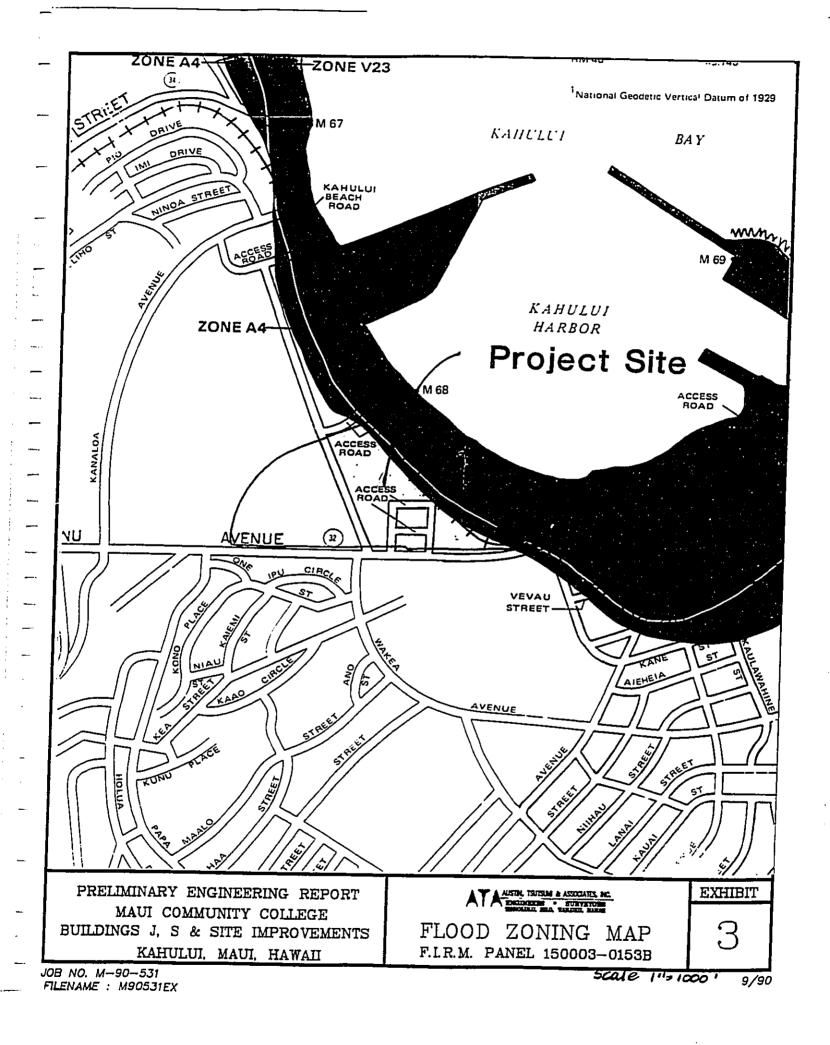


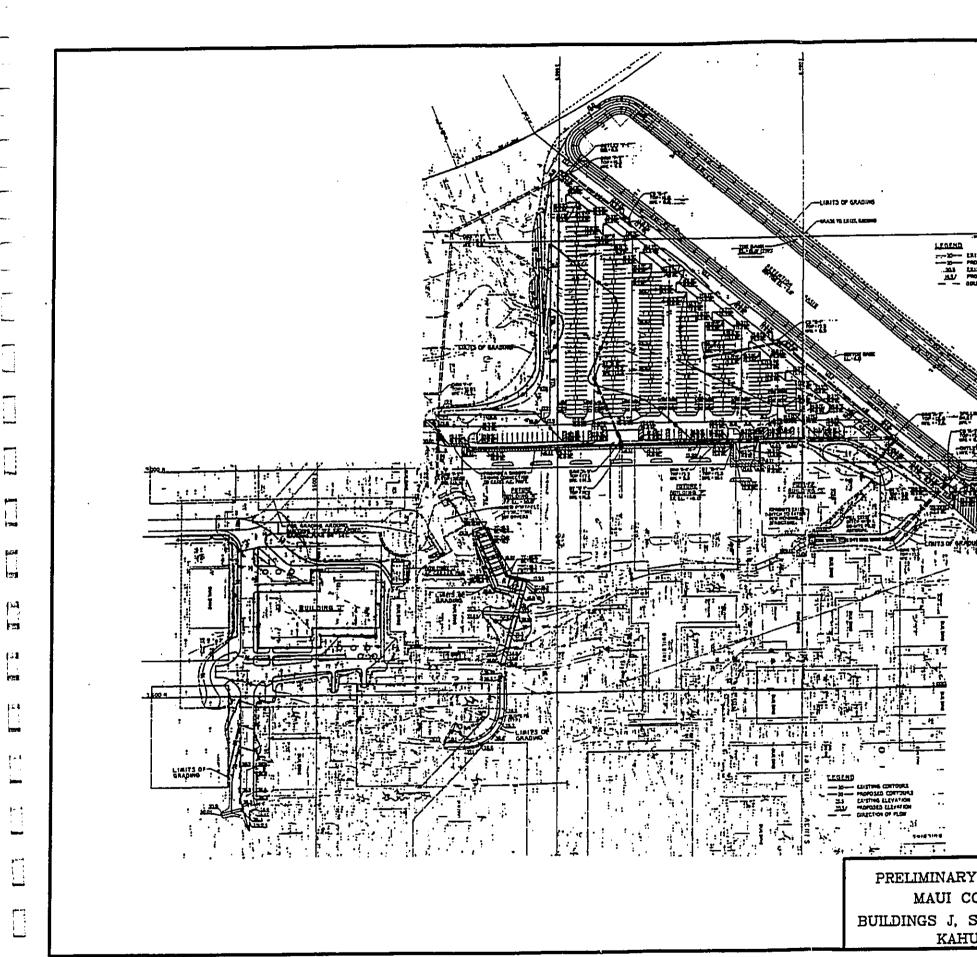


