Masunaga Scawall

### DEPARTMENT OF LAND UTILIZATION

# CITY AND COUNTY OF HONOLULU

650 SOUTH KING STREET, 7TH FLOOR + HONOLULU, HAWAII 96813 PHONE: (808) 523-4414 . FAX: (808) 527-6743



LORETTA K.C. CHEE DEPUTY DIRECTOR

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DIRECTOR

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December 11, 1997

OFC. O. CONTRACTOR

The Honorable Gary Gill, Director Office of Environmental Quality Control State of Hawaii State Office Tower, Room 702 235 South Beretania Street Honolulu, Hawaii 96813

Dear Mr. Gill:

JEREMY HARRIS

MAYOR

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# CHAPTER 343, HRS Environmental Assessment (EA)/Determination Finding of No Significant Impact

Recorded Owner : Applicants :		Harold H. and Pauline L. Masunaga Trust Harold H. and Pauline L. Masunaga
	:	Group 70 International, Inc.
	:	68-705 Farrington Highway, Mokuleia, Oahu
Tax Map Key :	:	6-8-10: 26
Request :	:	Shoreline Setback Variance
Proposal :	:	Replacement of an existing unauthorized deteriorated vertical concrete masonry unit (CMU) seawall with a modified vertical seawall structure
Determination :	:	A Finding of No Significant Impact is Issued

Attached and incorporated by reference is the Final EA prepared by the applicant for the project. Based on the significance criteria outlined in Chapter 200, State Administrative Rules, we have determined that preparation of an Environmental Impact Statement is not required.

The Honorable Gary Gill, Director Page 2 December 11, 1997

We have enclosed a completed OEQC Bulletin Publication Form and four copies of the Final EA. If you have any questions, please contact Steve Tagawa of our staff at 523-4817.

Very truly yours,

DAN NACE SULLIVAN Director of Land Utilization

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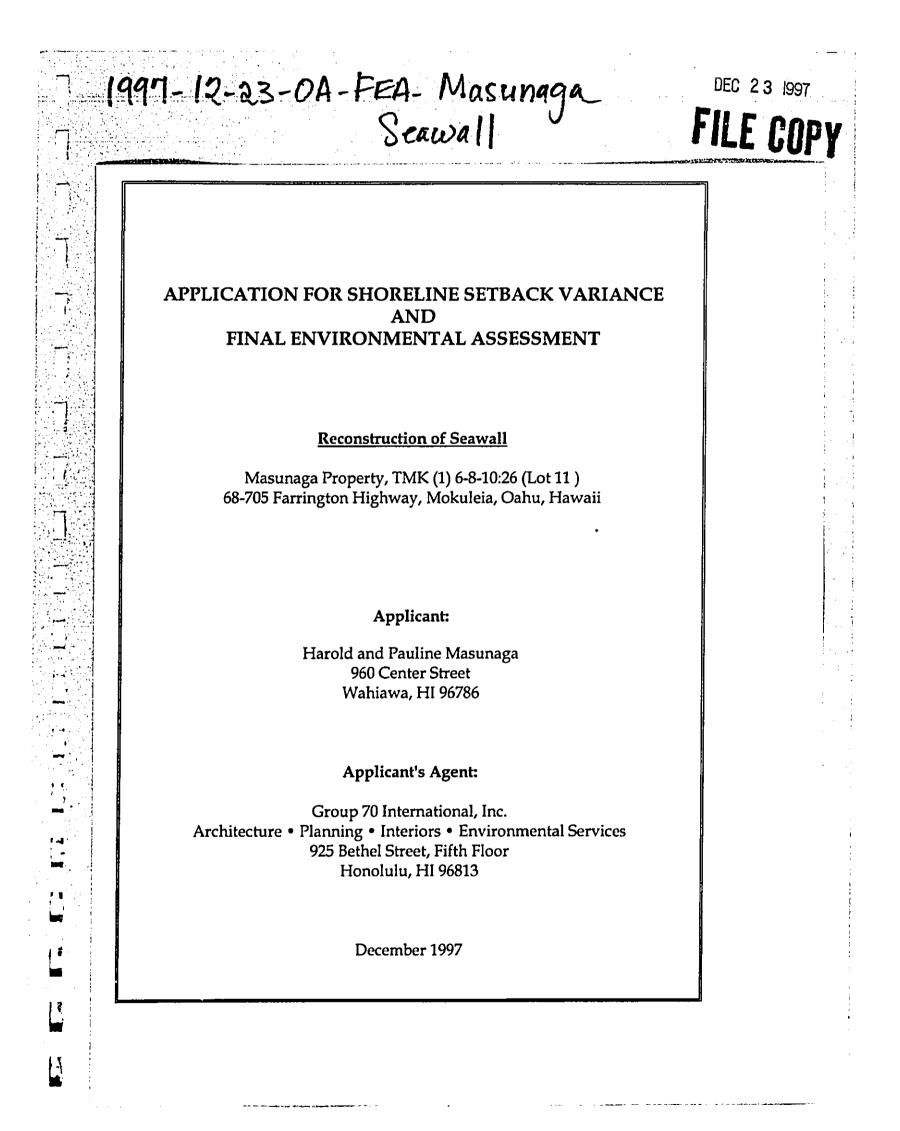
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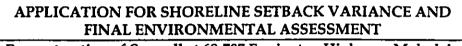
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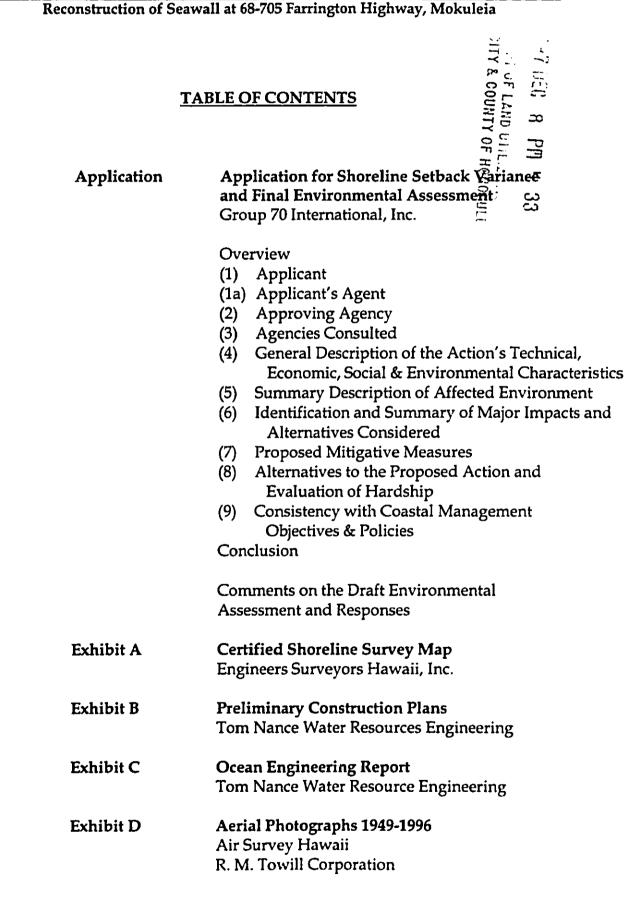
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December 1997

<u>Reconstruction of Existing Seawall Structure</u> Masunaga Property, TMK (1) 6-8-10:26 (Lot 11) 68-705 Farrington Highway, Mokuleia, Oahu, Hawaii

**Overview.** Approval is being sought for construction of a modified vertical seawall structure, to reconstruct an existing grouted rock and boulder seawall that was constructed across the shoreline frontage of the subject property between 1967 and 1969. The structure was built without City approvals, including a Shoreline Setback Variance (ROH 1992 Chapter 23) and a Building Permit (ROH 1990 Chapter 18). This application and environmental assessment provides a description of the action and addresses the potential impacts of the proposed shoreline structure to the coastal environment.

# (1) Applicant

Harold and Pauline Masunaga 960 Center Street Wahiawa, HI 96786 (808) 622-1116

# (1a) Applicant's Agent

Group 70 International, Inc. 925 Bethel Street, 5th Floor Honolulu, HI 96813-4307 Jeffrey Overton, Chief Environmental Planner (808) 523-5866 ext. 111

# (2) Approving Agency

City and County of Honolulu, Department of Land Utilization 650 South King Street, 7th Floor Honolulu, HI 96813 Art Challacombe, Environmental Review Branch (808) 523-4107

# (3) Agencies and Organizations Consulted

City and County of Honolulu, Department of Land Utilization City and County of Honolulu, Building Department State of Hawaii, Department of Land and Natural Resources North Shore Neighborhood Board No. 27

Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

# (4) General Description of the Action's Technical, Economic, Social and Environmental Characteristics

**Technical Characteristics.** The proposed action involves approval for construction of a modified vertical seawall made of grouted rock and boulder material to replace the existing shoreline structure protecting this lot on Farrington Highway in Mokuleia. The general location of the subject property is shown in Figure 1 and the TMK map (Figure 2). The site is directly across from the entrance road to the eastern end of Dillingham Airfield.

The subject seawall structure is located along the 68.00 ft. shoreline frontage of the Masunaga property which is 8,551 sq. ft. in area. The parcel is relatively level and improved with a single-family residence.

Based on historical aerial photographs of the Mokuleia coastline taken over the past 47 years (1949-1996), there has been a significant loss of shoreline at this location due to erosion activity since the lots were first subdivided. The subject property has lost between 40 to 50 feet of land along the makai edge, totaling approximately 3,000 sq. ft.

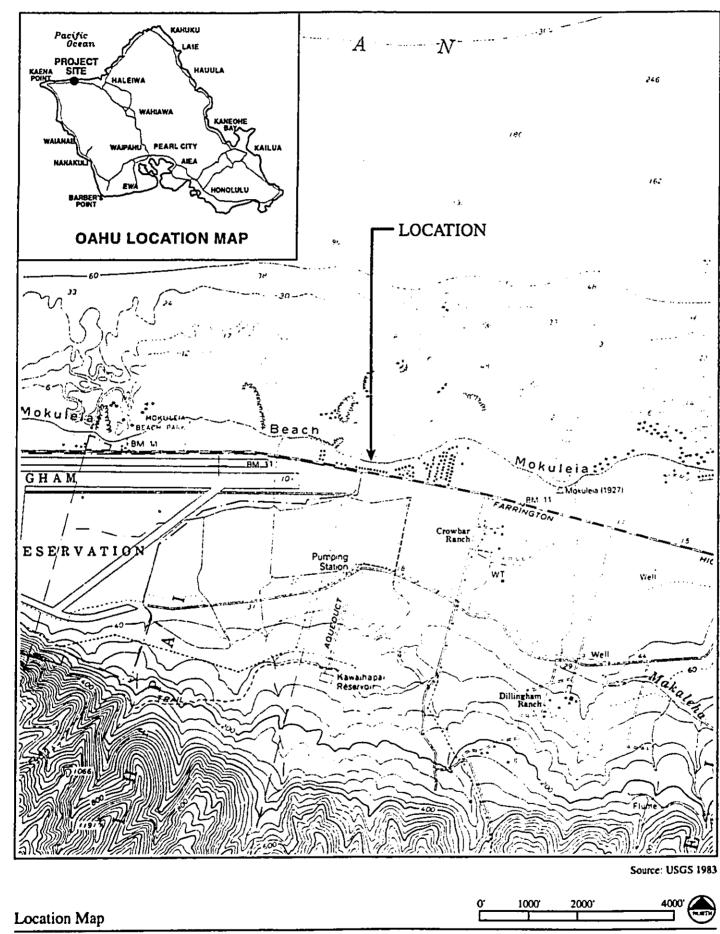
Since the 1950's, shoreline structures have been constructed along the ocean frontage of the adjoining properties to the east and west to help stabilize the retreating shoreline. There are 16 residential properties in a row that are all protected by vertical seawalls at this location. These walls were built between 1961 and 1981. Lots that do not have structural protection along the eastern portion of this coastal section are experiencing some shoreline erosion and storm wave damage.

Figure 3 shows the Shoreline Survey Map currently being processed for certification by the DLNR (also refer to Exhibit A). The Certification request was submitted to DLNR by Engineers Surveyors Hawaii in April 1997. Certified maps will be forwarded to the DLU upon receipt. This figure provides site specific details of the shoreline structure, showing location and elevation relative to the makai side and neighboring residential lots.

Figures 4, 5 and 6 are photographs of the subject property and existing shoreline structure. The seawall spans the entire shoreline frontage of the Masunaga property.

Figure 7 shows cross sections of the proposed modified vertical seawall structure composed of grouted rock and boulders. The height of the new structure will be approximately 9.5 ft. The seawall will be rebuilt with an appropriately designed foundation to avoid the undermining effect that has caused the failure of the existing wall. Design of the wall will be based on the maximum wave height that can break on the structure and to retain the bearing load on the inland side of the wall. The seaward side of the seawall foundation will be placed at the certified shoreline.

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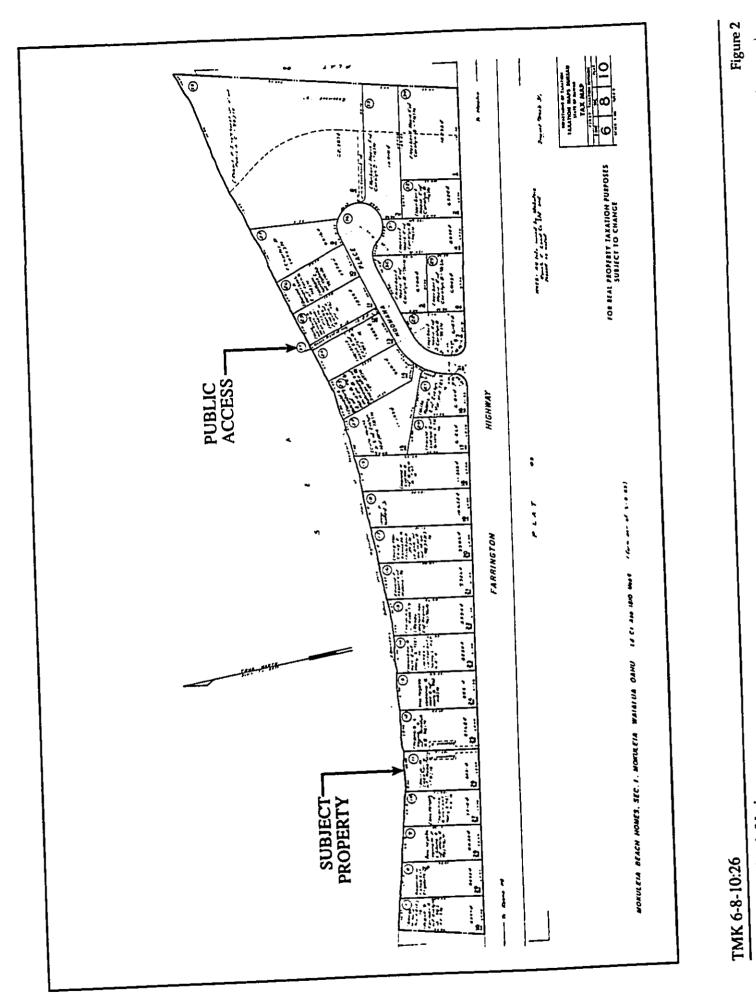
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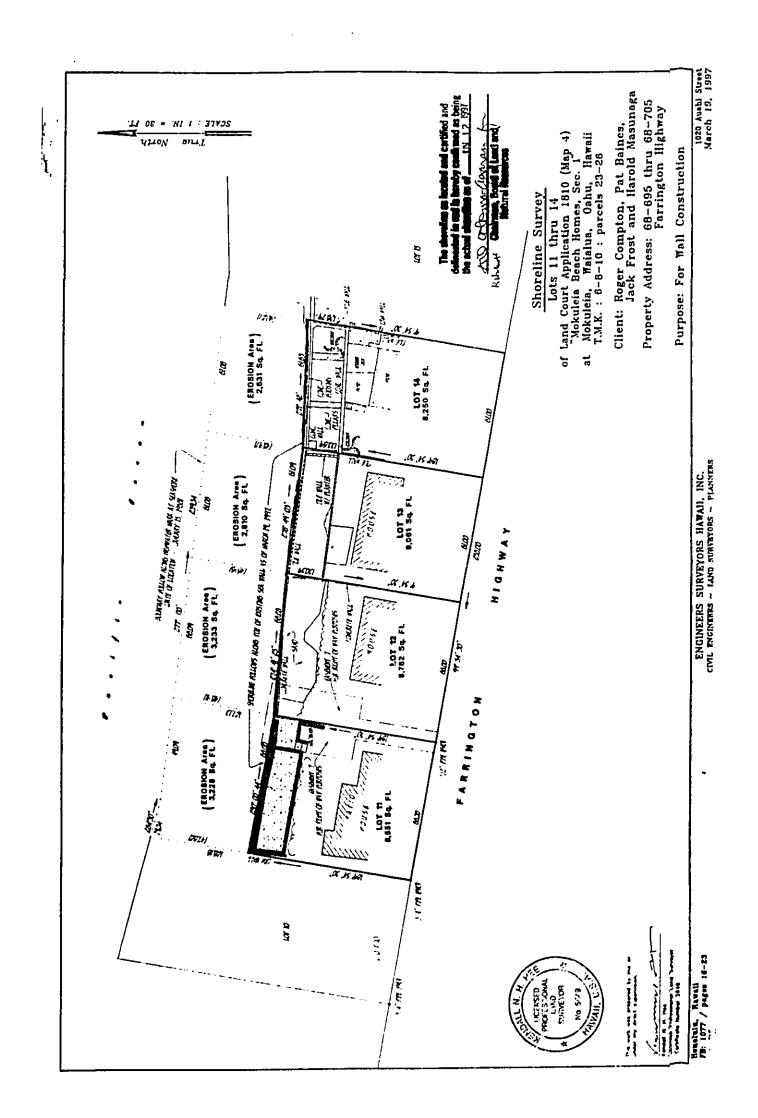
Figure 1



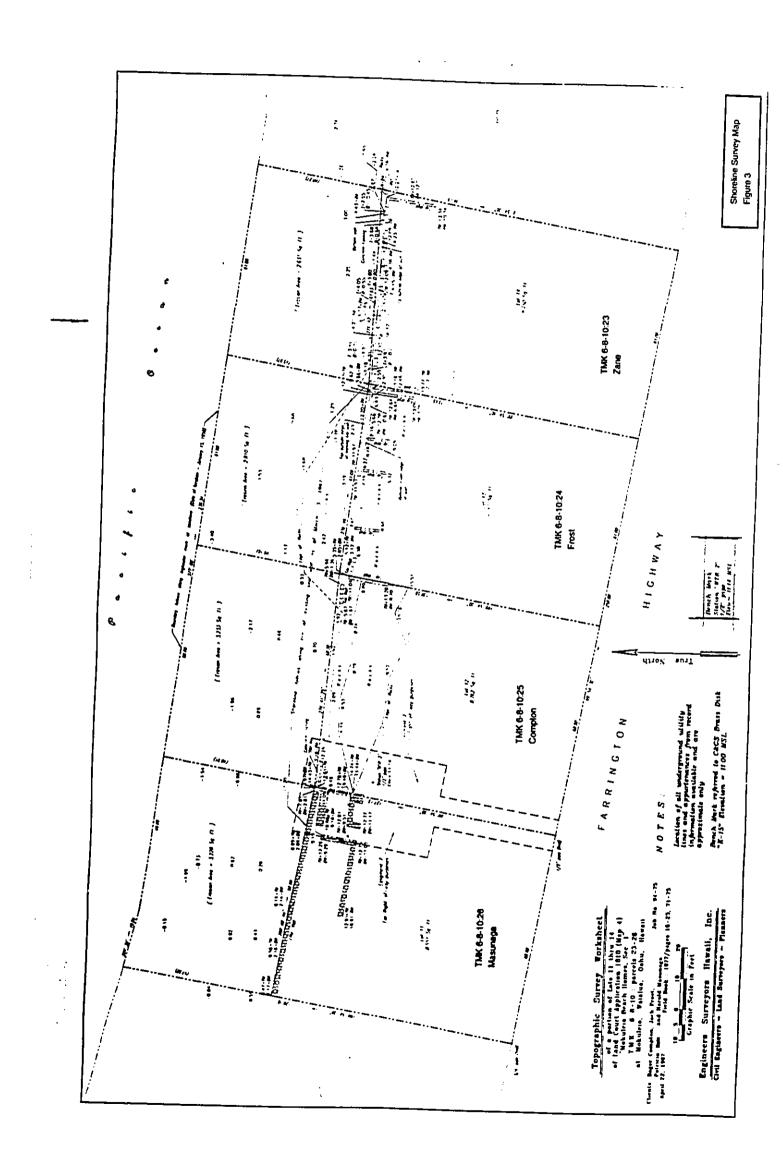
**ب**را , ,..., Shoreline Setback Variance а J 4 1 

Figure 2 ~

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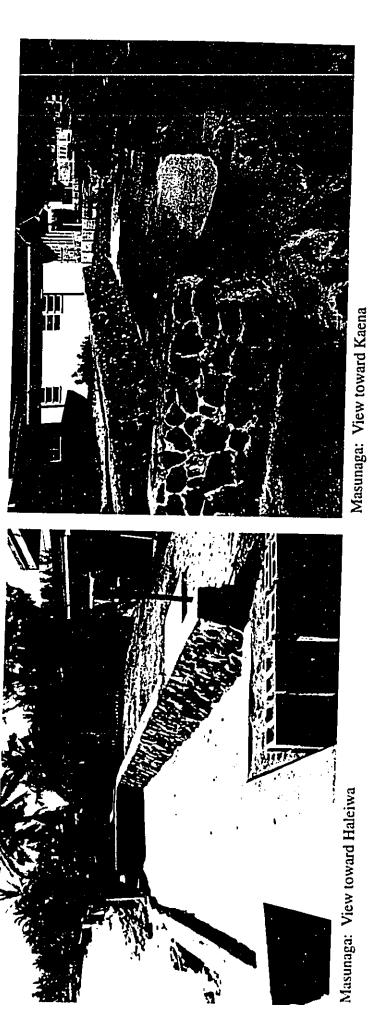
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View toward Kaena of Masunaga Property Rear Yard and Shoreline Structure 