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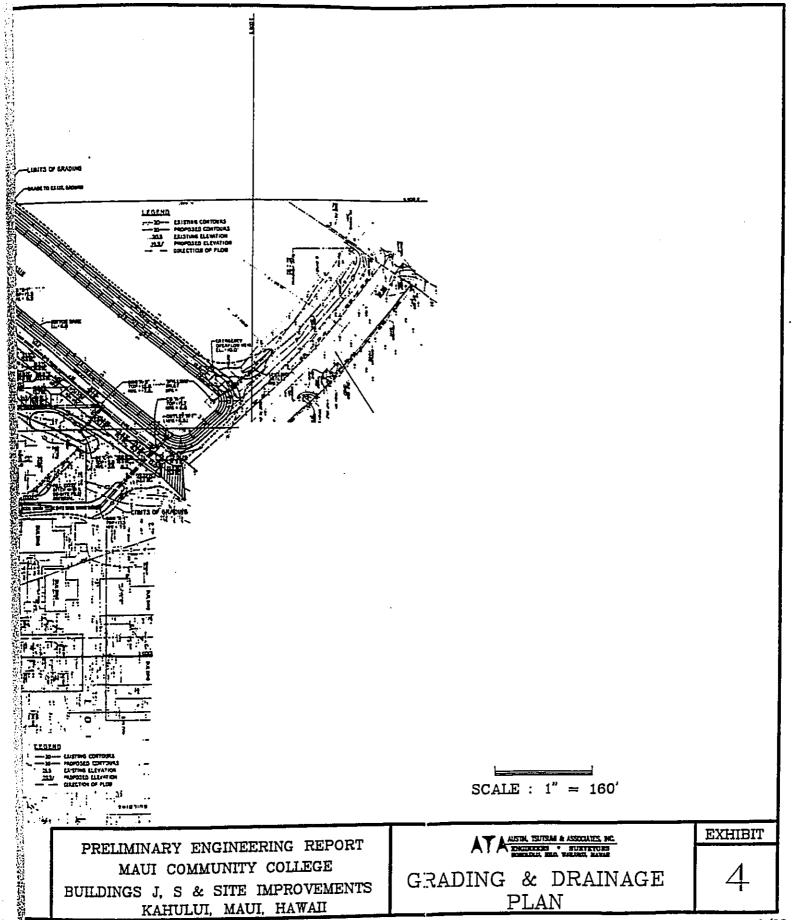