Shoreline Setback Variance

Figure 4

Figure 5



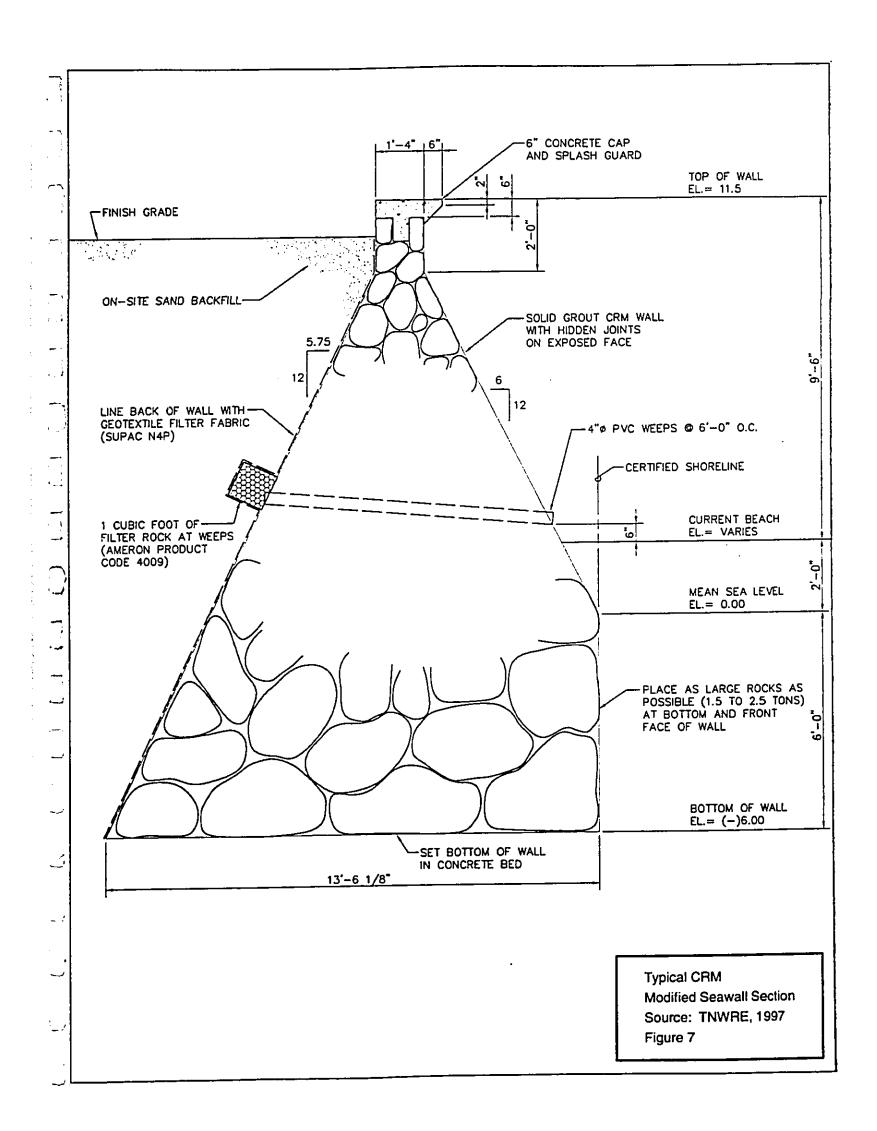
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(Source: TNWRE, July 1997) Figure 6

The rubble masonry seawall at TMK 6-8-10:26 has been undermined along its entire length. Portions of the stair structure (top left) has collapsed on the beach. <u>Masunaga:</u>



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Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

Borings at the site reached a depth of 31 feet without encountering bedrock. Coralline sand extends to a depth of 23 feet. The wall foundation will be placed at a depth of 6 feet below sea level (approximately 14 ft. below grade) in a bed of concrete placed on coralline sand. The results of this boring were similar to two boreholes done previously for the reconstruction of the seawall at TMK 6-8-10:29.

The seawall cross section is similar to the recently completed adjacent seawalls to the west on TMK 6-8-10:27, 28 and 29. The design combines a sloped lower half and a vertical upper half, which is a compromise between reducing wave reflection and retaining a small yard space between the houses and the wall. Void spaces in the wall material on the ocean side will not be grouted to allow for added energy dissipation and to minimize wave run-up. The new wall will replace the existing vertical structure.

<u>Present Condition of the Shoreline Structure.</u> Unlike the hollow tile walls of other lots in the vicinity, this seawall is made of grouted rock and boulders. However, because the foundation of this seawall does not go below sea level, this wall has experienced significant undermining. There has also been a collapse of a rock stairs at the east end of the wall. To stop the proliferation of the sink hole, caused by waves washing underneath the foundation, a substantial volume of concrete was installed on the inland side of the wall. The seawall is relatively low and subject to relatively frequent overtopping, therefore, a second interior wall was constructed about 14 feet inland of the front wall, and is about 3.5 feet higher.

Exhibit C includes a report completed by Tom Nance Water Resources Engineering (TNWRE)(July 1997). This report provides an oceanographic evaluation of the shoreline revetment and color photographs. An evaluation of the new modified vertical seawall, its materials and structural stability is also included with the TNWRE report.

**Socio-Economic Characteristics.** The total construction cost value for the new seawall is estimated at \$65,000. The construction will cause no economic impacts to the immediate community or the community at large.

Without the shoreline structure, further erosion of the shoreline frontage during high surf events could ultimately resulting in damage to the existing residential structure. The property owner could potentially lose the value of their land and improvements if the shoreline structure is not constructed. The proposed action will be undertaken to protect these assets.

**Environmental Characteristics.** The original shoreline structure was constructed sometime between 1967 and 1969. The oceanographic study completed by TNWRE (Exhibit C) evaluates the potential for erosion caused by the shoreline structure. The study of historical aerial photographs shows that erosion of the adjacent beach areas has not been accelerated by the presence of this structure. Without the seawall, erosion along the seaward frontage of the subject property would have continued unchecked, and probably would have threatened the existing residential structure.

APPLICATION FOR SHORELINE SETBACK VARIANCE AND FINAL ENVIRONMENTAL ASSESSMENT	
Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia	
Construction activities associated with the new seawall are not anticipated to cause	
significant adverse effects to ocean water quality. Boulder placement and construction activities will be limited to areas inland of the certified shoreline. No long-term effects	
to water quality will result.	
(5) Summary Description of the Affected Environment.	
C it was this margel are candy and well-drained. Excavation along the wall found the	
1 (reasonation to be mixed character, with coarse-granied calcaleous beach barre	
and other buried fill material (limestone cobbles and boulders). Vegetation of this site	
shrub plants including naupaka. There are no known significant habitat areas for charter	
terrestrial or aquatic flora or fauna directly found at the project site.	
Beach and offshore conditions are summarized in this section, based on the detailed	
assessment provided in TNWRE (July 1997)(Exhibit C).	
The property is situated near the center of an unnamed shoreline embayment defined	
by rock outcrops on the western end (Camp Mokuleia) and a sandy headland on the east side. The sandy headland appears to have been formed by the wave protection by	
is a submore and the generally shallower bathymetry directly offshore. The	
ambaument which faces directly to the north, is about 2,500 feet across and the	
indentation of the shoreline is a maximum of 500 feet at its center.	
Nearshore bathymetry is generally flat, although there are a number of boulders, some	
of which protrude above water at low tide. The bottom is comprised of dead control,	
Dopths are generally six to eight reet for utstances of 1,000 to 2,000	
feet offshore. At that point, a series of ledges create a relatively steep drop-off to depths of more than 100 feet within 4,000 feet of the shoreline. There is a significant submarine	
channel located just to the east of the embayment.	
The second s	
it and The more protect toaching the shoreline is much greater winning are many	
northeast. The wave energy reaching the uncounter frequent. Depths of the nearshore months when waves from these directions are most frequent. Depths of the nearshore shelf control the breaking of waves and the amount of energy reaching the beach. At	
low tide, the wave energy at the shoreline is far less than at high tude, shippy tude to the	
different water depths at the tidal extremes.	
Despite the fact that only moderate-sized waves can translate across the nearshore shelf and break on the beach, all of the lot owners along this embayment from Camp	
the total second to the beach access pasement at no uniana trace that come to	
the anstruction to stop the progressive loss of ulen beach norting out	
damage to structures. Seawalls protecting three lots to the west of the subject property	

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Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

(TMK 6-8-10:27, 28 & 29) were recently reconstructed with a modified vertical seawall design. Of the remaining 12 lots to the east, the next four lots (Masunaga, Compton, Frost and Zane) have walls that are most in need of reconstruction. Their deteriorated condition results from inadequate foundations, and also due to their position at the center of the bay where the beach is narrow and the highest waves strike the shore.

There have been significant shoreline changes along this section of the Mokuleia shoreline. Review of historical aerial photographs from 1949 to 1995 verify the shoreline changes during this period. Exhibit C presents a series of these photographs are presented with overlays showing the past and present shoreline positions.

There has been a diminishing width of beach sand over the 1949 to 1996 period. The width of beach sand is now 18 to 25 feet, which is less than one-third the width that prevailed in 1949. This loss of beach sand width explains the recent failure of a number of seawalls along this embayment shore. In all cases, the walls were built with their foundations above sea level. At the time of their construction, there was a substantial beach sand deposit between these footings and the shoreline. Much of the sand has been eroded, with waves that continually wash beneath the footings to the inland side of the wall. A chronology of house and seawall construction, movement of the vegetation line, and approximate beach widths based on aerial photographs are presented in Exhibits C and D (aerial photos).

# (6) Identification and Summary of Major Impacts and Alternatives Considered

**Potential Short-term Impacts.** The reconstruction of the seawall along the frontage of this lot will create some minor short-term effects on vegetation, water quality and noise conditions. A small amount of landscaping vegetation (grass and low shrubs) will be removed by the construction activity. During construction, there is always the potential for soils to erode from the upland area and cause silt runoff to ocean waters. Measures will be taken during construction to protect soils. Lastly, construction noise will be noticeable to residents at the neighboring properties. Construction activity will occur during allowed daytime periods and will not cause excessive noise levels off-site.

# Potential Long-term Impacts.

<u>Shoreline Processes.</u> The effect of the seawall on shoreline processes at this location is considered, given that there are existing walls on adjacent properties to the east and west. The subject seawall structure has been in place for over 25 years. The impact on shoreline processes of the seawall has been negligible due to the presence of a series of shoreline structures on adjoining lots.

For more than a 3,000 foot stretch of the Mokuleia shoreline in this area, nearly all of the lots are protected by seawalls. Lots toward the eastern end of this coastal cell remain unprotected. All other lots in this stretch are protected by vertical seawalls. Shoreline retreat of the remaining unprotected lot frontages is definitely occurring.

Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia	
Over a record period of 38 years, the Oahu Shoreline Study (Sea Engineering, 1989)	,
found a shoreline retreat of in this sector of the Laie coastline where there are no	<b>*</b> -•
protective structures. The study identifies the area from the Episcopal Camp to Mokuleia Beach Colony as Mokuleia - transect 11. Excerpts from the study discussion of	
this portion of the Mokuleia coast are included below.	New y
This is a small embayment, 3000 feet long, that is completely developed. Polipoli	1
Stream discharges in the center of the embayment. The shoreline from the	<b>b</b> 4
Episcopal Camp to the stream is lined with shore protection structures, excent	:
for the four lots just west of the stream. The unprotected houses have only a few feet of vegetation between them and the beach.	
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The structures are generally vertical seawalls of varying heights and types. At the west and martinglashe the seawalls	•
the west end, particularly, the walls protrude varying distances out onto the beach.	* ~ <b>1</b>
Citran the enterint of the control of	ı
Given the extent of the existing seawalls and the proximity of the unprotected houses to the waterline, shore protection should be allowed throughout this area.	
The shore protection structure of choice will probably be a vertical seawall, since	,
there is little room for sloping reverments. The DLU should ensure that the	<b>•</b> .
design is adequate and that the alignment matches the surrounding areas.	,
At present, there is lateral access along this beach, at least during some seasons, but if erosion continues, this will be lost.	· .
The Oahu Shoreline Study presents shoreline retreat rates for Mokuleia at transects 10	
and 12, which are located on either side of the subject coastal section. Shoreline retreat	· · ·
in the 38-year period evaluated at these nearby transect areas ranged from 12 to 14 feet.	· •
This description and management recommendation is consistent with the findings of	· ,
this specific ocean engineering assessment for the subject property. With the pattern of	
shoreline protection which has been established, an individual lot owner has little choice but to protect his property with a structure similar to the one existing along the	•
frontage of the subject property.	
Aesthetics. The existing seawall at the subject property is similar in aesthetic condition	
to the surrounding lots with shoreline structures. The vertical seawall planned for this	•
property is concrete rubble masonry (CRM) which contains significant amount of	
exposed rock material on the seawall face. The rock material provides a more natural appearance to the vertical seawall.	•~~
7) Proposed Mitigative Measures	
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everal mitigative measures have been taken and are proposed to reduce or eliminate he potential impacts of the seawall reconstruction at the subject lot.	•
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Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

Best Management Practices. Water quality will be protected during reconstruction of the seawall structure. Measures will be taken during the construction activities to avoid erosion and silt runoff to surface water in the ocean. Soils on the mauka side of the structure will be stabilized to prevent silt runoff to the beach and ocean water. Work will be done during the drier summer months, with an expected duration of six to eight weeks. Foundation work will take about two weeks, and water will be directed away from the construction by the use of a temporary berm or sheet piles installed makai of the wall. Lands mauka of the wall will be planted in grass at the end of construction, however, the potential for erosion is remote since the new wall will contain the material placed mauka of the wall.

Aesthetic Effects. The owner will construct a more natural appearing rock wall face to this seawall structure. This will be an aesthetic improvement in comparison to the existing wall.

# (8) Alternatives to the Proposed Action & Evaluation of Hardship

There are several issues which must be considered in the evaluation of hardship for the application for Shoreline Setback Variance at the subject property. Three alternative approaches are considered possible at this time, including:

- (a) No-action alternative require removal of the seawall,
- (b) Construct a sloping rock revetment in place of the seawall, and
- (c) Attempt a non-structural approach to protect this property.

These options are discussed individually in terms of their potential impacts, including hardship to the applicant.

(a) No action - Remove seawall structure

The no-action scenario would involve removal of the seawall and leave the shoreline frontage of the lot unprotected. This action would expose the property to storm wave erosion, causing the makai 20 to 30 feet of the property to erode. The residence on the subject property would potentially be exposed to storm wave run-up and damage.

Shoreline structures fronting parcels on either side of the subject lot could also potentially be back-cut by the erosional activity. The no-action alternative would potentially cause damage and property loss to the subject lot, and is not considered feasible. The historical trend of this stretch of shoreline is steady erosion on the order of 0.5 feet per year.

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FINAL ENVIRONMENTAL ASSESSMENT	•
Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia	
(b) Construct a sloping rock revetment in place of the vertical seawall	-
(-)	
A sloping boulder revetment at this location could be constructed to provide additional structural strength and provide a greater slope for wave energy dissipation. As compared to vertical seawall, revetments generally cause less energy reflection to the	p.
nearshore shallow water area, creating less erosional force. This would require removal of the existing seawall structure and construction of a sloping revetment with	٠.
corresponding short-term environmental effects.	
This option is not practical in this situation, however, because there is no space to accommodate a sloping boulder revetment. A properly constructed revetment would	<b>▲</b>
take up all of the owner's usable lot area in rock slope extending to the edge of the	۱
home. In addition, a revetment at this location would not match the structures on the adjoining properties. There are no other revetments existing along this stretch of	
Mokuleia, where the lots are protected with vertical seawalls for 16 properties in a row. Deviations in the shoreline structure design from seawall to revetment would create an	• •
uneven visual setting. The mix of structure types would likely causes changes to the	
wave energy distribution along the shoreline, and possibly affecting the flank area of adjoining properties which are currently protected by vertical seawalls.	<b>پ</b> انو ب
(c) Attempt "soft structure" and non-structural solutions along this property	•
There are a number of non-structural approaches to curbing shoreline erosion that have	
been suggested for the shoreline of Oahu. These options include the use of sand-filled sea bags, offshore sand mining for beach replenishment, and moving structural improvements further mauka to avoid ocean wave damage.	
Sand-filled sea bags have shown to provide some effectiveness in temporarily curbing	•
shoreline property loss to erosion at some locations. In this situation, the sea bags	,
would interfere with lateral access in front of the subject property. The sea bags would temporarily take the place of the seawall, and would be a short-term solution to an	. 1
obviously long-term erosion problem at this location. The owner would need to	· · · ·
continually maintain the bags and periodically replace them at continuing cost. There would be no real environmental benefit from this option.	
	· •
Offshore sand mining and beach replenishment has been proposed for a number of	
ocations in Hawaii. The intent of beach replenishment is to offset erosion activity along a coastline by providing sand material from offshore sand reserves or other nearby	· .
sources. Sand replenishment can be used in an attempt to re-create the beach and dune	
structure. This alternative could be potentially feasible in areas where offshore sand reserves exist (not known to be present at this location) and a government agency or	n *
arge private entity can fund this activity. This type of area-wide massive beach	
replenishment project would not be a practical solution for a small single property owner. Formation of an improvement district would be a possible long-term approach o solving erosion problems along this coastal section. This solution would take	* •
a solving provide problems along this second leasting. This solution of the t	

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# APPLICATION FOR SHORELINE SETBACK VARIANCE AND FINAL ENVIRONMENTAL ASSESSMENT Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia extensive time to plan the program and assemble the government approvals and

extensive time to plan the program and assemble the government approvals and resources to complete the project. In the current situation at the subject property, this would not be a practical way to satisfy an urgent need to protect against imminent property loss and damage.

Another alternative to the shoreline structure would be to move the structural improvement (residence) further mauka placing it outside of the erosion and ocean wave hazard. At this location, moving the residence mauka to avoid erosion activities would not be practical, since there is no space on the lot to shift the building.

(9) Consistency with Coastal Management Objectives and Policies.

The objectives of the Hawaii Coastal Zone Management Program, Syction 205A-2, HRS, are to protect valuable and vulnerable coastal resources such as coastal ecosystems, special scenic and cultural values and recreational opportunities. The objectives of the program are also to reduce coastal hazards and to improve the review process for activities proposed within the coastal zone. Described below are the ten objectives and policies of the Hawaii Coastal Zone Management Program and an assessment of the project impacts relative to the CZM objectives and policies.

- (1) <u>Recreational Objective.</u> "Provide coastal recreational opportunities accessible to the public."
- (A) Improve coordination and funding of coastal recreation planning and management.
- (B) Provide adequate, accessible, and diverse recreational opportunities in the coastal zone management area by:
  - (i) Protecting coastal resources uniquely suited for recreational activities that cannot be provided in other areas;
  - (ii) Requiring replacement of coastal resources having significant recreational value, including but not limited to surfing sites and sandy beaches, when such resources will be unavoidably damaged by development; or requiring reasonable monetary compensation to the State for recreation when replacement is not feasible or desirable;
  - (iii) Providing and managing adequate public access, consistent with conservation of natural resources, to and along shorelines with recreational value;
  - (iv) Providing an adequate supply of shoreline parks and other recreational facilities suitable for public recreation;
  - (v) Encouraging expanded public recreational use of county, State, and federally owned or controlled shoreline lands and waters having recreational value;
  - (vi) Adopting water quality standards and regulating point and nonpoint sources of pollution to protect and where feasible, restore the recreational value of coastal waters;
  - (vii) Developing new shoreline recreational opportunities, where appropriate, such as artificial lagoons, artificial beaches, artificial reefs for surfing and fishing;

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	APPLICATION FOR SHORELINE SETBACK VARIANCE AND FINAL ENVIRONMENTAL ASSESSMENT	•
	Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia	L
	Reconstruction of Scattan at 65 to 19	<b></b>
	is a second second second with recreational value for	,
	(viii) Encouraging reasonable dedication of shoreline areas with recreational value for public use as part of discretionary approvals or permits by the land use commission, and crediting	
	public use as part of discretionary approvals of permiss of the tale and crediting board of land and natural resources, county planning commissions; and crediting	<b>≜</b> •
	such dedication against the requirements of section 46-6.	ť
		<b>4</b> 1.1
	<b>Discussion:</b> Public access to the beach fronting the property is not affected by the	
	t it stantage The controll on this property lids been at place for other	
	There is a very narrow beach extending along the shore which is affected by wave run- up during high tides, particularly during high surf events. There is lateral access along	<b>6</b> - 4
	this shoreline and its recreational use will not be diminished by the proposed action.	
		<b>L</b> - •
	(2) <u>Historic Resources Objective</u> . "Protect, preserve and, where desirable, restore those	ډ
	natural and man made historic and pre-historic resources in the cousin 20th minus entering	
	area that are significant in Hawaiian and American history and culture."	ه اسل
	(A) Identify and analyze significant archaeological resources.	٠
	(A) Identify and analyze significant archaeological resources of remains and artifacts or salvage (B) Maximize information retention through preservation of remains and artifacts or salvage	ملا
		-
	operations. (C) Support State goals for protection, restoration, interpretation and display of historic	
	resources.	њ.,
	Discussion: Archaeological resources are not affected by the shoreline structure at this	
	avoid exposure of any unknown buried cultural deposits and remains.	• •
	Browner Objective "Protect preserve and where desirable,	
	3) <u>Scenic and Open Space Resources Objective</u> . "There, prediction and preserve and	•
		· •
	(A) Identify valued scenic resources in the coastal zone management area.	. ,
	and a second and a second and a second the first use of the second and a second and a second and a second a s	
	and locating such developments to minimize the alteration of natural analytics and outcome	: •
	public views to and along the shoreline. (C) Preserve, maintain, and, where desirable, improve and restore shoreline open space and	
		•
	scenic resources. (D) Encourage those developments which are not coastal dependent to locate in inland areas.	
	Discussion: The shoreline structure at the subject property is built of lava rock material	• •
	Discussion: The shoreline structure at the subject property involves visual relief which which has a natural appearance. The rock material provides visual relief which "softens" the appearance of the structure. The shoreline transition provided by the rock	
	"softens" the appearance of the structure. The shortened dimensional or concrete-faced material is more visually appealing than the standard CMU wall or concrete-faced	•
	seawall structure.	
	(4) <u>Coastal Ecosystems Objective</u> . "Protect valuable coastal ecosystems from disruption and	•
	<i>minimize adverse impacts on all coastal ecosystems.</i> "	
	(A) Improve the technical basis for natural resource management.	

Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

(B) Preserve valuable coastal ecosystems of significant biological or economic importance.

- (C) Minimize disruption or degradation of coastal water ecosystems by effective regulation of stream diversions, channelization, and similar land and water uses, recognizing competing water needs.
- (D) Promote water quantity and quality planning and management practices which reflect the tolerance of fresh water and marine ecosystems and prohibit land and water uses which violate state water quality standards.

Discussion: The project will have no significant adverse effect on coastal ecosystems. Runoff will be controlled at the project site. Mitigative measures to reduce runoff for the short-term construction and long-term use of the site are planned. Best management practices will be applied in site construction activities.

- "Provide public or private facilities and improvements (5) Economic Uses Objective. important to the State's economy in suitable locations."
- (A) Concentrate in appropriate areas the location of coastal dependent development necessary to the state's economy.
- (B) Insure that coastal dependent development such as harbors and ports, visitor industry facilities, and energy generating facilities are located, designed, and constructed to minimize adverse social, visual, and environmental impacts in the coastal zone management area.
- (C) Direct the location and expansion of coastal dependent developments to areas presently designated and used for such developments and permit reasonable long-term growth at such
  - areas, and permit coastal dependent development outside of presently designated areas when:
    - (i) Utilization of presently designated locations is not feasible;
    - (ii) Adverse environmental effects are minimized; (iii) Important to the State's economy.

Discussion: The subject property has no economic activity at present. The proposed action will generate short-term economic benefits from construction activity.

- (6) Coastal Hazards Objective. "Reduce hazard to life and property from tsunami, storm waves, stream flooding, erosion and subsidence."
- (A) Develop and communicate adequate information on storm wave, tsunami, flood, erosion, and subsidence hazard.
- (B) Control development in areas subject to storm wave, tsunami, flood, erosion, and subsidence hazard.
- (C) Ensure that developments comply with requirements of the Federal Flood Insurance Program.
- (D) Prevent coastal flooding from inland projects.

Discussion: The subject property is located in the flood hazard area and complies with the Federal Flood Insurance Program. The shoreline structure at this property serves to stem erosion along the shoreline frontage, which protects the residence on this property, adjoining properties and inland areas.

Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

- (7) <u>Managing Development Objective</u>. "Improve the development review process, communication, and public participation in the management of coastal resources and hazards."
- (A) Effectively utilize and implement existing law to the maximum extent possible in managing present and future coastal zone development.

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- (B) Facilitate timely processing of application for development permits and resolve overlapping or conflicting permit requirements.
- (C) Communicate the potential short and long-term impacts of proposed significant coastal developments early in their life cycle and in terms understandable to the general public to facilitate public participation in the planning and review process.

**Discussion:** The landowner has commissioned the preparation of this Shoreline Setback Variance Application and Environmental Assessment in part to provide the public with details about their shoreline structure and shoreline conditions. The applicant has been in contact with the City Department of Land Utilization and State Department of Land and Natural Resources. Agencies, organizations and individuals will be notified of this proposed action in the <u>Environmental Notice</u> published by the Office of Environmental Quality Control. A public hearing will be held by the Department of Land Utilization, unless a public hearing waiver is granted.

(8)	Public Participation Objective.	"Stimulate pu	ublic awareness, education, and	
	participation in coastal management			

- (A) Maintain a public advisory body to identify coastal management problems and to provide policy advice and assistance to the coastal zone management program;
- (B) Disseminate information on coastal management issues by means of educational materials, published reports, staff contact, and public workshops for persons and organizations concerned with coastal-related issues, developments, and government activities; and
- (C) Organize workshops, policy dialogues, and site-specific mediations to respond to coastal issues and conflicts.

**Discussion**: Refer to discussion for Objective 7.

(9) <u>Beach Protection Objective</u>. "Protect beaches for public use and recreation."
(A) Locate new structures inland from the shoreline setback to conserve open space and to minimize loss of improvements due to erosion;
(B) Prohibit construction of private erosion-protection structures seaward of the shoreline, except when they result in improved aesthetic and engineering solutions to erosion at the sites and do not interfere with existing recreational and waterline activities; and
(C) Minimize the construction of public erosion-protection structures seaward of the shoreline.
Discussion: The shoreline structure at this property is located inland of the certified

shoreline. There is no loss of public recreation space and open space as a result of this

Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

structure. Erosion of property and improvements is minimized by this shoreline structure. There are few viable options remaining for this property owners except to properly reconstruct the existing vertical seawall structure.

(10) <u>Marine Resources Objective.</u> "Implement the State's ocean resources management plan."

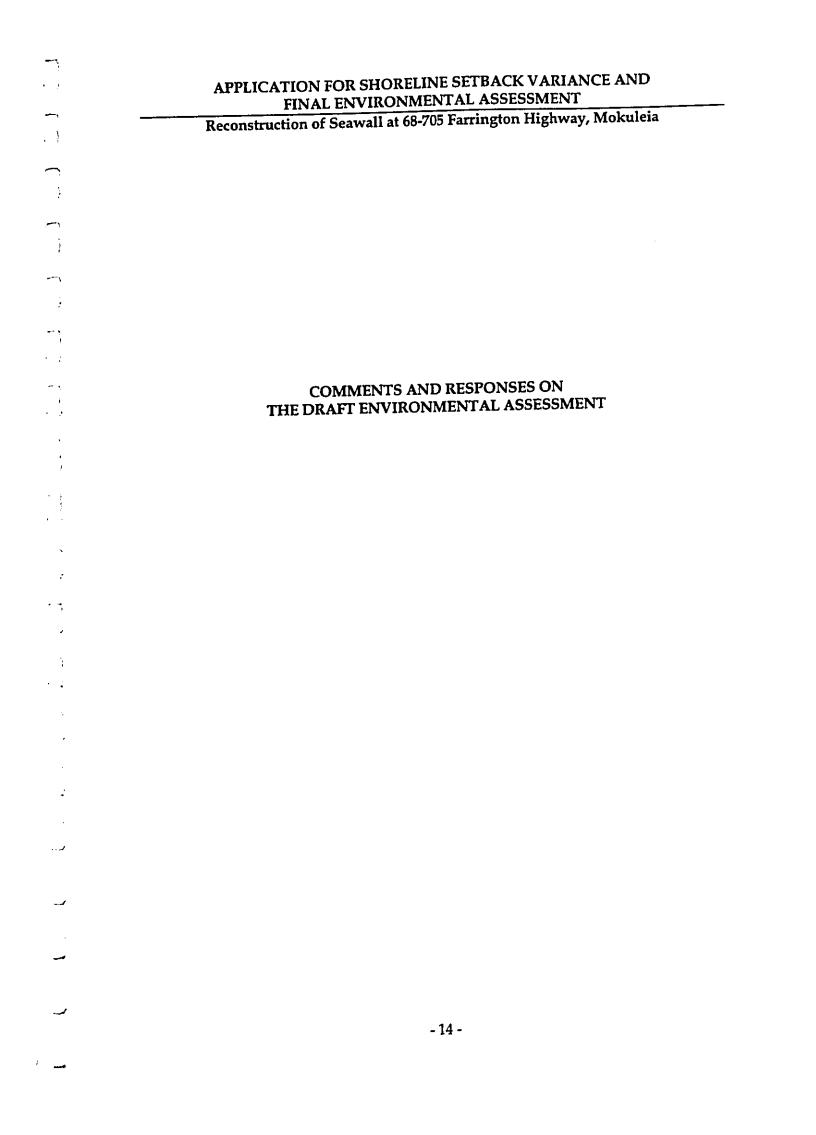
- (A) Exercise an overall conservation ethic, and practice stewardship in the protection, use, and development of marine and coastal resources;
- (B) Assure that the use and development of marine and coastal resources are ecologically and environmentally sound and economically beneficial;
- (C) Coordinate the management of marine and coastal resources and activities management to improve effectiveness and efficiency;
- (D) Assert and articulate the interests of the State as a partner with federal agencies in the sound management of ocean resources within the United States exclusive economic zone;
- (E) Promote research, study, and understanding of ocean processes, marine life, and other ocean resources in order to acquire and inventory information necessary to understand how ocean development activities relate to and impact upon ocean and coastal resources; and
- (F) Encourage research and development of new, innovative technologies for exploring, using, or protecting marine and coastal resources. [L 1977, c 188, pt of §3; am L 1993, c 258, §1; am L 1994, c 3, §1; am L 1995, c 104 §5]

<u>Discussion</u>: The landowner will follow the most environmentally sound approach to reconstructing their seawall by utilizing rock material for natural appearance, using a modified seawall design to disperse wave energy, and placement following the alignment of the adjoining shoreline structures.

**CONCLUSION.** The findings of this Environmental Assessment indicate that the proposed action is found to create minimal environmental impact and appears to be reasonable, when considering other possible alternative actions at this location. In terms of oceanographic processes, the modified seawall structure does not cause adverse effects to the beach at the adjoining and nearby properties. The preparers of this assessment recommend that a Finding of No Significant Impact (FONSI) be issued for this action.

The modified seawall structure will be properly designed to withstand seasonal ocean wave wash at this location. There is a very well-documented recent history (past 50 years) of shoreline retreat along this portion of the Mokuleia coast. The landowner would necessarily experience hardship if the seawall was not reconstructed, with a likely loss of property and potential damage to residential structure. For these reasons, and based on the documentation provided, this landowner requests approval of a variance from the shoreline setback ordinance.

Other permits will be obtained as necessary to complete the project, including a Department of Army Nationwide Permit and coordination with the State Department of Health regarding requirements for a Section 404 Water Quality Certification. A City Building Permit will be required for construction to proceed.



CITY AND COUNTY OF HONOLULU

60 SOUTH KING STREET, 7TH FLOOR + HONOLULU, HAWAII 96813 PHONE: (808) 523-4414 + FAX: (808) 527-6743

JEREMY HARRIS MAYOR

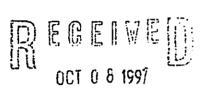
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October 7, 1997

JAN NAGE SULLIVAN DIRECTOR

LORETTA K.C. CHEE DEPUTY DIRECTOR 97/SV-003 (ST) 97/SV-004 (ST) 97/SV-005 (ST) 97/SV-006 (ST)

FROUD 70

Mr. Jeffrey Overton Group 70 International, Inc. 925 Bethel Street, Fifth Floor Honolulu, Hawaii 96813

Dear Mr. Overton:

Project Names: Zane Seawall Reconstruction (97/SV-3) Frost Seawall Reconstruction (97/SV-4) Compton Seawall Reconstruction (97/SV-5) Masunaga Seawall Reconstruction (97/SV-6) 68-695, 68-697, 68-701 and 68-705 Farrington Highway, Mokuleia, Oahu Tax Map Keys: 6-8-10: 23, 24, 25, 26

We are forwarding copies of all comments we have received relating to the Draft Environmental Assessments (DEAs) for the abovereferenced projects.

In accordance with the provisions of Chapter 343, Hawaii Revised Statutes (HRS), you must respond in writing to these and any other comments which were received during the 30-day comment period which began with the publication of the notice of availability of the DEAs in <u>The Environmental Notice</u> on August 23, 1997. The final Environmental Assessments must include these comments and responses, as well as revised text, if appropriate. Mr. Jeffrey Overton Page 2 October 7, 1997

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Should you have any questions, please contact Steve Tagawa of our staff at 523-4817.

Very truly yours,

For JAN NAOE SULLIVAN Director of Total

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Director of Land Utilization

JNS:am Encls.

cc: Henry and Rosaline Zane (w/o encls.) Jack Frost (w/o encls.) Roger and Jean Compton (w/o encls.) Harold and Pauline Masunaga (w/o encls.)

g:ppd\97sv3-6.sht



# 5 December 1997

Francis S. Oda, AIA, AICP
 Norman G.Y. Hong, AIA
 Sheryl B. Seaman, AIA, ASID
 Hitoshi Hida, AIA

INTERNATIONAL

Roy H. Nihei, AIA, CSI James J. Nishimoto, AIA

<sup>11</sup> Ralph E. Portmore, AICP
 Stephen H. Yuen, AIA
 <sup>11</sup> Linda L. Chung, AIA

Paul P. Chorney, AIA

Dean H. Kitamura, AIA ---- Norma J. Scott, AIA

- Stephen E. Callo, CPA
- George I. Atta, AICP Jeffrey H. Overton, AICP
- Kathryn A. Nam
- Roy A. Inouve
   Mary J. O'Leany
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Group 70 International, Inc. + Architecture + Planning + Interior Design + Environmental Services + Building Diagnostics + Assets Management 925 Bethel Street, Fifth Floor + Honolulu, Hawaii 96813-4307 + Phone (808) 523-5866 + FAX (808) 523-5874 + http://www.group70int.com

Jan Naoe Sullivan, Director Department of Land Utilization City and County of Honolulu 650 South King Street, 7th Floor Honolulu, HI 96813

Dear Ms. Sullivan:

Subject:

Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments

We have received your letter dated October 8, 1997 transmitting the comments provided on the Draft Environmental Assessments for the subject properties. Group 70 has prepared written responses to comments received during the 30day comment period. The Final Environmental Assessments will include these comments and responses, as well as revised text, as appropriate. Based on the EA process, we recommend a Finding of No Significant Impact (FONSI).

The owners are anxious for the Shoreline Setback Variance application to be processed. On September 23, 1997, the North Shore Neighborhood Board No. 27 voted to recommend DLU approval of the four variance requests.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

Sincerely,

GROUP 70 INTERNATIONAL, INC.

Jeffrey H. Overton, AICP Chief Environmental Planner

P:\Planning\97011-11 Compton SSV\EA RESPONSES\Ijns001jo\_moksw12037\_DLUresp rtf

	DEPARTMENT OF BUSINESS, ECONOMIC DEVELOPMENT & TOURISM	NJAMIN RADLEY DEPL	J. CAYETANO GOVERNOR SEIJI F. NA DIRECT J. MOSSM JTY DIRECTOR RICK EGGED OF PLANN
	OFFICE OF PLANNING       Tel.:         235 South Beretania Street, 6th Flr., Honolulu, Hawaii 96813       Fax:         Mailing Address: P.O. Box 2359, Honolulu, Hawaii 96804       Compared to the street of the stre	(808)	587-2840 587-2824
	Ref. No. P-6892 August 14, 1997	1997 AUG 25	1 51
	Ms. Jan Naoe Sullivan Director Department of Land Utilization City and County of Honolulu 650 S. King Street, 7th Floor Honolulu. Hawaii 96813	5 AN 10: 43	(1 1) 
	Dear Ms. Sullivan: Subject: Draft Environmental Assessments for Reconstruction of Seawall on Zane, Compton and Masunaga Properties in Mokuleia		
	This is in response to your letter of August 12, 1997, requesting review and comment on the subject draft environmental assessments. We have reviewed the assessments and have no comments to offer at this time.		ا ب : ن
	If there are any questions, please contact Jeffrey Walters of our CZM Program at 587-2883.		• •
	Sincerely		,
·	Rick Egged Director Office of Planning		·
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97-6004



DEPARTMENT OF THE ARMY U. S. ARMY ENGINEER DISTRICT, HONOLULU FT. SHAFTER, HAWAII 96858-5440

REPLY TO ATTENTION OF

August 19, 1997

1997 AUG 20 AM 10: 27 OLFT. OF LANG STRENZATION CITY & COUNTY OF HONOLULH

Planning and Operations Division

Ms. Jan Naoe Sullivan, Director City and County of Honolulu Department of Land Utilization 650 South King Street, 7th Floor Honolulu, Hawaii 96813

Dear Ms. Sullivan:

Thank you for the opportunity to review and comment on the Environmental Assessments (Eas) for Projects Within the Shoreline Setback (Zane, Frost, Compton, and Masunaga Seawall Reconstruction), Mokuleia, Oahu (TMK 6-8-10: 23-26). The following comments are provided in accordance with Corps of Engineers authorities to provide flood hazard information and to issue Department of the Army (DA) permits.

a. Under Section 10 of the Rivers and Harbors Act and Section 404 of the Clean Water Act, the Corps has regulatory jurisdiction in waters of the U.S., including navigable waters. Based on the information provided, the seawall reconstruction project will require a DA permit. Please contact Mr. Alan Everson of our Regulatory Section at 438-9258 for further information.

b. The flood hazard information provided on page 11 of each EA submitted is correct.

Sincerely,

Paul Mizue, P.E. Acting Chief, Planning and Operations Division



GROUP 70

# 5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Scaman, AIA, ASID Hitoshi Hida, AIA Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA

Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA Stephen E. Callo, CPA George I. Atta, AICP Jeffrey H. Overton, AICP Kathryn A. Nam Roy A. Inouye Mary J. O'Leary Paul Mizue, P.E. Acting Chief, Planning and Operations Division U.S. Army Engineer District, Honolulu Fort Shafter, Hawaii 96858-5440

Dear Mr. Mizue:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments ő. I

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We have received a copy of your letter to the Department of Land Utilization dated August 19, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

We recognize that the seawall reconstruction will require a DA permit, and hereby request general permit coverage under Nationwide Permit 13. The four seawalls have portions of their foundations that will be placed within the mean high water limit. The total amount of fill to be placed within the jurisdictional area is approximately 285 cu. yd. Please refer to the attached exhibit and the Final EA for further detailed information.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information regarding our request for Nationwide Permit coverage.

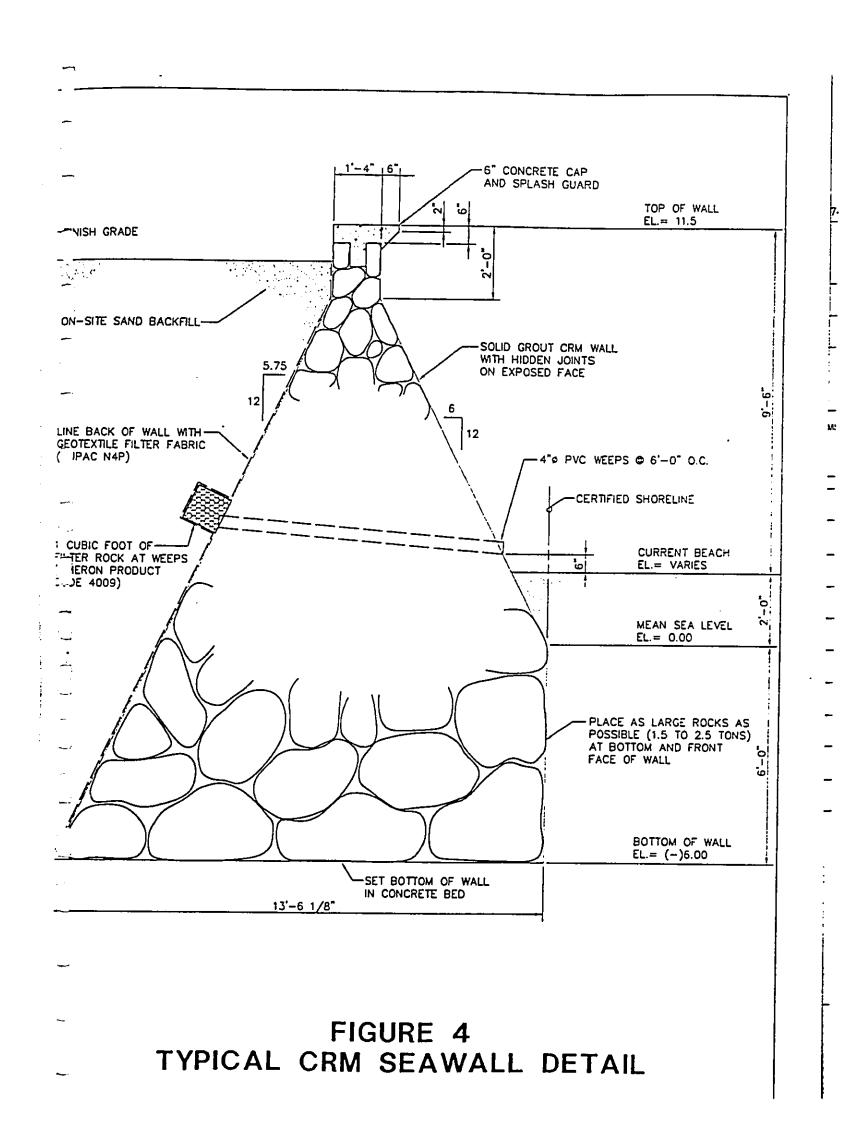
Sincerely,

**GROUP 70 INTERNATIONAL, INC.** 

Geffrey H. Overton, AICP Chief Environmental Planner

P:\Planning\97011-11 Compton SSV\EA RESPONSES\ldh001jo\_molaw11207\_ARMYresp.rtf

Group 70 International, Inc. • Architecture • Planning + Interior Design • Environmental Services • Building Diagnostics • Assets Management 925 Bethel Street, Fifth Floor • Honolulu, Hawaii 90813-4307 • Phone (808) 523-5866 • FAX (808) 523-5874 • http://www.eroup?0int.com



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IDN F. CAYETANO FOR OF HAWAB	÷		MICHARL D. WILFON, CHAIRFURFON ( BOARD OF LAND AND NATURAL RESOURCES
		(Aleraka)	DEPUTHES
	7 SEP -8 9:56		Gilbert Coloma-Agaran
66) C111	PT. OF LARE STREATION ( & COUNTY OF HONOLOLI)		ADUACULTURE DEVELOPMENT PROGRAM
	-	STATE OF HAWAII	AQUATIC RESOURCES CONSERVATION AND
	DEPARTMEN	NT OF LAND AND NATURAL RESO	URCES ENVIRONMENTAL AFFAIRS CONSERVATION AND
	3	TE HISTORIC PRESERVATION DIVISION IS SOUTH KING STREET, 6TH FLOOR HONOLULU, HAWAII 96813	RESOURCES ENFORCEMENT CONVEYANCES FORESTRY AND WILDUFE HISTORIC PRESERVATION
September	4, 1997		OVISION LAND MANAGEMENT STATE PARKS WATER AND LAND DEVELOPMENT
Jan Naoe S	ullivan, Director		
Department	t of Land Utilization		
•	unty of Honolulu		· · · · ·
650 South	King Street, 7th Flo	or	
	lawaii 96813		LOG NO: 19977 🛩
			DOC NO: 9708EJ41
Dear Ms. S	ullivan:		
SUBJECT:	Assessments, Sea (97/SV-004), Con		

A review of our records shows that there are no known historic sites at these parcels. The proposed project will repair existing seawalls on the individual parcels. Since any historic sites present at these parcels would have been disturbed by construction of the original seawall we believe that this repair project will have "no effect" on historic sites.

In the unlikely event that historic sites, including human burials, are uncovered during routine construction activities, all work in the vicinity must stop and the State Historic Preservation Division must be contacted at 587-0047.

Aloha,

DON HIBBARD, Administration State Historic Preservation Division

EJ:jk



5 December 1997

 Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA
 Sheryl B. Seaman, AIA, ASID
 Hitoshi Hida, AIA
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Stephen E. Callo, CPA
George I. Atta, AICP
Jeffrey H. Overton, AICP
Kathryn A. Nam
Roy A. Inouye
Mary J. O'Leary

Mr. Don Hibbard, Administrator State Historic Preservation Division Department of Land and Natural Resources 33 South King Street, 6th Floor Honolulu, HI 96813

Dear Mr. Hibbard:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments

We have received a copy of your letter to the Department of Land Utilization dated August 19, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

In the unlikely event that historic sites, including human burials, are uncovered during routine construction activities, all work in the vicinity will stop and the State Historic Preservation Division will be contacted.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

Sincerely,

GROUP 70 INTERNATIONAL, INC.

Feffrey H. Overton, AICP Chief Environmental Planner

P:\Planning\97011-11 Compton SSV\EA RESPONSES\ldh001jo\_moksw11207\_SHPDrespWD6.rtf

Group 70 International, Inc. + Architecture + Planning + Interior Design + Environmental Services + Building Diagnostics + Assets Management 925 Bethel Street, Fifth Floor + Honolulu, Hawaii 96813-4307 + Phone (808) 523-5866 + FAX (808) 523-5874 + http://www.group70int.com

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LAWRENCE MIKE

DIRECTOR OF HEALTH



STATE OF HAWAII DEPARTMENT OF HEALTH P.O. BOX 3378 HONOLULU, HAWAII 96801 September 8, 1997

in reply,	picase	refer	ю

97-170/epo

Ms. Jan Naoe Sullivan, Director Department of Land Utilization City & County of Honolulu 650 South King Street Honolulu, Hawaii 96813

Dear Ms. Sullivan:

· . .

BENJAMIN J. CAYETANO

**GOVERNOR OF HAWAII** 

Subject: ENVIRONMENTAL ASSESSMENTS FOR THE RECONSTRUCTION OF FOUR (4) SEAWALLS WITHIN THE SHORELINE SETBACK MOKULEIA, OAHU, HAWAII

> Zane Property, Lot 14 (97/SV-003) 68-695 Farrington Highway TMK: (1) 6-8-10: 23

Frost Property, Lot 13 (97/SV-004) 68-697 Farrington Highway TMK: (1) 6-8-10: 24

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CITY & COUNTY OF HONOLULI

Compton Property, Lot 12 (97/SV-005) 68-701 Farrington Highway TMK: (1) 6-8-10: 25 . .

Masunaga Property, Lot 11 (97/SV-006) 68-705 Farrington Highway TMK: (1) 6-8-10: 26

Thank you for allowing us to review and comment on the subject projects. We have the following comments to offer:

### Water Pollution

- The applicant should contact the Army Corps of Engineers to 1. identify whether a federal permit (including a Department of Army permit) is required for this project. If a federal permit is required, then a Section 401 Water Quality Certification is required from the State Department of Health, Clean Water Branch.
- A National Pollutant Discharge Elimination System (NPDES) 2. general permit is required for the following discharges to waters of the State:
  - Storm water discharges relating to construction a. activities, such as clearing, grading, and excavation, for projects equal to or greater than five acres;
  - Storm water discharges from industrial activities; b.

<b>-</b> .	
	Ms. Jan N. Sullivan, Director 97-170/epo
-	September 8, 1997 Page 2
~	c. Construction dewatering activities;
	d. Noncontact cooling water discharges less than one million gallons per day;
	e. Treated groundwater from underground storage tank remedial activities; and
	f. Hydrotesting water.
	Any person requesting to be covered by a NPDES <u>general</u> permit for any of the above activities should file a Notice of Intent with the Department's Clean Water Branch at least 30 days prior to commencement of any discharge to waters of the State.
	3. After construction of the proposed facility is completed, an NPDES <u>individual</u> permit will be required if the operation of the facility involves any wastewater discharge into State waters.
<b>-</b>	Any questions regarding these comments should be directed to Mr. Denis Lau, Branch Chief, Clean Water Branch at 586-4309.
	Noise Concerns
l mark	Construction activities must comply with the provisions of Hawaii Administrative Rules, Chapter 11-46, "Community Noise Control."
	a. The contractor must obtain a noise permit if the noise levels from the construction activities are expected to exceed the allowable levels of the regulations as stated in Section 11-46-6(a).
· · ·	b. The contractor must comply with the conditional use of the permit as specified in the regulations and the conditions issued with the permit as stated in Section 11-46-7(d)(4).
•'	Should there be any questions regarding these comments, please contact Mr. Jerry Haruno, Environmental Health Program Manager of the Noise, Radiation & Indoor Air Quality Branch at 586-4701.
<u> </u>	Sincerely,
_	Sunghadman
_	BRUCE S. ANDERSON, Ph.D. Deputy Director for Environmental Health
	C: CWB NR&IAQB
-	



GROUP 70

5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Seaman, AIA, ASID Hutoshi Hida, AIA Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA

Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA Stephen E. Callo, CPA George I. Atta, AICP Jeffrey H. Overton, AICP Kathryn A. Nam Roy A. Inouye Mary J. O'Leary Bruce S. Anderson, Ph.D. Deputy Director for Environmental Health Department of Health State of Hawaii P. O. Box 3378 Honolulu, HI 96801

Dear Dr. Anderson:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments ы-н 1

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We have received a copy of your letter to the Department of Land Utilization dated September 8, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

1. U.S. Army Corps of Engineers. We have received comments from the Army Corps stating that the four subject properties will require a Department of Army permit for fill within the mean high water line, jurisdictional waters of the United States. It is anticipated that a DA General Permit will be obtained for these properties sometime early next year. We will concurrently request a Section 401 Water Quality Certification from the State Department of Health, Clean Water Branch.

2. National Pollutant Discharge Elimination System (NPDES). The project will not involve an area of five acres, therefore, the construction stormwater permit will not be applicable. Industrial activities, cooling waters, underground storage tank and hydrotesting are not involved with the proposed project. There may be the need for dewatering during the construction period, and we will consult with the Clean Water Branch to determine the applicability of this permit to the construction process planned for these shoreline structures. There will be no ongoing discharge of wastewater to State waters following construction.

Group 70 International, Inc. • Architecture • Planning • Interior Design • Environmental Services • Building Diagnostics • Assets Management 925 Bethel Street, Fifth Floor • Honolulu, Hawaii 96813-4307 • Phone (808) 523-5866 • FAX (808) 523-5874 • http://www.group70int.com Letter to Dr. Bruce S. Anderson December 5, 1997 Page 2

3. Noise Levels. Noise from construction activities is not expected to exceed the allowable levels of the regulations stated in Section 11-46-6(a). Should a noise permit be required, the contractor will be responsible for obtaining a noise permit and for compliance with the conditional use of the permit as specified.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

Sincerely,

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GROUP 70 INTERNATIONAL, INC.

Jéffréy H. Overton, AICP Chief Environmental Planner

P:\Planning\97011-11 Compton S5V\EA RESPONSES\Ibsa001jo\_moksw11207\_DOHresp.rtf

Group 70 International, Inc. • Architecture • Planning • Interior Design • Environmental Services • Building Diagnostics • Assets Management 925 Bethel Street, Fifth Floor • Honolulu, Hawaii 96813-4307 • Phone (808) 523-5866 • FAX (808) 523-5874 • http://www.groupToint.com • m.ol@groupToint.com

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AQUACULTURE DEVELOPMENT PROGRAM AQUATIC RESOURCES BOATING AND OCEAN RECREATION CONSERVATION AND EMVIRONMENTAL AFFAIRS CONSERVATION AND RESOURCES ENFORCEMENT

CONVEYANCES FORESTRY AND WILDLIFE

HISTORIC PRESERVATION LAND MANAGEMENT STATE PARKS

WATER AND LAND DEVELOPMENT WATER RESOURCE MANAGEMENT

CHY & COUNTY OF HONOLUL



## STATE OF HAWAII DEPARTMENT OF LAND AND NATURAL RESOURCES

P.O. BOX 621 HONOLULU, HAWAII 96809

September 22, 1997

LD-NAV REF.: 97SV003.RCM

Honorable Jan Naoe Sullivan Director of Land Utilization City and County of Honolulu 650 S. King Street 7th Floor Honolulu, Hawaii 96813

Dear Ms. Sullivan:

SUBJECT:		Environmental Assessment( <i>5</i> ) 97/SV-003, 97/SV-004, 97/ <i>S</i> V-005 and 97/SV-006
		Reconstruction of Seawall (s) Zane, Frost, Compton & Masunaga
	Appircancs.	Zane, Frost, Compton & Masquaga
	Location :	68-695, 68-697, 68-701 & 68-705 Farrington
		Highway, Mokuleia, Island Of Oahu, Hawaii
	<u>TMKs</u> :	1st/ 6-8-10: 23, 24, 25 and 26

Thank you for the opportunity to review and comment on the subject Environmental Assessment for the proposed project.

Our Land Division Planning and Technical Services reviewed the documentation submitted to substantiate the subject request for the Shoreline Setback Variances from the City and County of Honolulu, and have the following comments.

We note that the project consists of removing four deteriorated vertical seawalls built to protect four homes on adjacent parcels on an eroding section of beach, and then rebuilding a properly designed, massive vertical seawall at the same location on the four parcels.

The existing walls were built over a period of years during the 1960s and 70s, for the most part without the benefit of any engineering or building permit review or any land use approvals, as is now evidenced by their catastrophic failure.

Further, the entire project will occur on private, residential-zoned land mauka of the shoreline.

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Page 2 Environmental Assessment 97/SV-003

Given the circumstances of the current condition of the existing walls, and the advanced state of erosion and deterioration of the beach, as well as the lack of feasible options in this area, we do not object to the project. However, we would like to see any approvals conditioned such that all the remnants of the collapsed walls be removed from the area seaward of the shoreline to provide a maximum amount of safe, useable open space along the beach.

Finally, although the submitted documentation was generally well-written and provided useful information, we wish to make some comments on certain points:

From the information included in the discussions and l) tables on shoreline retreat based on movement of the vegetation line and diminishing width of beach sand, it seems fairly obvious that when the seawalls were built (primarily in the '67 to '69 period, as the beach was re-accreting from the pre-'67 erosion period), they were all located too far makai onto the unstable, recently-accreted portion of the beach. Had they instead been located at the mauka extent of the erosion/vegetation line, there would have been more open beach space for the natural littoral processes to occur, and perhaps there would have been much less interaction between the wash of the waves and the (poorly-designed) walls. This may have resulted in less beach loss, and less damage to the walls. Hopefully a lesson can be learned from this.

Unpermitted walls are often poorly-designed walls, and 2) when they ultimately fail, the public trust resources suffer.

Although the claim is made in the draft environmental 3) assessment (DEA) that the attached oceanographic study by TNWRE "shows that erosion of the adjacent beach area is not being accelerated by the presence of the subject existing walls," we find this claim to be unsubstantiated. No where does the study indicate that the beach erosion problem has not been accelerated by the presence of the walls; in fact, it suggests the opposite may be true.

The study indicates substantial beach loss occurred after the subject walls were initially constructed, even though the beach was generally accreting at the time. Further, it claims that three other recently reconstructed walls immediately to the west of these parcels has put these four parcels in imminent need of protection, apparently because of how seawalls generally do exacerbate neighboring beach erosion/beach loss problems; if the neighboring walls are putting these lots in imminent peril due to exacerbated beach erosion, then these subject walls certainly can cause beach erosion and loss too

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Page 3 Environmental Assessment 97/SV-003

4) We wonder why the DEA concludes by requesting an <u>after-</u> <u>the-fact</u> variance, when we understand that it is the proposed new wall for which approval is sought.

The Department of Land and Natural Resources has no other comments to offer on the subject matter at this time.

Should you have any questions, please contact Nicholas Vaccaro of our Land Division's Support Services Branch at 587-0438 or Tom Eisen of the Planning and Technical Services Branch at 587-0386.

HAWAII: Earth's best!

Aloha,

MICHAEL D. WILSON

c: Oahu Land Board Member At Large Land Board Member Oahu District Land Office



5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Seaman, AIA, ASID Hitoshi Hida, AIA Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA

Stephen E. Callo, CPA George I. Atta, AICP Jeffrey H. Overton, AICP Kathryn A. Nam Roy A. Inouye

Mary J. O'Leary

Mr. Michael D. Wilson, Director Department of Land and Natural Resources P.O. Box 621 Honolulu, HI 96809

Dear Mr. Wilson:

Subject:Shore Setback Variances for Reconstruction of SeawallsZane, Frost, Compton & Masunaga Properties, Mokuleia, OahuTMK (1) 6-8-10:23, 24, 25 and 26Responses to Comments on Draft Environmental Assessments

We have received a copy of your letter to the Department of Land Utilization dated September 22, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

1) Locations for New Seawalls. The new seawalls will be built at the location of the current seawalls, and cannot be moved further inland due to the proximity of the improvements and shallow lot depth of each property.

2) Seawall Design. The four new walls will be built with a modified vertical seawall design to include a sloped boulder base section. This design will aid by providing energy dissipation and better aesthetics than the vertical CMU walls being replaced. The footing for the new walls will be placed six feet below sea level to minimize undermining which has caused the existing walls to fail.

3) Beach Loss for the Past 40 Years. The aerial photograph history shows the inland progression of the beach at the rate of approximately one foct per year. This rate of retreat can be shown for the 20-year period preceding the construction of seawalls. Measurements of the inland progression over the past 20 years do not indicate the walls to have accelerated this retreat.

The study does not find that the recently reconstructed walls to the west are a threat to the four subject walls. The modified vertical seawall design has been

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Letter to Mr. Michael D. Wilson 5 December 1997 Page 2

shown to be effective, and a narrow beach exists along the frontage at these walls where no beach existed previously.

4) Permit for Seawall Reconstruction. We appreciate your correction of our error stating that the shoreline setback variance is an after-the-fact permit. A new variance and building permit will be obtained for each seawall. One . exception is that the Zane family had previously obtained a building permit for their existing seawall.

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Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

Sincerely,

GROUP 70 INTERNATIONAL, INC.

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Jeffrey H. Overton, AICP Chief Environmental Planner

Group 70 International, Inc. • Architecture • Planning • Interior Design • Environmental Services • Building Diagnostics • Assets Management 925 Bethel Street, Fifth Floor • Honolulu, Hawaii 96813-4307 • Phone (808) 523-5866 • FAX (808) 523-5874 • http://www.group?0int.com • mail@group70int.com



STATE OF HAWAI'I OFFICE OF HAWAIIAN AFFAIRS 711 KAPI'OLANI BOULEVARD, SUITE 500 HONOLULU, HAWAI'I 96813-5249 PHONE (808) 594-1888 FAX (808) 594-1865

September 11, 1997

Jeffrey Overton Group 70 International, Inc. 925 Bethel Street, 5th Floor Honolulu, Hawaii 96813

Re: Draft Environmental Assessments and Applications for Shoreline Setback Variance for Reconstruction of Seawalls at TMK's: 6- 8-10:26(Lot 11), 6-8-10:25(Lot 12), 6-8-10:24(Lot 13), and 6-8-10:23(Lot 14).

Dear Mr. Overton:

Thank you very much for the opportunity to review the four above-referenced Shoreline Setback Variance Applications and Draft Environmental Assessments (DEA).

The applicants Masunaga (Lot 11), Compton (Lot 12), Frost (Lot 13), and Zane (Lot) 14 are proposing to construct 9.5 foot modified seawall structures of grouted rocks and boulders to protect their shoreline properties. All of the subject properties are situated adjacent to one another and are located on Farrington Highway in Mokuleia directly across the eastern entrance of Dillingham Airfield.

The Office of Hawaiian Affairs (OHA) understands the imminent threat to the residential structures on the subject parcels, and that the construction of these seawalls may be the most feasible alternative for the applicants at this time.

However, OHA does have several concerns regarding the proposed type of development and with some of the information presented in the DEA. OHA's main concerns with the proposed seawalls relate to shoreline access, safety hazards, longterm shoreline processes, and increased erosion to adjacent shoreline areas.

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Letter to Jeffrey Overton Page two

First, The preparers of the DEA conclude that the Oceanographic and Structural Evaluation (Exhibit C) "shows that the erosion of the adjacent beach areas is not being accelerated by the presence of this structure" (page 3).

We agree that without the seawall structures erosion along the seaward frontage of the subject properties would likely occur. However, it is not accurate to conclude that the erosion of beach areas are not being "accelerated" by such structures. The net effect of armoring structures (especially seawalls) is the reflection of wave energy, which causes increased sand scouring and beach loss. It is highly probable that shoreline retreat is indeed accelerated in the process.

Second, the preparers conclude that "the impact on shoreline processes of the seawall has been negligible due to the presence of shoreline structures on adjoining lots" (page 5).

This determination of "negligible impact" by the seawalls is based upon the existence of similar shoreline structures on adjoining lots. This conclusion is not only unfounded and inaccurate, but it is contrary to the data presented in the Oceanographic and Structural Evaluation (Exhibit C, pp. 7-8).

It is clear that the impacts of seawalls on shoreline processes are adverse and by no means "negligible". In fact, the armoring of the shoreline has a <u>major</u> impact on the natural littoral processes of erosion and accretion.

The continued construction of revetments and seawalls results in even greater erosion, and the transfer of erosion problems to adjacent shoreline properties. This leads to further construction of erosion-control structures and the eventual "hardening" of the shoreline.

Third, OHA has concerns about the restriction of lateral shoreline access as a result of seawall construction. The DEAs state that "public access to the beach fronting the properties will not be affected". In the same paragraph it is stated that the beaches fronting these properties are "very narrow...(and) are affected by wave run-up during high tides, particularly during high surf events" (page 9).

OHA believes that lateral shoreline access will be affected by these seawalls because of the continued erosion of the fronting beaches. Furthermore, any access by the public to these "very narrow" beaches could be extremely dangerous presenting a serious safety hazard.

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Letter to Jeffrey Overton Page three

Figure 7 in the DEAs illustrate the design of the proposed seawall structures. The tops of the proposed seawalls are only 1' 10" wide. This hardly seems adequate to allow "safe" lateral access to the shoreline especially during times of high surf events. The proposed seawalls should be designed to allow shoreline access without presenting a safety hazard to the public.

The conflict between the protection of private property and the preservation of public beaches in the context of coastal zone management in Hawaii is an unresolved issue which needs to be addressed. The preparers of the DEA are not expected to address this issue directly. However, it should not be glossed over in the DEA by the presentation of inconclusive evidence and broadbased assumptions.

OHA would appreciate the applicant's cooperation by providing our office with a written response to the above concerns. If you have any questions or need additional information, please contact Lynn Lee, Acting Land and Natural Resources Division Officer or Richard Stook, EIS Planner at (594-1888).

Randall Ogata Administrator

Lynn Lee, Acting Officer,

Sincerely yours,

Land & Natural Resources

RS:rs

cc: Trustee Clayton Hee, Board Chair Trustee Rowena Akana, Land & Sovereignty Chair Trustee Abraham Aiona, Board Vice-Chair Trustee Haunani Apoliona Trustee Billie Beamer Trustee Frenchy DeSoto Trustee Moses Keale Trustee Collette Machado Trustee Hannah Springer



#### 5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Seaman, AIA, ASID Hitoshi Hida, AIA Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA

Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA Stephen E. Callo, CPA George I. Atta, AICP Jeffrey H. Ovenon, AICP Kathryn A. Nam Roy A. Inouye Mary J. O'Leary Mr. Randall Ogata, Administrator Ms. Lynn Lee, Acting Officer, Land & Natural Resources Office of Hawaiian Affairs State of Hawai'i 711 Kapi'olani Boulevard, Suite 500 Honolulu, Hawai'i 96813-5249

Dear Mr. Ogata and Ms. Lee:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments 6.4

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Thank you for your letter to the Department of Land Utilization dated September 11, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

1. Erosion of Adjacent Beach Area/Impact on Shoreline Processes. We have obtained aerial photography for this section of the coastline dating back to 1949. The presence of the four vertical seawalls does not indicate direct evidence of accelerated shoreline retreat due to the seawall structures on these properties. The beach has retreated at an average rate of one foot per year since 1949. This rate of retreat applies to the 20-30 years before the subject seawalls were built. It is clear that the current situation is a constant reflection of energy and scouring of sand from the area fronting the seawalls.

Your comments are well taken from a pure academic examination of vertical structures on sand beaches. It is important to understand that each and every shoreline situation is unique. However, if the trend of shoreline retreat was allowed to continue at these properties, the beach would have continued to retreat at a rate of approximately one foot per year, and these four house lots would now be eliminated. Of note, if these four lot owners did not armor their shoreline sections, Farrington Highway would have become threatened by

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shoreline erosion, and armoring of the road by the State would likely have been undertaken.