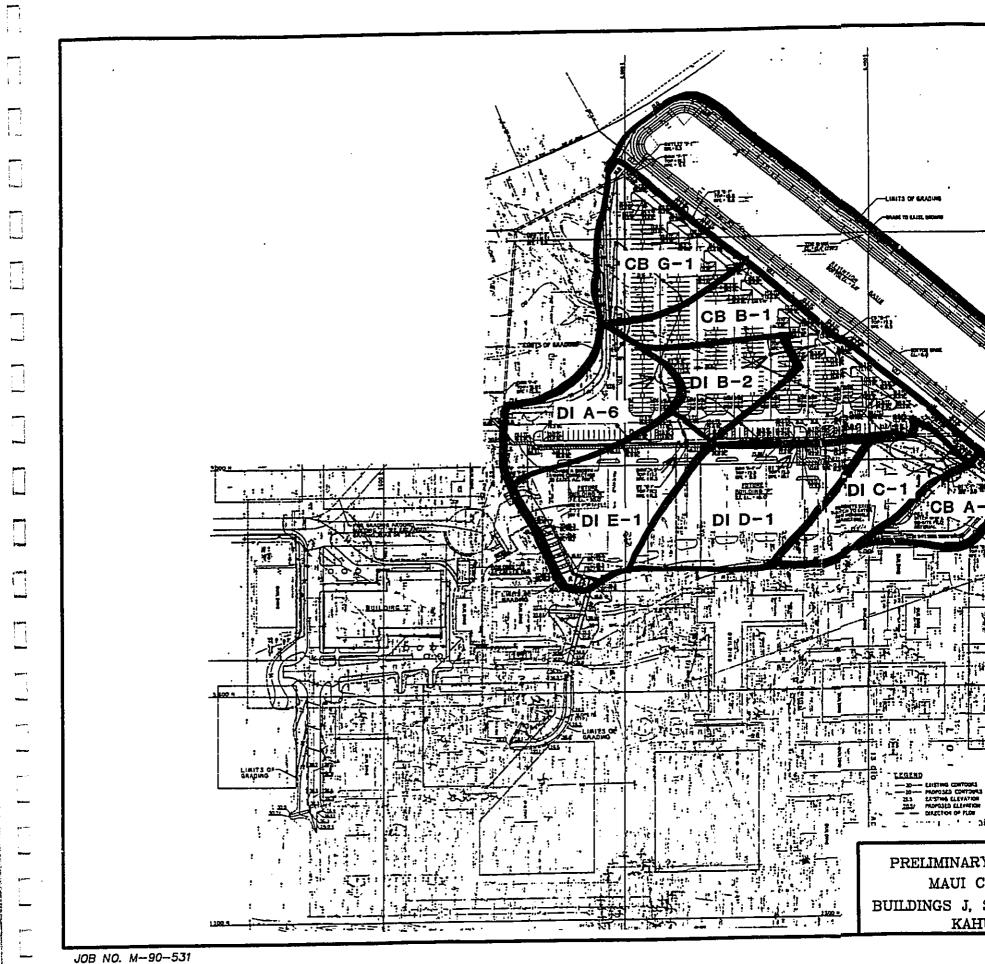
KAHULUI, MAUI, HAWAII



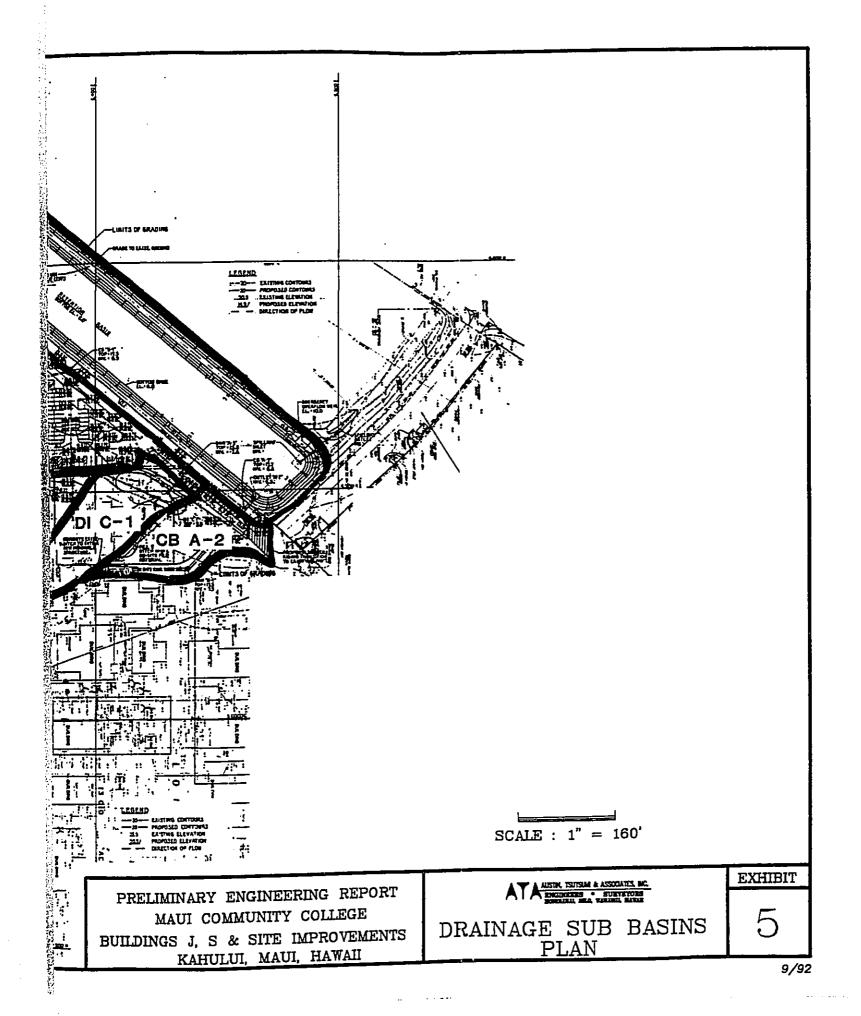


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# **APPENDIX**

VERSION 1.11

### TR-55 GRAPHICAL DISCHARGE METHOD

Toject: MAUI COMMUNITY COLLEGE
Wounty: MAUI-CENTRAL State: MI
Subtitle: EXISTING CONDITIONS- ONSITE User: lvt Date: 08-23-92

Checked: ...... Date:

Acres

Hours

nata: Drainage Area : 55.8
Runoff Curve Number : 83
Time of Concentration: 0.42
Rainfall Type : I
Pond and Swamp Area : NONE

				======			
Storm Number	1	2	3	4	5	6	7
frequency (yrs)	   J	2	5	1 0	25	5.0	1 100
24- Hr Rainfall (in)	   2.6	   3   !	5	! ! 5 !	   7.5   	្ [ ន	8.8 (
Ta/P Ratio	0.16	0.14	0.08	0.08	0.05	0.05	0.05
U s. e. d	0.16	0.1.4	0.10	0.10	0.10	0.10	0.10
Runoff (in)	1.13	1.45	3.17	1   3.17 	5.50	5.98	6.74
Unit Peak Discharge (cfs/acre/in)	0.434	0.449	0.475	0.475	0.475	0.475	0.475
Pond and Swamp Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Poak Discharge (cfs)	27	36	84	84 =======	146	158	3.79

VERSION 1.11

## TR-55 GRAPHICAL DISCHARGE METHOD

F oject: MAUI COMMUNITY COLLEGE
county: MAUI-CENTRAL State:
Subtitle: EXISTING CONDITIONS- ONSITE Date: 08-28-92 User: lvt Date: ..... State: HI Checked: \_\_\_\_\_

Data: Drainage Area

Drainage Area : 5.2 Acres
Runoff Curve Number : 80
Time of Concentration: 0.42 Hours
Rainfall Type : I
Pond and Swamp Area : NONE

Hours

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	1. 1.	2	3	4	S (			
storm Number		i						
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Frequency (yrs)						1		
<u> </u>		3 1	5 Ì	i 5 1	7.5	[ ধ i	8.8	
24-Hr Rainfall (in)	2.6	3 1				i		
24-H1 Kuziii 1	1			l i			0.06	
		0.17	9.10	3.10	0.07	9.06	0.00	
Ia/P Ratio	0.19	0.17		i		l <sup>2</sup>	<u> </u>	
	1					0.10	j Ø.10 l	
44	0.19	0.17	0.10	0.10	0.10	0.70		
Used	1 0.13		1	1	1		1	
i promije	Į	<b>l</b>		2.89	5.16	5.63	6.38	
11- 22-13-1	0.96	125	2.89	1 2.09	1 3.20			
Runoff (in)			Ì	l		}		
!	l	!	1 475	0.475	0.475	13.475	0.475	
munit Peak Discharge	10.409	0.428	0.475	10.4/5	1		i	
minute hear programme	i -	Ì	1	1	)	ļ	!	
(cfs/acre/in)	1	:	i	1	<u> </u>	Į.	]	
t <del>ani</del>	]	ļ	1	1.00	i 1.00	1.00	1.00	
	1.00	1.00	1.00	1 1.00	1	1		
Pond and Swamp Factor		i	ĺ	1	1	1	ļ.	
0.0% Ponds Used	l	!	}	i	1			
1				7	13	1.4	1.6	
	i 2	i 3	7	/	1 13	1 3.4		
Peak Discharge (cfs)	-					======		