2. Lateral Shoreline Access. The four seawalls are aligned together and match the adjoining properties on either side. At this location, there are 16 properties in a row that are all protected by vertical seawalls. The beach along this entire section of the Mokuleia coast could be considered very narrow. Lateral passage along this shoreline in front of the seawalls will be improved by the proposed action, which will remove loose rock debris and deteriorated wall sections, and replace these with an engineered modified vertical seawall structure. With the lower sloped portion consisting of large rock material, there will be a return of a slightly wider beach for lateral passage. At the three lots to the west where modified vertical seawalls were recently built with the hybrid wall design, there has been a recent return of a narrow strip of beach sand along a section where there was no beach sand for lateral passage in the recent past.

It was never intended for people to walk along the top of the 9-foot high walls, as this would be hazardous. People who like to walk this portion of the beach for fishing, gathering or other recreation will be able to continue this practice without interruption. The new walls will not diminish lateral access and are likely to improve it through the new hybrid wall design, as shown by the trend of the beach fronting the three lots to the west.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

Sincerely,

GROUP 70 INTERNATIONAL, INC.

Jeffrey H. Overton, AICP Chief Environmental Planner

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GARY GILL DIRECTOR



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BENJAMIN J. CAYETANO GOVERNOR

# STATE OF HAWAII

OFFICE OF ENVIRONMENTAL QUALITY CONTROL

236 SOUTH BERETANIA STREET SUITE 702 HONOLULU, HAWAII 96813 TELEPHONE (808) 586-4185 FACSIMILE (808) 596-4186

September 15, 1997

Ms. Jan Nace Sullivan Director of Land Utilization City and County of Honolulu 650 South King Street Honolulu, Hawaii 96813

Dear Ms. Sullivan:

Subject: Compton Seawall Reconstruction, Mokuleia, Oahu

It is the policy of the State of Hawaii under HRS Chapter 205A to discourage all shoreline hardening that may affect access to, or the configuration of, our island beaches.

Any EA prepared in conjunction with an application to construct a seawall, revetment or similar structure should be accompanied by appropriate justification and detailed studies including, but not limited to, the following:

- 1. A Historical Shoreline Analysis of coastal erosion and accretion rates. This should include a description of all movements of the neighboring shoreline over at least the past 30 years. This analysis should be based, at least in part, on aerial photographs available through government agencies and private vendors. The analysis should provide a detailed history of erosion and accretion patterns using all available evidence.
- A description of the nature of the affected shoreline, whether sandy, rocky, mud flats or any other configuration. The history and characteristics of adjoining sand dunes and reefs should be included.
- 3. Site maps that clearly show the current certified shoreline, previous certified shorelines, the private property line and the location of the proposed structure. Any nearby public access right-of-way should also be depicted.
- 4. Beach profiles that extend off shore at appropriate intervals along the beach indicating the width and slope of both the submerged and dry portions of the beach.

Ms. Sullivan September 15, 1997 Page 2

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- 5. An analysis of any existing nearby walls or revetments and their cumulative impacts on the shoreline.
- 6. A description of structures and improvements (such as homes or swimming pools) on the subject property, their distance from the property line and shoreline, and how they may be affected by the construction of the proposed hardening project.
- 7. A wave and storm frequency analysis for the area in question. This should include any relevant coastal processes such as longshore currents and seasonal wave patterns.
- 8. An analysis that predicts the location of future shorelines with and without the proposed wall at least 30 years into the future or over the expected life of the hardening project.
- 9. Photos of the site that illustrate past and present conditions and locate the proposed structure.
- 10. All alternatives to shoreline hardening should be thoroughly researched and analyzed. These alternatives should include beach replenishment, dune-scaping, retreat from the shoreline by moving existing structures inland, and a no action alternative.

The inclusion of this information will help make an Environmental Assessment complete and meet the requirements of Chapter 343, HRS. Our review of the draft environmental assessment indicates that many of these points have been addressed. Please answer the remaining questions (highlighted in bold text) in the final environmental assessment. Only after thorough study and analysis should any permit for shoreline hardening be considered.

Should you have any questions please call Jeyan Thirugnanam at 586-4185.

Sincerely, Gary Gil! Director

c: Roger and Jean Compton



INTERNATIONAL

#### 5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Seaman, AIA, ASID Hitoshi Hida, AIA Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA

Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA Stephen E. Callo, CPA George I. Atta, AICP Jeffrey H. Overton, AICP Kathryn A. Nam Roy A. Inouye Mary J. O'Leary Mr. Gary Gill, Director Office of Environmental Quality Control State of Hawai'i 236 South Beretania Street, Suite 702 Honolulu, HI 96813

Dear Mr. Gill:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments 6- L

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We have received a copy of your letter to the Department of Land Utilization dated September 15, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties, and these are incorporated into the Final EA in accordance with your request.

1) Pubic Access. The closest public access right-of-way is depicted in the TMK map shown in Figure 2. An access easement is found approximately 500 feet to the east off Ho'omana Street.

2) Future Shoreline Prediction. The predicted location of the shoreline with and without the proposed improvement is discussed indirectly in the Draft EA. The current shoreline is found at the toe of the existing wall, and is anticipated to remain at this location with the new modified vertical seawall. Without the reconstructed wall, the rate of shoreline retreat at this location over the past 50 years will likely continue at one foot per year. With a lot depth of approximately 80 feet, the new shoreline in 30 years without a structure would likely occur at least 30 feet inland. Unless the inland progression of the shoreline slowed at this location, the shoreline would eventually be found at the toe of a new shoreline structure that would need to be built by the State Department of Transportation to protect Farrington Highway.

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3) Dune Scaping and Moving Existing Structures. The alternatives to the proposed action for creating artificial dunes or moving the existing home further mauka are not practical. There is no room to move the homes further mauka. This shoreline is a high-energy environment that could possibly be stabilized temporarily by sand replenishment and dune creation. The quantity of sand fill material would be tremendous – to provide a 100 foot wide beach with a single dune across the entire 1,800-foot coastal cell would require depositing over 40,000 cu. yd. of sand. At a unit cost of anywhere from \$125 to \$250/cu. yd., the estimated cost for such a project would be \$5.0 to 10.0 million, which is prohibitively expensive for these 16 homeowners. Further, the creation of a new beach would have little chance of remaining given the existing energy regime along this coast, without structural containment measures such as a groin field. The added sand would most likely be carried off this shore and could pose a risk to the nearshore reef ecosystem.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

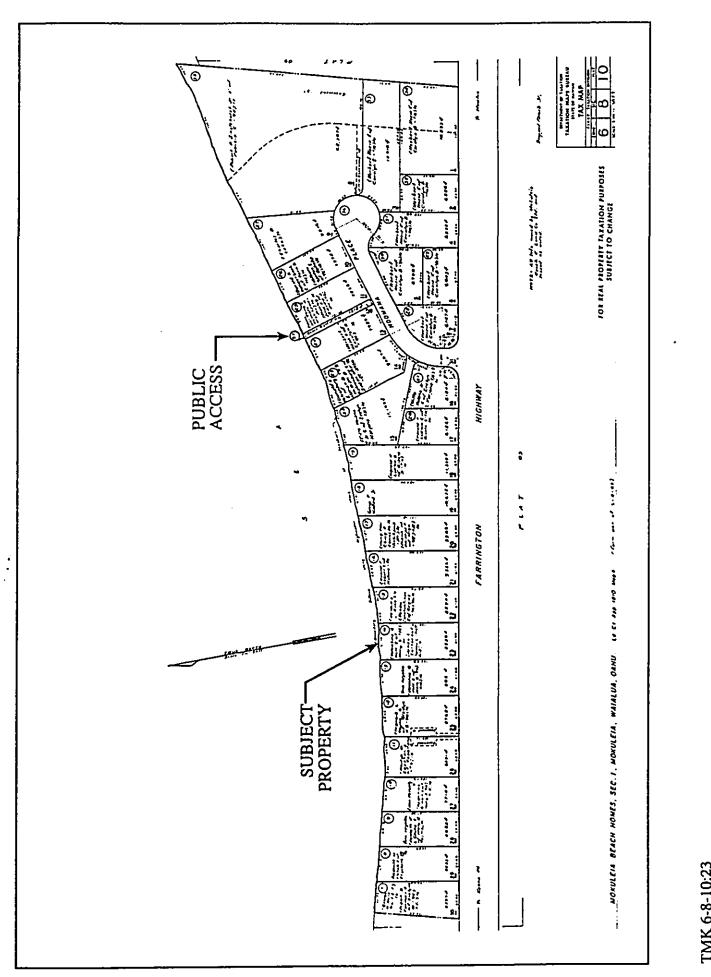
Sincerely,

GROUP 70 INTERNATIONAL, INC.

Jeffrey H. Overton, AICP Chief Environmental Planner

P:\Planning\97011-11 Compton S5V\EA RESPONSES\igg001jo\_molaw11207\_OEQCrespWD6.rtf

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Figure 2

TMK 6-8-10:23 Shoreline Setback Variance

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WILLIAM D. BALFOUR, JR. ACTING DIRECTOR

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COUNTY OF HONDLOLD

# DEPARTMENT OF PARKS AND RECREATION

# CITY AND COUNTY OF HONOLULU

650 SOUTH KING STREET HONOLULU, HAWAII 96813

JEREMY HARRIS MAYOR



September 18, 1997

TO:

# DEPARTMENT OF LAND UTILIZATION WILLIAM D. BALFOUR, JR., ACTING DIRECTOR

JAN NAOE SULLIVAN, DIRECTOR

FROM:

SUBJECT: ENVIRONMENTAL ASSESSMENTS, CHAPTER 343, HRS PROJECTS WITHIN THE SHORELINE SETBACK SEAWALL RECONSTRUCTION ZANE (97/SV-003), FROST (97/SV-004), COMPTON (97/SV-005) and MASUNAGA (97/SV-006) 68-695, 68-697, 68-701, & 68-705 FARRINGTON HIGHWAY MOKULEIA, OAHU, HAWAII TAX MAP KEYS 6-8-10: 23, 24, 25, & 26

Thank you for the opportunity to review and comment on the draft environmental assessment for the above-mentioned projects.

Of the three options proposed for the protection of the above properties, Option b, "Construct a sloping rock revetment in place of the vertical seawall" is preferred. A gentle sloping boulder revetment would provide greater wave energy dissipation and allow for lateral access along the shoreline.

The proposed seawall will probably lead to further beach narrowing and loss. The loss of the beach would mean a loss of lateral access and curtail recreational use.

Please have your staff contact Mr. Carl Emura, Planner, of our Advance Planning Branch, at extension 6301 if you need further information.

w.D.Balfom.J.

WILLIAM D. BALFOUR, JR. Acting Director

WDB:ei

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#### 5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Seaman, AIA, ASID Hitoshi Hida, AIA Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA

Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA Stephen E. Callo, CPA George I. Atta, AICP Jeffrey H. Ovenon, AICP Kathryn A. Nam Roy A. Inouye Mary J. O'Leary Mr. William D. Balfour, Jr., Acting Director Department of Parks and Recreation City and County of Honolulu 650 South King Street, 5th Floor Honolulu, HI 96813

Dear Mr. Mizue:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments 6....

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We have received a copy of your letter to the Department of Land Utilization dated September 18, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

1) Sloping Rock Revetment Alternative. We recognize that the revetment structure would provide additional energy dissipation, however, the physical limitations of the four lots studied do not allow for the construction of sloping rock revetments. The revetment structure would extend to the existing residences, entirely eliminating the makai-side yards of these properties. The revetment would be added potential for wave run-up during very high surf conditions as compared to a vertical seawall. The proposed design of a modified vertical seawall, with a sloping lower half consisting of boulders, will provide some revetment style while retaining a yard.

2) Beach Narrowing and Lateral Access. The beach fronting the four seawalls is currently very narrow, and there is very little beach to be narrowed along the front of these walls. People can now transit the area fronting these walls for recreational purposes, and the new seawalls will not reduce lateral access. The proposed modified vertical seawall will allow for improved lateral access, and we anticipate some sand to return due to the change in structure type. The same design was used for the three lots to the west, and there has been a return of a narrow sand beach in that area.

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Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

Sincerely,

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GROUP 70 INTERNATIONAL, INC.

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Jeffrey H. Overton, AICP Chief Environmental Planner

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-Group 70 International, Inc. • Architecture • Planning • Interior Design • Environmental Services • Building Diagnostics • Assets Management 925 Bethel Street, Fifth Floor • Honolulu, Hawaii 96813-4307 • Phone (808) 523-5866 • FAX (808) 523-5879 • http://www.group70int.com • mail@group70int.com

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Y HARRIS AYON		JONATHAN K, SHIMADA, PHD
	September 15, 1997	ENV 97-188
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MEMORANDU	<u>M:</u>	
то:	JAN NAOE SULLIVAN, DIRECTOR DEPARTMENT OF LAND UTILIZATION	
FROM:	JONATHAN K. SHIMADA, PhD DIRECTOR AND CHIEF ENGINEER	nit = 1
SUBJECT:	ENVIRONMENTAL ASSESSMENT (EA) PROJECTS WITHIN SHORELINE SETBACK TMK: VARIOUS	

	detail. For example, what time of the year (wet and dry season) be considered in the construction? What is the anticipated duration of construction? Will water be directed from shoreline during construction of the seawall? Will immediate planting be made to mitigate erosion of silt and sediment?
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If you have any questions, please contact Alex Ho at Local 4150.

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5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA
Sheryl B. Seaman, AIA, ASID
Hitoshi Hida, AIA
Roy H. Nihei, AIA, CSI
James I. Nishimoto, AIA
Ralph E. Portmore, AICP
Stephen H. Yuen, AIA
Linda L. Chung, AIA

Paul P, Chorney, AIA
 Dean H, Kitamura, AIA
 Norma J, Scott, AIA
 Stephen E, Callo, CPA
 George I, Alta, AICP
 Jeffrey H, Overton, AICP
 Kathryn A, Nam
 Roy A, Inouye
 Mary J, O'Leary

Dr. Jonathan K. Shimada, Director and Chief Engineer Department of Public Works City and County of Honolulu 650 South King Street, 11<sup>th</sup> Floor Honolulu, HI 96813

Dear Dr. Shimada:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments

We have received a copy of your memo to the Department of Land Utilization dated September 15, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

1) Seawall Construction. The design for the new seawalls will set the foundation at six feet below the mean sea level. The foundation is planned to rest approximately four feet below the lowest tide elevation of -2.01.

2) Mitigating Measures. Construction is planned for next year during the low surf season, which occurs generally from May to August. This time frame also tends to be the drier time of the year. The construction will require approximately six to eight weeks to complete. There will be a need to deflect water from the work area during the construction of the foundation and lower section of the walls. This will be accomplished by constructing a small berm on the beach or using temporary sheet piles.

There is no possibility to establish plants on the makai side of the wall due to the narrow width of the beach. The introduction of suspended sediments to coastal waters will be minimized, following conditions imposed under the Army Corps of Engineers Permit and State Water Quality Certification.

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Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

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Sincerely,

GROUP 70 INTERNATIONAL, INC.

Jeffrey H. Overton, AICP Chief Environmental Planner

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O'AHU GROUP SIERRA CLUB, HAWAI'I CHAPTER P.O. Box 2577, Honolulu, Hawaii 96803 Phone: (808) 538-6616 rat sep 18 .... 10: 53 -

FIT & DAME I HAMPENIE

September 10, 1997

Jan Sullivan Director Department of Land Utilization 650 S. King St 7th Floor Honolulu, HI 96813

Dear Ms. Sullivan,

RE: WAIALUA SEAWALL VARIANCE APPLICATIONS

The O'ahu Group of the Sierra Club has concerns regarding the four applications for shoreline setback variances in Waialua. Please include this in the public record for both the variance application and the environmental assessment for all four applications.

Shoreline Setback Variances must be consistent with the objectives and policies of HRS 205A-2. 205A-4(b) These objectives and policies include:

- providing recreational opportunities accessible to the public;

- protecting the quality of coastal scenic and open space resources;

- protecting beaches for public use and recreation;

- providing and managing adequate public access to and along shorelines with recreational values; and

- prohibiting construction of private erosion-protection structures seaward of the shoreline, except when they result in improved aesthetic and engineering solutions to erosion at the sites and do not interfere with existing recreational and waterline activities.

No variance may be granted unless safe lateral access to and along the shoreline is provided. 205A-46(c)(1) No variance may be granted unless conditions are imposed to minimize adverse impacts to beach processes. 205A-46(c)(2). No variance may be granted unless conditions are imposed to minimize loose rocks from impacting public property. 205A-46(c)(3). No variance may be granted unless conditions are imposed to minimize adverse impacts on public views. 205A-46(c)(4).

We know that 25% -- about 10 miles -- of Oahu's beaches have eroded thanks to coastal armoring. Studies done by the Army Corps of Engineers, the University of Hawai'i and the Coastal Zone Management Program (all of which DLU has in its records and all of which are incorporated into the record by reference) demonstrate that if a shoreline is undergoing long-term retreat,

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beach narrowing and loss can be expected if the beach is armored. See, e.g., Hwang and Fletcher, Beach Management Plan with Beach Management Districts (June 1992).

The loose boulders along the shoreline placed by the applicants make lateral access almost impossible. The existing seawalls along the coastline appear to have caused extensive beach erosion.

What kind of assurance do the applicants provide that recreational resources of the beach will be able to be enjoyed by the public? What kind of lateral access is provided?

If DLU imprudently grants the variance, at the very least, it should have an expiration date to ensure that no vested right is granted.

Sincerely, Halle Philip Bogetto

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Chair



5 December 1997

Francis S. Oda, AIA, AICP Norman G.Y. Hong, AIA Sheryl B. Seaman, AIA, ASID Hitoshi Hida, AIA , Roy H. Nihei, AIA, CSI James I. Nishimoto, AIA Ralph E. Portmore, AICP Stephen H. Yuen, AIA Linda L. Chung, AIA ..... Paul P. Chorney, AIA Dean H. Kitamura, AIA Norma J. Scott, AIA Stephen E. Callo, CPA "" George I. Ana, AICP leffrey H. Overton, AICP Kathryn A. Nam Roy A. Inouve Mary J. O'Leary

Mr. Philip Bogetto, Chair Sierra Club, Hawai'i Chapter, O'ahu Group P.O. Box 2577 Honolulu, HI 96803

Dear Mr. Bogetto:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments

We have received a copy of your letter to the Department of Land Utilization dated September 10, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

We appreciate your information regarding objectives and policies of HRS 205A-2 and 205 A-4. The owners are addressing these objectives and policies in the manner they have proposed to reconstruct their existing walls using a siructural design that maximizes the lateral access options, is an aesthetic improvement, and minimizes erosional forces due to the structure.

The four seawalls are aligned together and match the adjoining properties on either side. At this location, there are 16 properties in a row that are all protected by vertical seawalls. The beach along this entire section of the Mokuleia coast could be considered very narrow. Lateral passage along this shoreline in front of the seawalls will be improved by the proposed action, which will remove loose rock debris and deteriorated wall sections, and replace these with an engineered modified vertical seawall structure. With the lower sloped portion consisting of large rock material, there will be a return of a slightly wider beach for lateral passage. At the three lots to the west where modified vertical seawalls were recently built with the hybrid wall design, there has been a recent return of a narrow strip of beach sand along a section where there was no beach sand for lateral passage in the recent past.

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It was never intended for people to walk along the top of the 9-foot high walls, as this would be hazardous. People who like to walk this portion of the beach for fishing, gathering or other recreation will be able to continue this practice without interruption. The new walls will not diminish lateral access and are likely to improve it through the new hybrid wall design, as shown by the trend of the beach fronting the three lots to the west.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information regarding our request for Nationwide Permit coverage.

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Sincerely,

GROUP 70 INTERNATIONAL, INC.

Jeffrey H. Overton, AICP

Chief Environmental Planner

Group 70 International, Inc. • Architecture • Planning • Interior Design • Environmental Services • Building Diagnostics • Assets Management 925 Bethel Street, Fifth Floor • Honolulu, Hawaii 96813-4307 • Phone (808) 523-5866 • FAX (808) 523-5874 • http://www.group70int.com • mail@group70int.com

DECEI OCT 0 3 1997

Wednesday, October 1, 1997

warding 30

Roger and Jean Compton 312 Ilimalia Look Kailua, HI 96734

Dear Mr. and Mrs. Compton,

In yesterday's Honolulu Advertiser I saw that the Army Corps of Engineers are granting a permit to clear the Makaleha Stream. I also noticed that you have applied for a Seawall Reconstruction in The Environmental Notice of 8/23/1997.

I am concerned with shoreline erosion and have been for a number of years. We live on the beach front on Maui, and I should like to share with you the three approaches we took (that failed) as well as the approach that led to success in curbing beach erosion.

We here at Sugar Cove sandbagged a part of our shoreline in 1988 and again in 1989. The bags broke up. We built a tire revetment in 1990. It also fell apart. In 1993 we built a boulder revetment that started to disintegrate during the next winter's storms. By 1995 the boulders had fallen seriously in three areas. (There is 500 feet of our shoreline and 100 feet of our adjoining neighbor's.)

We started in the fall of 1995 with minor sand feeding that helped us through the following winter. In June of 1996 we instigated a large sand feeding operation. This immediately moved the water (wave action) offshore and returned the beach to us. We are committed to ongoing sand feeding.

I know you are saying, "But sand feeding is so expensive." We spent over \$600,000 on the failed approaches (over \$300,000 on the boulders alone), and we have spent only \$95,000 to date on the sand feeding. Granted we have sand sources here on Maui, but you may have some on Oahu that haven't been discovered yet. More of that later.

**Beach erosion** happens for many reasons. In our case the culprit was mining sand from our shoreline for a hundred years. The sugar industry used sand to make lime to process sugar, and sand was also used to make roads and filter water. But that was done before we came, and we were faced with continuing erosion.

## Beach erosion occurs when there is no longer shallow water far enough offshore to cause the waves to break away from the shoreline.

Let's put it another way. Waves break when they hit shallow water, whether on a reef, a breakwater, or best and more naturally they break on a gradually tapering beach. A wall to protect one's property is no different than a breakwater out in the ocean, and a sudden breaking of a wave or waves has a lot of force. Breakwaters are notorious for requiring maintenance because of the huge forces that impinge on them during storms when waves are big.

# So how do you protect your property <u>without</u> the heavy duty rocks you are proposing to install?

You put in sediment to move the shoreline farther from your door by creating shallow water farther offshore.

### How do you accomplish this?

By added fill. The fill can be anything the ocean can move around, and this can be cobbles, broken concrete (as from building construction debris), coral rubble (if it were available), gravel, broken rocks, or of course sand. But the sand can veneer the fill after the filling of the offshore is completed.

### And why does this work?

Because nature wants to hold back the sea, and it tries very hard to even when its beach or buffer zone is deprived of the movable sediment it needs to keep this natural system functioning properly.

# What is this natural system or buffer zone that holds back the sea?

It is comprised of three parts:

- (1) The offshore sediment that creates shallow water when necessary to move wave action away from the shore.
- (2) The swash zone where the waves run up and down or back and forth on the wet and dry sand.
- (3) The reservoir of sand or sand bank that forms a dune or dry sand on a healthy beach.

## Why are our beaches in Hawaii in trouble?

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The diminishing of Part (3) is most often the culprit. This reservoir is deprived of sand that needs to be in the bank for times of big surf, most often by people who want to protect their property. They build walls or fortification of some sort. These walls then cut off part of the reservoir, so that their neighbor's sand is called upon to supply the beach's natural system with what it needs. Nature doesn't know whose beach front is whose. It uses sand that is available.

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The first person to recognize danger builds a wall without understanding that the whole beach needs sand. He saves his lawn (that was built on sand in the first place), but the other neighbors' yards will now be called upon to replenish the supply that is cut off when storm waves come along.

When storm waves come along, the beach knows that it needs to move sediment offshore so that the larger waves will break farther away from the land and run up the swash slowly rather than hitting the shore with great force.

# So what can you do about this?

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Gather your neighbors that are complaining about **your wall** and explain that they were in a large part responsible for the more recently deteriorated beach. Until now **you** were contributing **your sand** to maintain the natural beach system, but finally there is no money (sand) left in your bank account. Now it is time for everyone along your beach to contribute to the restoration of the protective beach system that nature intended for all these properties.

## How can this be accomplished?

- (1) By everyone in your neighborhood cooperating.
- (2) By seeking sources of sediment to apply to the beach system.
- (3) By hauling or dredging or shoveling sediment that the ocean can (and will) move into the cavities that have developed offshore.

# Why is this of imminent importance?

Because until the offshore slope on your shoreline is made gradual, the beach will continue to erode. Regardless of the size and strength of the wall you put up, you will be faced with continual maintenance of it until the beach is restored with sand or movable sediment.

My parting shot is this, and it is from the age old wisdom of Jeremiah

5:21,22 "Hear now this, O foolish people, and without understanding; which have eyes, and see not; which have ears, and hear not: Fear ye not me? saith the Lord: will ye not tremble at my presence, which have placed the sand for the bound of the sea by a perpetual decree, that it cannot pass it: and though the waves thereof toss themselves, yet can they not prevail; though they roar, yet can they not pass over it?"

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This means that the sand binds the sea providing the protection needed. Man needs to understand that this binding cannot be broken and hold the sea in its place. The binding is the buffer zone of the beach system. The dune and the offshore are an integral part of this buffer zone, and to put up a wall without providing sediment in front of it is asking for trouble.

Call on your neighbors. Let them know that it is sediment in front of their walls that is needed. The whole area needs to cooperate with a greater sense of community.

I invite you to call or write to me. I am a private homeowner who is also interested in saving beach front property as well as saving the beach so everyone can enjoy it.

Yours sincerely,

Subara Juild

Barbara Guild 320 Paani Place 1A Paia, Maui, HI 96779 808-877-3109 808-877-3524 fax

CC:

Jeffrey Overton (523-5899 x 111) Group 70 International, Inc. 925 Bethel Street, 5th Floor Honolulu, HI 96813

Steve Tagawa (523-4817) 527-6743fax City & County of Honolulu Deptartment of Land Utilization 650 South King Street, 7th Floor Honolulu, HI 96813



5 December 1997

- Francis S. Oda, AIA, AICP
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Paul P. Chorney, AlA – Dean H. Kitamura, AlA Norma J. Scott, AlA

- <sup>\*</sup> Stephen E. Callo, CPA George I. Atta, AICP
- leffrey H. Overton, AICP
- ---Kathryn A. Nam Roy A. Inouye
- Mary J. O'Leary

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Ms. Barbara Guild 320 Pa'ani Place 1A Pa'ia, Maui, HI 96779

Dear Ms. Guild:

Subject: Shore Setback Variances for Reconstruction of Seawalls Zane, Frost, Compton & Masunaga Properties, Mokuleia, Oahu TMK (1) 6-8-10:23, 24, 25 and 26 Responses to Comments on Draft Environmental Assessments

We have received a copy of your letter to the Department of Land Utilization dated October 1, 1997. The following letter responds to the comments provided on the Draft Environmental Assessments for the subject properties.

We truly appreciate the comments you have provided, as it provides a testament to the struggle that many other fellow shoreline property owners face in protecting their investment and human safety. You have obviously become deeply familiar with the dynamics of your coastline, and have accumulated a knowledge base of erosion problems on Maui. Your suggestion to discuss the options with our neighbors and seek a common solution is a very good approach. Mr. Compton has been spearheading an effort to get his neighbors together over the past two years to seek a solution.

At the four properties in Mokuleia, the owners are faced with a similar problem of a retreating shoreline and failing structures. Shoreline retreat along this stretch of coast has been at the rate of one foot per year for the time period 1949-1996, including the 20 years prior to the first wall built in this area. With shallow lots, the owners have little choice but to protect their homes with a shoreline structure.

The four owners of the subject properties are reconstructing their CMU seawalls with a modified vertical seawall design. This design includes a sloping lower section of boulders on a footing six feet below sea level. The strong foundation and grouted rock of these walls will not allow for boulders to slip away. The

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energy of waves striking the structure is reduced by the sloped lower section and spaces between the boulders.

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Sand replenishment and other soft structure options have been considered by the owners. The cost to pump or truck sand to this beach is prohibitive. There would be long time delays due to an uncertain federal, state and county permitting process and the possibility of community controversy. The North Shore of O'ahu has a famous high ocean energy regime that directly affects these properties, and it is unlikely that placed sand would remain without additional structural containment such as a groin field. Offshore transport of placed sand from the beach could also cause smothering of nearshore benthic habitats.

Offshore fill material placement could reduce the energy striking this coast, and a breakwater structure could provide a reduction in wave energy at the four properties. However, placement of offshore fill or construction of a breakwater would be a costly and controversial use of the ocean and public underwater lands. The owners do not have years to save their property – they could lose everything this winter. They need to reconstruct next summer with a well designed structure and continue to discuss the long-term options with their neighbors and government.

Thank you from providing your comments on the Draft EA. Please contact me if you have questions or require additional information.

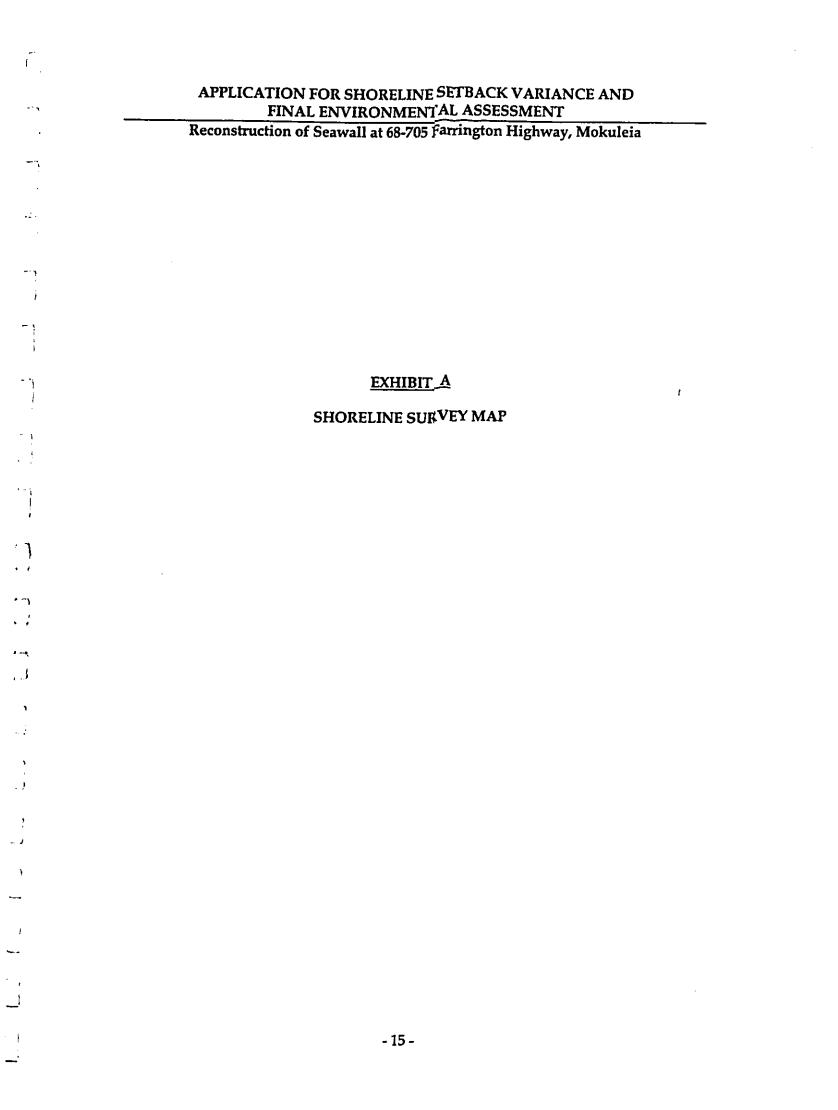
Sincerely,

GROUP 70 INTERNATIONAL, INC.

Jeffrey H. Overton, AICP Chief Environmental Planner

P:\Planning\97011-11 Compton SSV\EA RESPONSES\lbg001jo\_mokaw11207\_GUILDresp.rtf

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	STATE OF HAWAII DEPARTMENT OF LAND AND NATURAL RE LAND DIVISION P.O. BOX 621 HONOLULU, HAWAII 96809	STATE PARKS WATER RESOURCE MANAGEMENT
Ref.:LD-PEM Mr. Kendall Hee Engineers Surveyor: 1020 Auahi Street, 1 Honolulu, Hawaii 9	Bldg 6, Suite 1	DEBEINED JUL 21 1997.
Dear Mr. Hee: Subject:	Shoreline Certification Request Applicant: <u>Engineers Surveyors Hawaii</u> , In Property Owner: <u>Roger Compton</u> , Pat Bair Location - Island: <u>Oahu</u> District: <u>Waialu</u> Tax Map Key: 6-8-10:23-26	nes, Jack Frost, & Harold Masunaga

ist.

This is to inform you that the subject shoreline certification request has been certified and no appeal has been received. <u>Ten (10)</u> certified copies of the map are enclosed herewith.

Beach Homes, Sec 1" at Mokuleia, Waialua, Oahu

Land Management Case No.: OA-629

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Should you have any questions regarding this matter, please feel free to contact Patti Miyashiro of our Honolulu Office at 587-0430.

Very truly yours,

Property Description: Lots 11 thru 14, Ld Ct App 1810, (Map 4), "Mokuleia

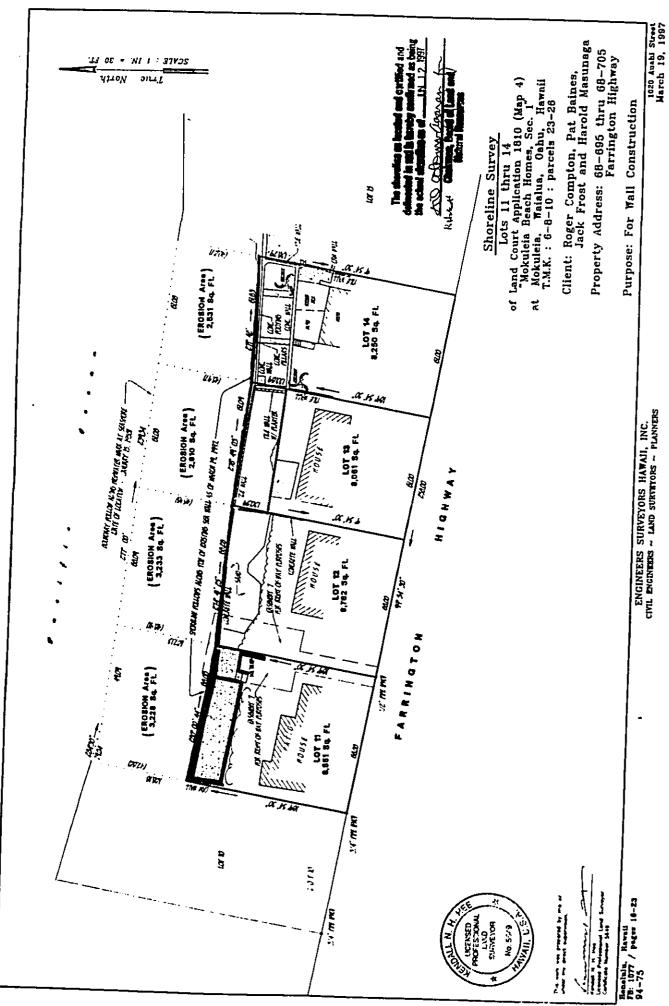
DEAN Y. UCHIDA Administrator

Enclosures

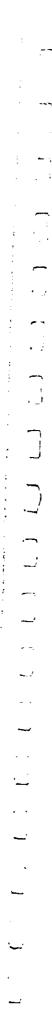
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Oahu Land Board Member At-Large Land Board Member Oahu District Land Office Survey Div., DAGS (w/enclosures)

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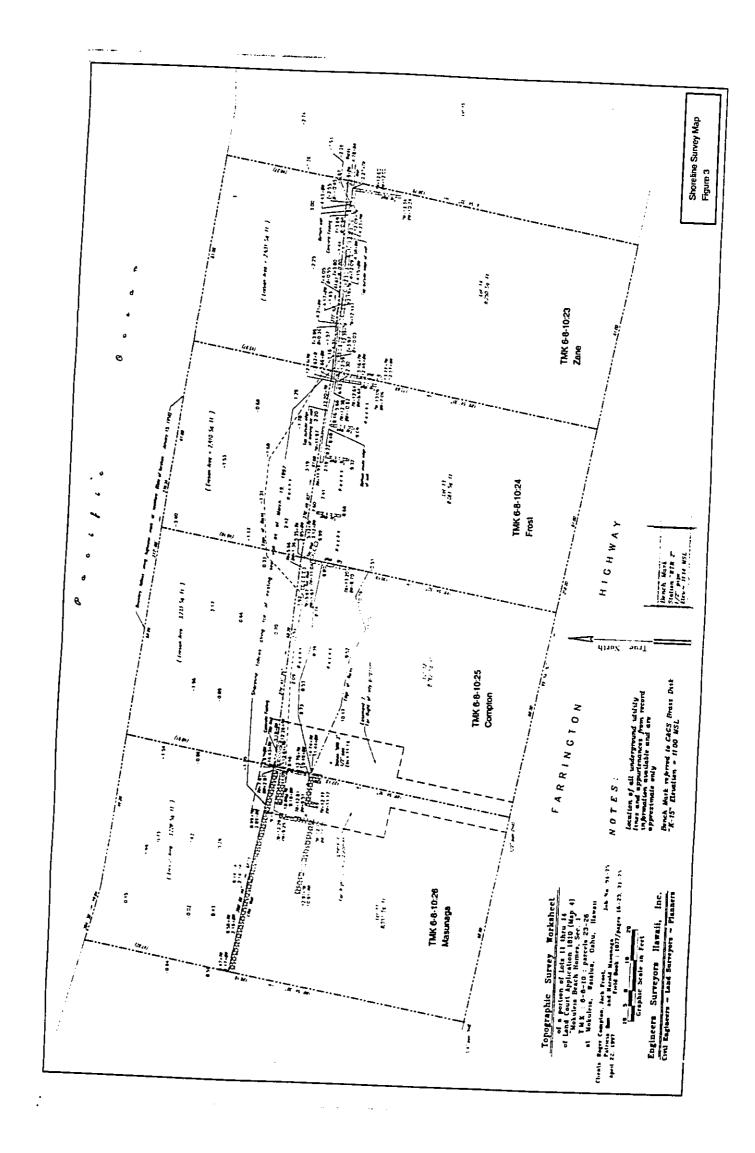


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DOCUMENT CAPTURED AS RECEIVED

## APPLICATION FOR SHORELINE SETBACK VARIANCE AND FINAL ENVIRONMENTAL ASSESSMENT Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

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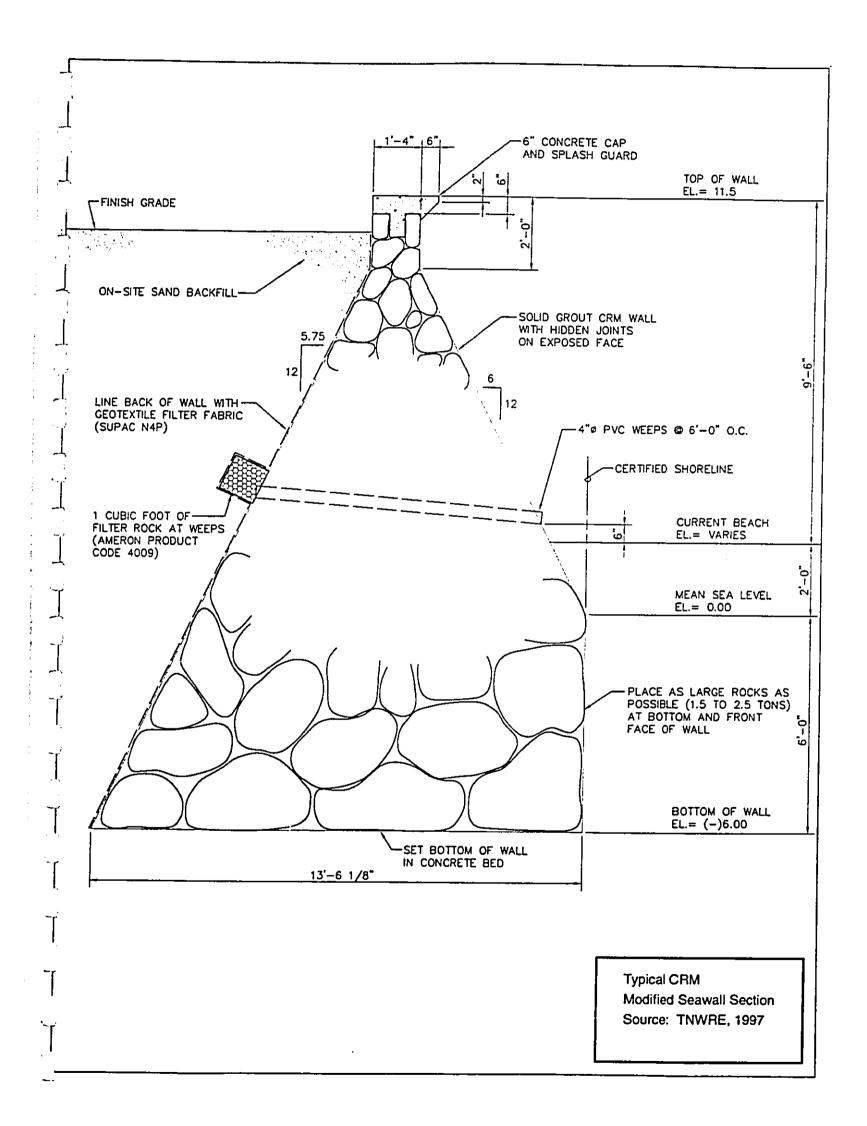
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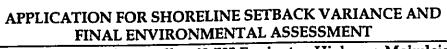
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### EXHIBIT B

### PRELIMINARY CONSTRUCTION DRAWINGS





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Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

### EXHIBIT C

### OCEANOGRAPHIC AND STRUCTURAL EVALUATION

### EVALUATION OF THE EXISTING SHORELINE AND SEAWALL STRUCTURE AT TMK 6-8-10:25 (Lot 10) Mokuleia, Oahu, Hawaii

### Proposed Seawall Reconstruction For TMKs 6-8-10:23 Through 26 at Mokuleia, Oahu, Hawaii

### Prepared for

Group 70 International, Inc. 925 Bethel Street Honolulu, Hawaii 96813-4398

Prepared by

Tom Nance Water Resource Engineering 680 Ala Moana Boulevard - Suite 406 Honolulu, Hawaii 96813

July 1997

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Shoreline Setting	1
Present Condition of the Shoreline Structures Fronting TMKs 6-8-10:23 Through 26	4
Shoreline Changes, 1949 to 1996	5
Proposed Seawall Construction	9

### Appendix

Α	Photographs of Existing Shoreline Conditions
в	Foundation Investigation and Recommendations By Geolabs-Hawaii
С	Retaining Wall Calculations By Structural Analysis Group

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1	Location of the Project Site in Mokuleia	2
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### Introduction

This report has been prepared to provide technical and environmental information to support a Shoreline Setback Variance application for the construction of seawalls across the shoreline frontage of four contiguous house lots in Mokuleia, Oahu. Moving from east to west, the four lots are identified as follows:

тмк	Address	Area of the Lot (Ft <sup>2</sup> )*	Length of Shoreline Frontage (Feet)
6-8-10:23	68-695 Farrington Highway	8250	61.63
6-8-10:24	68-697 Farrington Highway	8061	61.09
6-8-10:25	68-701 Farrington Highway	8762	68.28
6-8-10:26	68-705 Farrington Highway	8551	68.00

 These lot areas extend substantially beyond the present shoreline. Actual usable lot areas on the landward side of the present shoreline are 2631 to 3233 square feet less than this.

Information on which this report is based includes: discussions with the four lot owners; a topographic survey dated March 19, 1997 by Engineers Surveyors Hawaii, Inc.; a series of seven aerial photos for the 47 years from 1949 to 1996; borings done by Geolabs Hawaii to probe the depth to basement rock; and field investigation over a period of several months.