VERSION 1.11

## TR-55 GRAPHICAL DISCHARGE METHOD

roject: MAUI COMMUNITY COLLEGE
County: MAUI-CENTRAL State: HI
Subtitle: EXISTING CONDITIONS- OFFSITE User: lvt Date: 08-28-92 Date: .... Checked:

Hours

Data: Drainage Area : 16
Runoff Curve Number : 84
Time of Concentration: 0.37
Rainfall Type : I
Pond and Swamp Area : NONE

		***					
Storm Number	1.	1 2	3 	1 4	5 	6 	7   
Frequency (yrs)	] 1	2	5	1.0	25	50	1.00
24 Hr Rainfall (in)	   2.6	   3	   5	5	7.5	   8	ត្រ.៩
Ta/P Ratio	0.15	0.13	0.08	0.08	0.05	0.05	0.04
ੀ Used	0.15	0.13	0.10	9.10	0.10	0.10	0.10
Runoff (in)	1.19	1.52	3.27	   3.27	5.62	6.10	6.87
Unit Peak Discharge (cfs/acre/in)	  0.466 	0.481	0.501	0.501	0.501	0.501	0.501
Pond and Swamp Factor 0.0% Ponds Used	1.00   	1.00	1.00	1.00	1.00	1.00	1.00
Peak Discharge (cfs)	9	12	26	26	45	49	55

#### TR-55 GRAPHICAL DISCHARGE METHOD

VERSION 1.11

oject : MAUI COMMUNITY COLLEGE
County : MAUI-CENTRAL State: HI
Subtitle: DEVELOPED CONDITIONS- ONSITE User: lvt Checked: \_\_\_\_\_ Date: 08-28-92 Date:

Data: Drainage Area : 61
Runoff Curve Number : 92
Time of Concentration: 0.25
Rainfall Type : I
Pond and Swamp Area : NONE

		======	A=####	======	*****	======			
	Storm Number	1	2	1 3	4	5	6	   7	=
	Frequency (yrs)	1		   5	10		50	1 100	1
	24-Hr Rainfall (in)	2.6	   3 	   5 	5	7.5	   8 	   8.8 	
	Ja/P Ratio	0.07 	0.06	0.03	0.03	0.02	0.02	0.02	1
İ	Used	0.10	0.10	0.10	0.10	0.10	0.10	0.10	   
ţ	™ Runoff (in)	1.79	2.16	4.09	4.09	6.55	7.04	7.84	i   
	Unit Peak Discharge (cfs/acre/in)	0.584	0.584	0.584	0.584	0.584	0.584	0.584	   
da Ta	Pond and Swamp Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
į	Peak Discharge (ofs)	64	77	1.46	146	233	251.	279	

	TR-55 STORAGE VOLUME FOR	DETENTION BASINS	VERSTON 1.11
ປະປຸດ	ject : MAUI COMMUNITY COLLEGE nty : MAUI-CENTRAL State: HI title: RETENTION VOLUME : 10-YR STORM	User: lvt Checked:	Date: 08-28-92 Date:
	Rainfall-Type: I	Rainfall Frequency:	
	<pre>24-Hour Rainfall: 5 inches Peak Inflow: 172 cfs Peak Outflow: 110 cfs</pre>	Runoff Cur∨e Number:	: 92
	Runoff Volume: 4.1 inches Detention Basin Storage Volume: 0.59	inches or 3.0 acre	feet
		;	

				TR-55	STORAGE	VOLUME	FOR	DETENTIO	N BA	SINS	VER	SION 1.11
	Cou	inty :	MAUI-0	CENTRAL	TY COLLE	State:	HI ORM			lvt	Date: Date:	08-28-92
			age Are		. Acres			Rainfa	11 F	requency:	50 ye	ers
1000	Ţ	21-Ho Peak	all-Typur Rair Inflow: Outflow	nfall: : 300	8 inch cfs cfs	es		Runoff	Cur	∨e Number	: 92	
Selection Supplies Light	75.4	Runof	f Volum	ne:	7.0 inch	es lume:	0.98	inches o	r	5.0 acre	feet	
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ووجران	ject : MA nty : MA	UI-CENT	RAL	S	tate:	нІ			1vt			08-28-92
Sub	title: RE			Acres			Rainf	all 8	Freque	ncy:	100 y	ears
	Rainfall 24 Hour Peak Inf	-Type: Rainfal low: 3	I 1: 8.	8 inch s	ies .		Runof	f Cui	rve Nu	mber:	92	
	Peak Out Runoff V Detentio	_			ime: 1	. 08	inches	or	5.5	acre	feet	
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