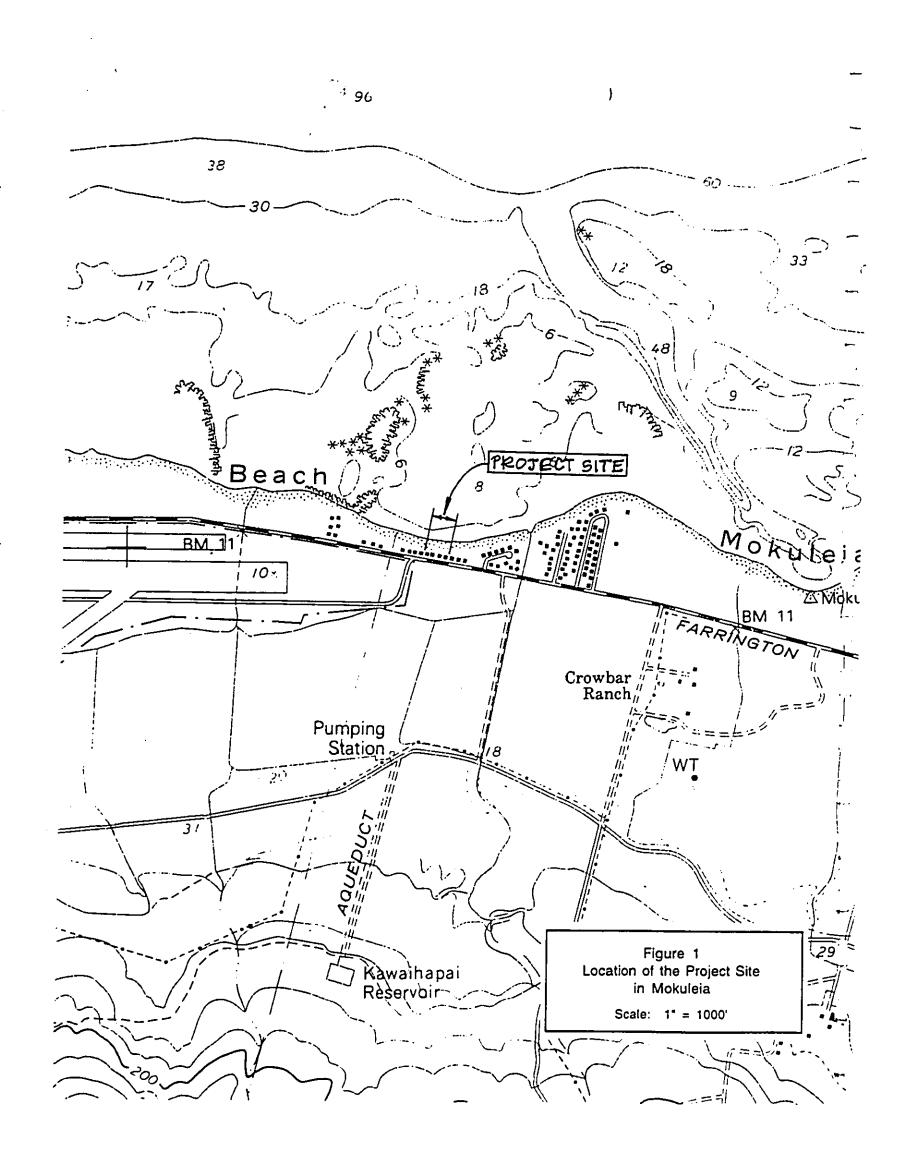
### Project Location

The four lots which are the subject of this report are located along Farrington Highway, just east of the entrance to Dillingham Airfield in Mokuleia. Their location is shown on the portion of the USGS Kaena quadrangle map reproduced as Figure 1 and the tax map reproduced as Figure 2.

### Shoreline Setting

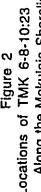
The four lots are situated near the center of an unnamed shoreline embayment which is defined by rock outcrops  $d^n$  the west side (directly offshore of Camp Mokuleia) and a sandy headland on the east side. The sandy headland appears to have been formed by the wave protection of rock outcrops offshore and the generally shallower bathymetry directly offshore. The embayment, which faces directly to the north, is about 2500 feet across. The indentation of the shoreline is a maximum of 500

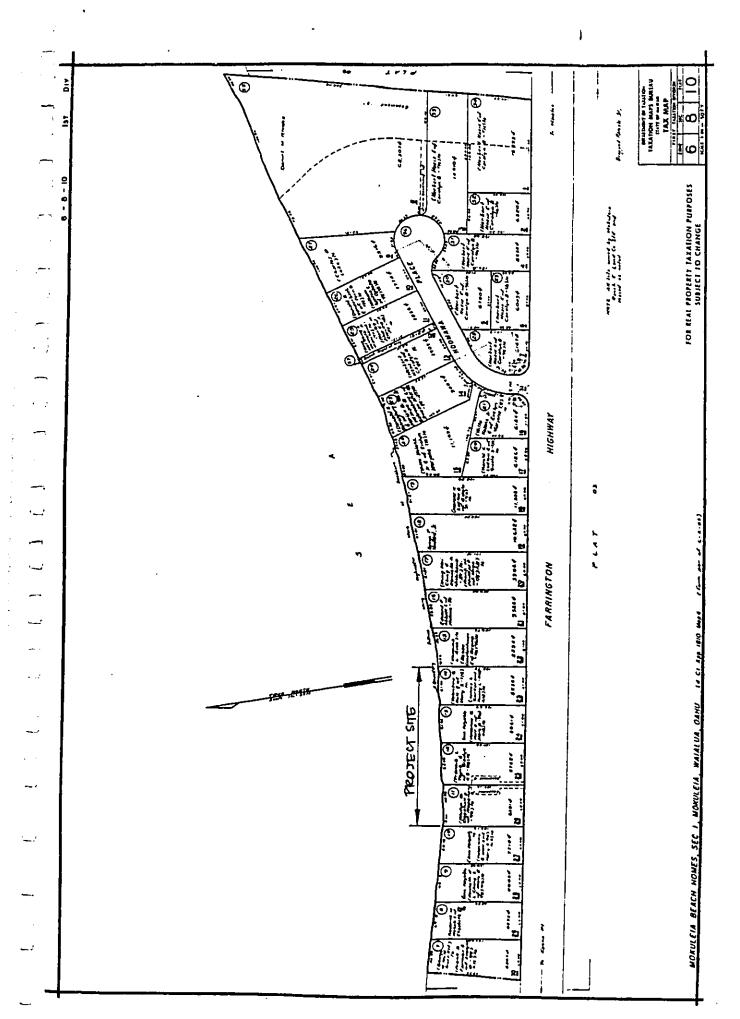
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feet at its center. Nearshore bathymetry is generally flat, although there are a number of coral encrusted boulders, some of which protrude above water at low tide. The bottom is comprised of dead corals, coralline algae, and shallow pockets of sand in depressions, all indicative of substantial wave energy. Water depths are generally six to eight feet for distances of 1500 to 2000 feet offshore. At that point, a series of ledges create a relatively steep drop-off to depths of more than 100 feet within 4000 feet of the shoreline. It might also be noted that there is a significant submarine channel located just to the east of the embayment and nearshore bathymetry described above (the channel is delineated by the bathymetric contours on Figure 1).

The north facing shoreline is directly exposed to waves from the northwest to the northeast, meaning that the wave energy reaching the shoreline is much greater during the winter months when waves from these directions are most frequent. Depths of the nearshore shelf control the breaking of waves and the amount of energy which reaches the beach. At low tide, the wave energy at the shoreline is far less than at high tide simply due to the different water depths at the tidal extremes.

Despite the fact that only moderate-sized waves can translate across the nearshore shelf and break on the beach, all of the lot owners along this embayment from Camp Mokuleia on the west end (TMK 6-8-03:8) to the beach access easement at Hoomana Place on the east end (TMK 6-8-10:13) have had to resort to seawall construction to stop the progressive loss of beach frontage and to prevent damage to structures behind the beach. Seawalls for the three lots immediately to the west of the four which are the subject of this report (TMKs 6-8-10:27, 28, & 29) were recently reconstructed. That construction has left the four lots in question with the most imminent need of shoreline protection.

### Present Condition of the Shoreline Structures Fronting TMKs 6-8-10:23 Through 26

The photographs in Appendix A, which were taken at low tide on the morning of April 18, 1997, depict the present condition of the seawalls at the four lots. Each is described in the paragraphs following.

TMK 6-8-10:23 (Refer to Photo Nos. 1. 2. 3. and 4). The hollow tile wall along the shoreline frontage of this lot has a prominent vertical crack located about midway across the property and another vertical crack at its western boundary with TMK 6-8-10:24. It appears that the wall's shallow footing is the cause of the crack at mid-property. The catastrophic failure of the wall at TMK 6-8-10:24 appears to have caused the crack at the west end. Wave action at these locations has pulled material from behind the wall seaward, creating sink holes that the Owner has had to repeatedly fill.

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This undermining on the landward side of the wall is shown in Photo Nos. 3 and 4. Although the Owner has backfilled with gravel and grout, these stop-gap measures are not long-term remedies for the wall's inadequate footing.

TMK 6-8-10:24 (Refer to Photo Nos. 5, 6, 7, and 8). As shown on Photo No. 5, only a 27-foot long section of the hollow tile seawall on the east end of this lot's shoreline frontage remains in place. However, it is tilting precariously seaward and is in danger of collapsing on the beach. The remaining 36 feet of the lot's frontage is strewn with boulders and pieces of concrete, some of which have been placed as an emergency shoreline protection measure (Photo No. 6). The bottom of the wall's foundation ends several feet above mean sea level, allowing it to be completely undermined as the beach in front of the wall eroded (Photo No. 7). The undermining has resulted in a substantial loss of material on the landward side of the wall (Photo No. 8). As is typical for all the older walls along this beach, an inadequate foundation appears to have been the cause of the wall's failure.

<u>TMK 6-8-10:25 (Refer to Photo Nos. 9, 10, 11, and 12)</u>. Except for 5- and 8-foot long sections at the east and west ends of this property, the hollow tile seawall has completely collapsed. The balance of the shoreline frontage is now comprised of basalt boulders which were placed this past winter as an emergency measure to protect the house from wave damage. As with the other walls, the foundation was placed above sea level. Undermining by wave action caused the wall's failure.

TMK 6-8-10:26 (Refer to Photo Nos. 13, 14, 15, and 16). Unlike the hollow tile walls of the other three lots, this seawall is made of grouted rock and boulders (Photo Nos. 13 and 14). However, since its foundation stops above sea level, it has also experienced significant undermining. There has also been a collapse of a rock stairs structure at the east end (left side of Photo No. 13). To stop the proliferation of sink holes caused by waves washing underneath the foundation, a substantial volume of concrete was installed on the landward side of the wall (Photo No. 15). Since the seawall is relatively low and subject to relatively frequent overtopping, a second, interior wall was constructed (also visible in Photo No. 15). It is set back 14 feet from the first wall and is about three and a half feet higher.

### Shoreline Changes, 1949 to 1996

Seven vertical aerial photographs were used to delineate shoreline changes over the 47-year period from 1949 to 1996. The dates of the seven photographs, all at approximately 1-inch equals 100-foot scale, are as follows: May 7, 1949; July 24, 1961; April 22, 1967; December 5, 1969; 1975; November 24, 1983; and 1996. The 1975 and 1996 photos were obtained from Air Survey

- 5 -

Hawaii and do not show the month and day they were taken. The other five aerial photos are from RM Towill Corporation. Only the 1996 photo is in color; all the others are black and white.

House and Seawall Construction. Using these seven aerial photos, Table 1 identifies the time periods for the construction of houses and seawalls. On all four lots, installation of the seawalls followed house construction by several to a number of years. The earliest seawall was installed some time between July 1961 and April 1967 at TMK 6-8-10:23, although it appears to have been realigned to conform to the crescent shape of the beach prior to the December 1969 photo. This seawall was among the first along the entire section of the shoreline from Camp Mokuleia to Hoomana Place, a distance of 3,000 feet.

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The last of the four seawalls was done in the 1975 to 1983 time period at TMK 6-8-10:25. In the 1975 aerial photo, it is the last remaining residential lot among this coastal segment without a seawall (Camp Mokuleia, which was constructed several years later in 1980-81, also did not have a seawall at this time). By 1983, all lots in this coastal segment, including Camp Mokuleia, had seawall protection.

Shoreline Retreat Based on Movement of the Vegetation Line. Distances of the vegetation line from the painted centerline of Farrington Highway for each of the seven aerial photographs are listed on Table 2. The rather modest changes of the vegetation line may be a little surprising, given the threat posed by waves that gave rise to the construction of seawalls along this entire coastal segment. However, most of the shoreline change has been in the continuously diminishing width of the beach rather than in the movement of the vegetation line.

Diminishing Width of Beach Sand. Table 3 documents the diminishing width of beach sand over the 1949 to 1996 period. Because it is impossible to determine the location of the mean sea level shoreline in the photographs, the width of beach sand has been taken as the distance from the vegetation line (or seawall) to the seaward extent of sand visible in the nearshore waters. Widths defined in this manner declined from May 1949 to April 1967. However, the widths recovered almost completely in the short period from April 1967 to December 1969. Following this, the width of sand has gotten progressively narrower, particularly in the most recent years. Beach sand widths are now 18 to 25 feet, a third or less of the widths that prevailed in 1949 and again in December 1969. Most of the loss of beach width occurred from 1983 to 1996. This explains the recent failure of number of seawalls, including the four which are the subject of this report. In all cases, the walls were built with their foundations above sea level. At the time of their construction, there was a substantial amount of beach

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## Table 1

# Chronology of House and Seawall Construction Based on Aerial Photographs

| Time Interval                          | TMK 6-8-10:23<br>(Bain/Zane)                         | TMK 6-8-10:24<br>(Frost)        | TMK 6-8-10:25<br>(Compton) | TMK 6-8-10:26<br>(Masuroco)              |   |
|----------------------------------------|------------------------------------------------------|---------------------------------|----------------------------|------------------------------------------|---|
| Up to May 1949<br>May 1949 - July 1961 | Vacant Lot<br>House Constructed                      | Vacant Lot<br>House Constructed | Vacant Lot                 | Vacant Lot                               | · |
| July 1961 - April 1967                 | Seawall Constructed                                  |                                 |                            | House Contraction                        |   |
| April 1967 - December 1969             | Seawall Reconstructed<br>With Different<br>Alignment | Seawall Constructed             |                            | Seawall Constructed                      |   |
| December 1969 - 1975                   |                                                      |                                 | House Constructed          | Seawall Reconstructed<br>Further Seaward |   |
| 1975 - November 1983                   |                                                      |                                 | Seawall Constructed        |                                          |   |
| November 1983 - 1996                   |                                                      | Seawall Collapsed               | Seawall Collemond          |                                          |   |
|                                        |                                                      |                                 | ocamali cullapsed          |                                          |   |

- 7 -

A rallroad track on the makai side of Farrington Highway crossing all four lots is visible in the 1949 photograph. -Notes:

Reconstruction of the seawall at TMK 6-8-10:23 in the 1967 to 1969 period realigned the wall parallel to the beach present. ~

The seawall reconstructed at TMK 6-8-10:26 in the 1969 to 1975 period was moved 7 to 10 feet seaward. The pre-existing seawall appears to be where the interior wall is today. ы.

Collapse of the seawalls at TMKs 6-8-10:24 and 25 occurred in the 1995-1996 period. 4.

### Table 2

| Date of<br>Aerial | Distance From Center Line of Farrington Highway to Vegetation Line (Feet) |                  |                 |                 |  |  |
|-------------------|---------------------------------------------------------------------------|------------------|-----------------|-----------------|--|--|
| Photograph        | TMK 6-8-10:23                                                             | ТМК 6-8-10:24    | ТМК 6-8-10:25   | ТМК 6-8-10:26   |  |  |
| May 1949          | 106                                                                       | 100              | 87              | 85              |  |  |
| July 1961         | 100                                                                       | 95               | 90              | 75              |  |  |
| April 1967        | 105 (to 1st Seawali)                                                      | 92               | 90              | 90              |  |  |
| December 1969     | 102 (to 2nd Seawall)                                                      | 100 (to Seawall) | 90              | 93 (to Seawall) |  |  |
| 1975              | 102 (to 2nd Seawall)                                                      | 100 (to Seawall) | 83              | 93 (to Seawall) |  |  |
| November 1983     | 102 (to 2nd Seawall)                                                      | 100 (to Seawali) | 95 (to Seawall) | 93 (to Seawall) |  |  |
| 1996              | 102 (to 2nd Seawall)                                                      | 100 (to Seawall) | 95 (to Seawall) | 93 (to Seawall) |  |  |

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Movement of the Vegetation Line Based on Aerial Photographs

### Table 3

### Approximate Widths of Beach Sand (in Feet) Based on Aerial Photographs

| Date of<br>the Aerial<br>Photograph | ТМК 6-8-10:23 | ТМК 6-8-10:24 | ТМК 6-8-10:25 | ТМК 6-8-10:26 |
|-------------------------------------|---------------|---------------|---------------|---------------|
| May 1949                            | 74            | 72            | 73            | 75            |
| July 1961                           | 55            | 60            | 55            | 65            |
| April 1967                          | 55            | 62            | 65            | 62            |
| December 1969                       | 72            | 75            | 70            | 68            |
| 1975                                | 58            | 58            | 70            | 60            |
| November 1983                       | 52            | 45            | 51            | 47            |
| 1996                                | 24            | 18            | 25            | 24            |

Note:

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Width of sand is the distance from the vegetation line (or seawall) to the end of the sand deposit in nearshore waters.

sand between the walls' footings and the shoreline. Now that much of the sand has been eroded, waves continually wash beneath the footings, creating sinkholes on the landward side of the walls.

### **Proposed Seawall Construction**

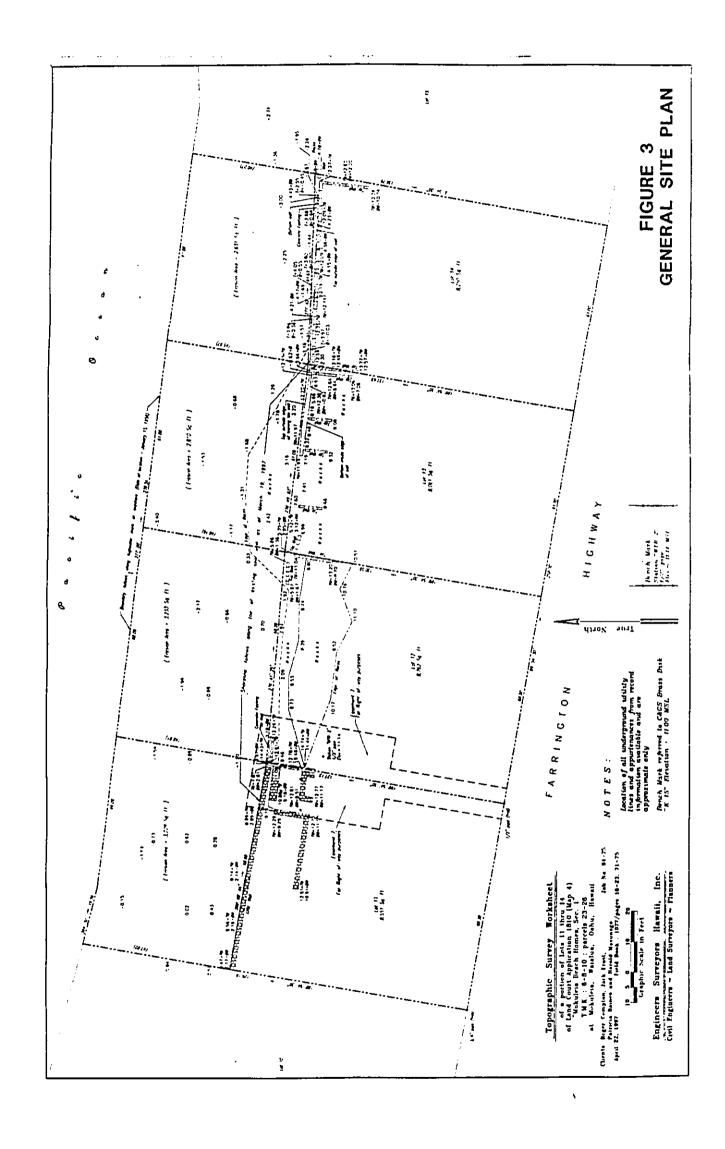
A decision has been made to completely rebuild all four seawalls with appropriately designed foundations to avoid the undermining that has been the downfall of the present walls. Design of the walls will be based on the depth-limited, maximum wave height that can break on the structure and to retain the bearing load on the landward side of the structure.

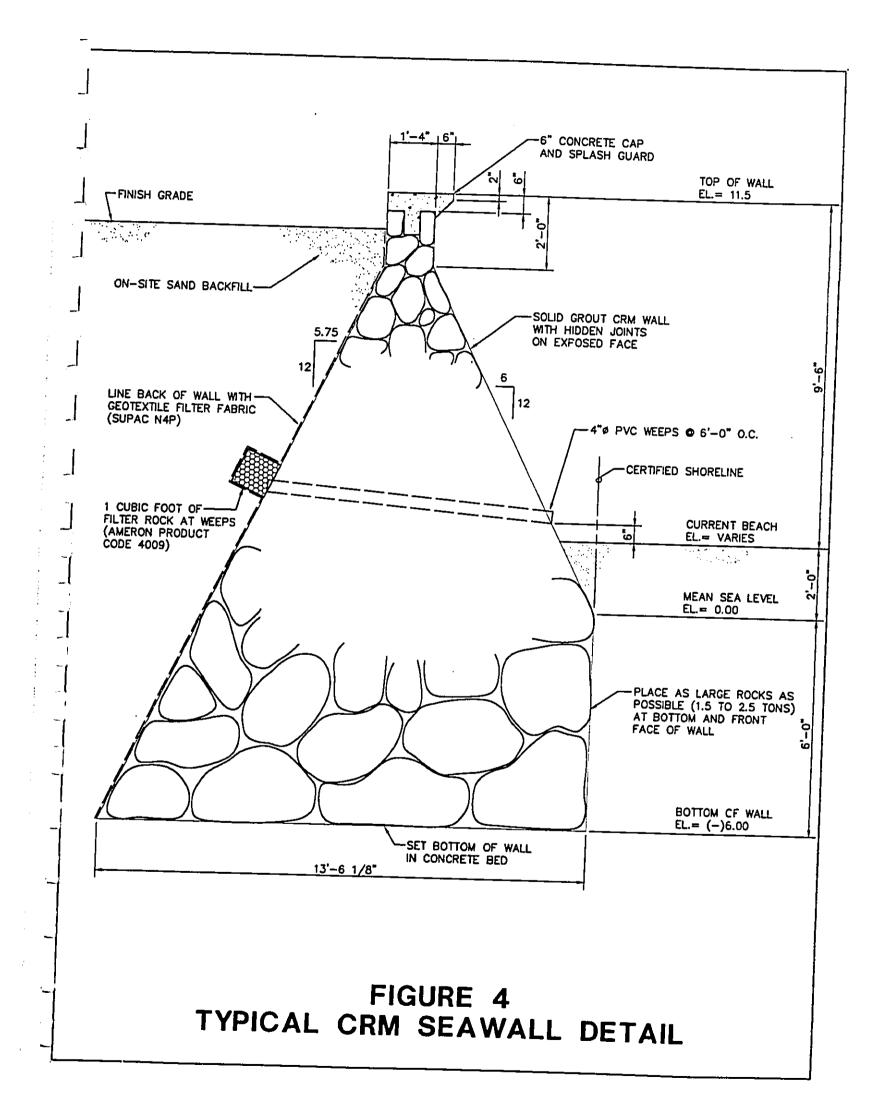
Seawall Alignment. The seaward side of the seawall foundation will be placed at the certified shoreline as determined by a survey by Engineers Surveyors Hawaii. This location is indicated on Figure 3. This is also the same alignment of the pre-existing seawalls and it generally conforms to the arcuate shape of the beach.

Foundation Conditions. Due to constraints of access, detailed foundation investigation was limited to a single boring on TMK 6-8-10:25. Since the results of this boring were similar to two boreholes done previously for the reconstruction of the seawall at TMK 6-8-10:29, they seem generally representative of conditions along this shoreline segment. The foundation investigation was conducted by Geolabs-Hawaii and its report can be found in Appendix B. Ground elevation at the borehole site was 8± feet (msl) and the boring reached a depth of 31 feet (23 feet below MSL) without encountering basement rock. Coralline sand extended to a depth of 23 feet. This was followed by six feet of very stiff slity clay and then coral sand for the last two feet of drilling. The depth of the basement rock makes it necessary to construct the wall's foundation in the coral sand. This will be done at a depth of six feet below mean sea level. Design recommendations for bearing pressures, lateral earth loading, and coefficient of friction are contained in the Geolabs-Hawaii report.

Wall Section. The proposed cross section of the seawall, which is shown on Figure 4, will be identical across all four lots. It is generally similar to the recently completed seawalls on TMK 6-8-10:27, 28, and 29 (Photo No. 17). The face of the wall is a compromise between reducing wave reflection (with the slope on the lower half of the wall) and keeping some yard space between the wall and houses (the vertical upper half of the section). Void spaces in the seaward side of the seawall will not be grouted in order to provide energy dissipation and minimize wave runup. It should also be noted that the proposed wall will replace entirely vertical (fully reflecting) seawalls on all four lots. The analysis of the wall as a retaining structure was done by Structural Analysis Group (SAG) using the design recommendations of Geolabs-Hawaii. SAG's analysis is contained in Appendix C.

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Analysis of the Seawall to Withstand Breaking Waves. The stability of the proposed wall to resist breaking waves is based on the following assumptions:

- 1. The design wave is the highest, depth-limited wave which can break by plunging on the shoreline structure.
- 2. Sand at the foot of the seawall is eroded to a depth of -2 feet (msl). At present, sand heights vary between +2.8 feet (msl) at TMK 6-8-10:26 to -0.45 feet (msl) at TMK 6-8-10:23.
- 3. The nearshore bathymetric slope is 20 (horizontal) to 1 (vertical) based on the ESH survey (Figure 3).
- 4. The maximum breaking wave occurs at high tide (+1 feet msl) with an additional wave setup of one foot.
- 5. Wave periods will range from 8 to 15 seconds.
- 6. The wall is considered to be a stand-alone rubble mound structure with no credit for grouting of the boulders or the lateral soil support behind it.

Using the Corps of Engineers Shore Protection Manual (Volume II) and these nearshore conditions, the maximum height of a wave plunging on the seawall is 5.5 to 6.6 feet for wave periods of 8 to 15 seconds, respectively. Applying the Hudson formula to determine the weight of boulder required (without the additional strength of cement grout), armor stones would need to be 4100 pounds or about 2 tons (refer to equation and values below). The primary stones in the lower section of the wall will be 0.75W to 1.25W (3075 to 4920 pounds) in size. For basalt rock, this will be stones of 3 to 4 feet in size. It is important to note that the use of cement grout (except on the seaward face) will add considerably to structural stability.

$$W = \frac{WrH^{3}}{K_{p}(Sr^{-1})^{3} \cot \emptyset} = \frac{(167)(6.6)^{3}}{5.8(2.593 - 1)^{3}(0.5)} = 4095 \text{ lbs.}$$

Wr = unit weight of stone (167 lbs/ft<sup>3</sup> for basalt rock)

- H = design wave height in feet
- $K_D$  = stability coefficient (5.8 for structure trunk and stone placement with the long axis placed perpendicular to the structure face)

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- Sr = specific gravity of the armor unit relative to seawater (167 lbs/ft<sup>3</sup> + 64.0 lbs/ft<sup>3</sup> = 2.593)
- Ø = angle of structural slope from the horizontal in degrees (60°)

- 12 -

<u>Wave Runup and Overtopping</u>. Computations using the Shore Protection Manual indicate that the top of the seawall would have to be placed impractically high to avoid overtopping by storm waves. As a matter of practical application, this was demonstrated quite clearly during the past winter when all three of the recently completed walls (TMKs 6-8-10:27, 28, and 29) were overtopped. To hopefully limit the frequency and amount of overtopping, the concrete cap shown on Figure 4 will be installed.

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### Appendix A

Photographs of Existing Shoreline Conditions

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The seawall fronting TMK 6-8-10:23 has a large vertical crack at mid-length and another at its joint with the wall for TMK 6-8-10:24. Undermining is most obvious at the west end (right-hand side of Photo No. 2).

<u>(</u>

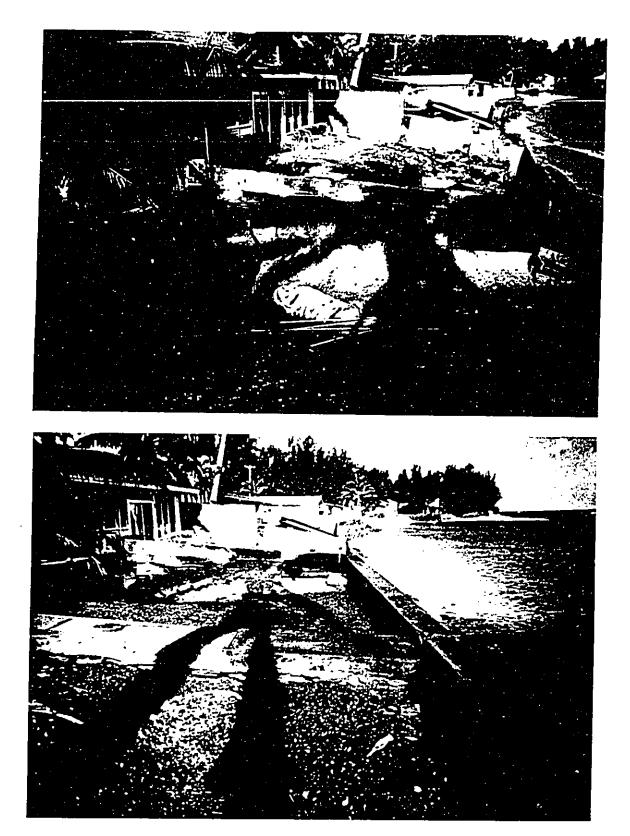


Photo Nos. 3 & 4. These photos illustrate the stop gap measures to stabilize the wall at TMK 6-8-10:23 and counteract the undermining occurring mostly at the west end.



measure (bottom photo). The hollow tile wall remaining on the east end is leaning outward and ready to fall on the beach (top photo).

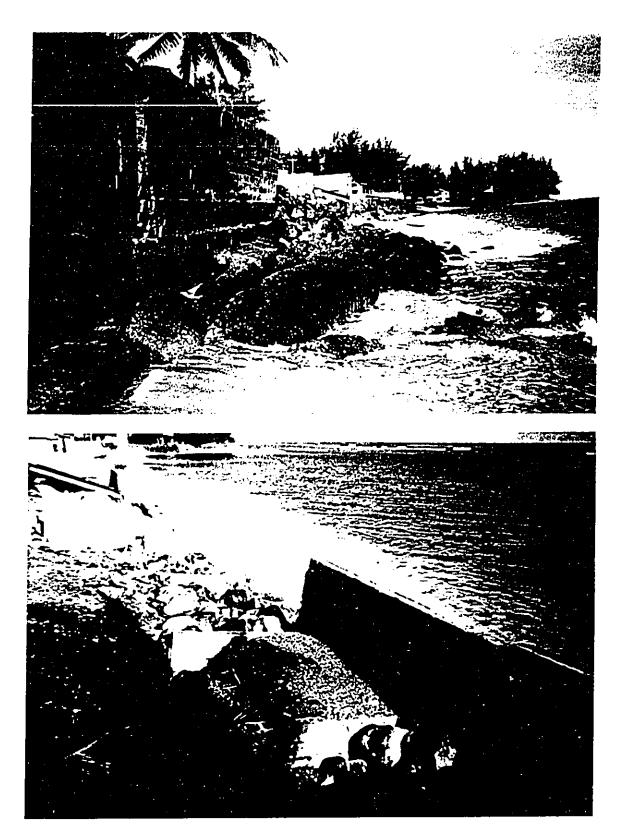


Photo Nos. 7 & 8. The top photo shows the undermining of the footing at TMK 6-8-10:24 and the outward tilt of the remaining portion of the seawall. The bottom photo shows some of the scour that has occurred behind the wall.





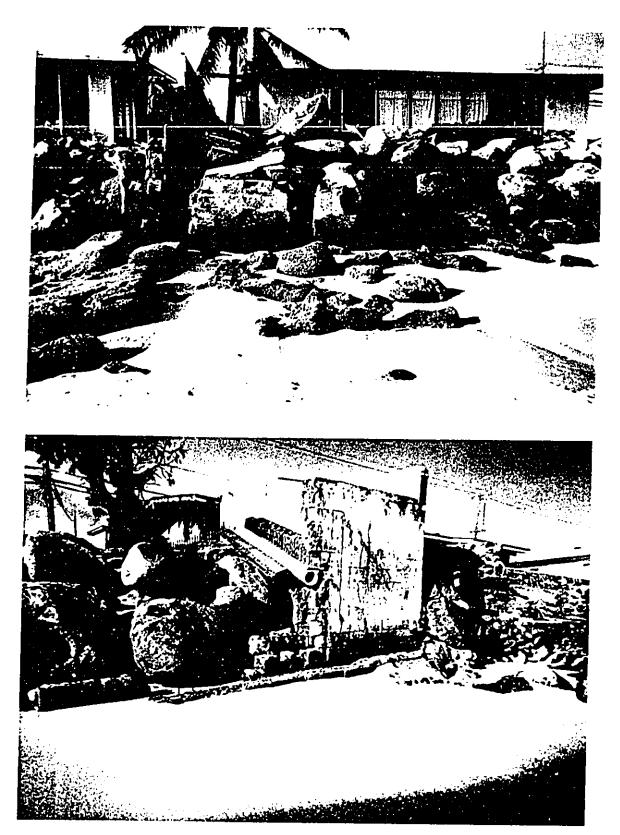


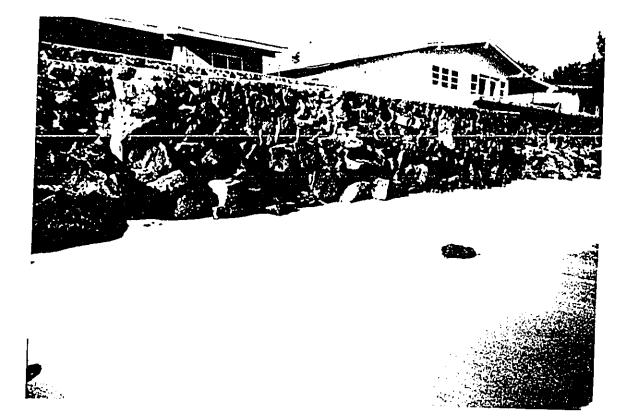
Photo Nos. 11 & 12. These details of the remnants of the seawall at TMK 6-8-10:25 show the top of the foundation to be well above sea level.

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The rubble masonry seawall at TMK 6-8-10:26 has been undermined along its entire length. Portions of the stair structure (top left) has collapsed on the beach.

## Photo Nos. 13 & 14.

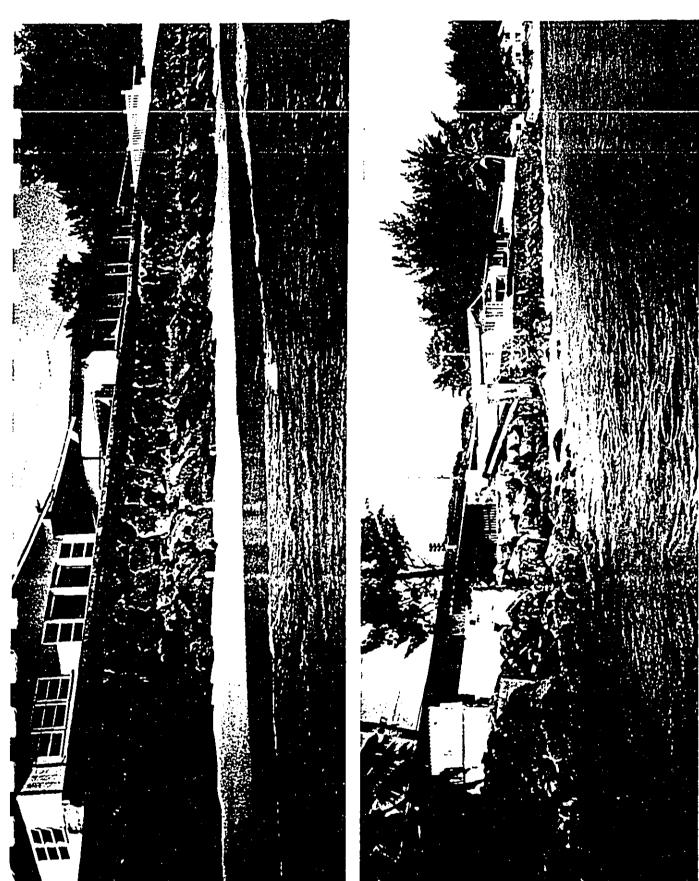


### Photo Nos. 15 & 16.

The top photo shows the extent of undermining of the rubble-rock seawall at TMK 6-8-10:26. Concrete has been poured on the landward side to avoid undermining there (photo at right). A second wall set 14 feet behind the seawall has been constructed to contain overtopping waves.





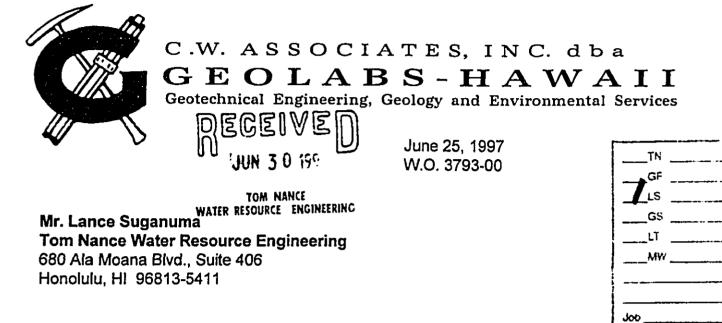




### Appendix B

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Foundation Investigation and Recommendations By Geolabs-Hawaii



Dear Mr. Suganuma:

Preliminary Geotechnical Recommendations Proposed Seawall at Mokuleia Beach Homes TMK: 6-8-10: 23 to 26 <u>Mokuleia, Waialua, Oahu, Hawaii</u>

We have recently completed our field exploration for the proposed Seawall at Mokuleia Beach Homes across from Dillingham Field on Farrington Highway in Mokuleia on the island of Oahu, Hawaii. This letter serves to present our preliminary geotechnical engineering recommendations for the seawall design of the proposed project.

### **Project Considerations**

We understand that it is proposed to construct a grouted stone masonry seawall along four adjacent lots at the proposed project site. At the time of our field exploration, the project site consisted of 4 beach homes on lots with existing seawalls in various stages of disrepair.

### **Subsurface Conditions**

Our field exploration program consisted of drilling and sampling one boring, designated as Boring No. 1 to a depth of about 31 feet below the existing ground surface. In general, our field exploration encountered a thin layer of clayey silt overlying coral sand to a depth of approximately 23 feet below the existing ground surface. The sandy materials were generally underlain by a layer of very stiff silty clay to a depth of about 29 feet below the existing ground surface. Beneath the clay layer, coral sand was encountered to the maximum depth drilled of approximately 31 feet below the existing ground surface. Detailed findings are presented on the Log of Boring, Plates A and A-1. The approximate boring location is shown on the Site Plan, Plate 2.

2006 Kalihi St. • Honolulu, Hawaii 96819 Facsimile: (808) 847-1749 • Phone: (808) 841-5064 Tom Nance Water Resource Engineering W.O. 3793-00 June 25, 1997

Groundwater was encountered in the drilled boring at an approximate depth of 8.2 feet below the existing ground surface at the time of our field exploration. However, groundwater levels can fluctuate depending on factors such as seasonal rainfall, groundwater withdrawal and/or injection, tidal effects and other factors.

### <u>Seawall</u>

Based on the anticipated subsurface conditions at the project site, it is our opinion that the following general guidelines may be used for preliminary design of the proposed seawall structure.

### Wall Foundations

In general, we believe that retaining wall foundations may be designed with an allowable bearing pressure of up to 3,000 pounds per square foot (p.s.f.) bearing on the in-situ coral sand. This bearing value is for dead plus live loads and may be increased by one-third for transient loads, such as those caused by wind or seismic forces.

Ideally, the walls should bear on hard, erosion-resistant formations to resist undermining by wave action; however, hard layers were not encountered within the depth of our exploration. In order to provide some resistance, the wall foundations should be embedded as deep as practicable. It should be understood that the walls will be susceptible to undermining by wave action.

### Lateral Earth Pressures

The retaining wall should be designed to resist the lateral earth pressures due to adjacent soils and surcharge effects. The recommended lateral earth pressures for design of retaining walls with level backfill conditions, expressed in equivalent fluid pressures, are presented below.

| Wall Condition | Equivalent Fluid Pressure<br>p <u>er foot of depth</u><br>(p.c.f.) |
|----------------|--------------------------------------------------------------------|
| Above Water    | 40                                                                 |
| Below Water    | 85                                                                 |

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Tom Nance Water Resource Engineering W.O. 3793-00 June 25, 1997

The values provided above assume the wall is free to deflect and that on-site granular fill and/or select granular fill will be used to backfill behind the wall. It is assumed that the backfill behind retaining walls will be compacted to between 90 and 95 percent relative compaction. Over-compaction of the retaining wall

In general, an active condition may be used for walls that are free to deflect by as much as 0.1 percent of the wall height. If the tops of walls are not free to deflect beyond this degree or are restrained, the walls should be designed for the at-rest condition. Surcharge stresses due to areal surcharges, line loads, and point loads within a horizontal distance equal to the depth of the wall should be considered in

the design. Lateral loads acting on the structure may be resisted by frictional resistance between the base of the foundation and the bearing materials. A coefficient of friction of 0.4 may be used for footings bearing on the coral sand or embedded in structural fill.

We recommend that footing excavations for the proposed seawall be observed by a representative of Geolabs-Hawaii prior to placement of reinforcing steel or concrete to confirm the foundation bearing conditions and the required embedment depths.

### <u>Drainage</u>

Retaining walls should be well-drained to reduce the build-up of hydrostatic pressures above the water level. A typical drainage system would consist of a 1 to 2-foot wide zone of permeable material, such as No. 3B Fine gravel (ASTM C 33, No. 67 gradation), immediately around a perforated pipe (perforations down) at an elevation above the water level discharging to an appropriate outlet or weepholes. Backfill behind the permeable drainage zone should consist of granular fill material less than 3 inches in maximum dimension.

### **Design Review**

Preliminary and final drawings and specifications for the proposed new seawall project should be forwarded to Geolabs-Hawaii for review and written comments prior to advertisement for bidding. This review is necessary to evaluate conformance with the intent of the earthwork and foundation recommendations provided herein. If this review is not made, Geolabs-Hawaii cannot be responsible for misinterpretation of our recommendations.

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Page 3

Tom Nance Water Resource Engineering W.O. 3793-00 June 25, 1997

### **Construction Monitoring**

It is recommended that Geolabs-Hawaii be retained to provide geotechnical engineering services during the construction of the proposed project. The items of construction monitoring that are critical requiring "Special Inspection" include observation of footing excavation, subgrade preparation, fill placement and compaction. Other aspects of earthwork construction should also be observed by a representative from Geolabs-Hawaii. This is to observe compliance with the design concepts, specifications, or recommendations and to expedite suggestions for design changes that may be required in the event that subsurface conditions differ from those anticipated at the time this letter report was prepared. The recommendations provided in this report are contingent upon such observations. If actual exposed subsurface conditions encountered during construction are different from those assumed or considered in this report, then appropriate modifications to the design should be made.

### <u>Closure</u>

The preliminary recommendations provided above are for information and preliminary design purposes. Detailed recommendations for design of foundations, site preparation, and pavements will be presented in our forthcoming report. If you have guestions or need additional information, please contact our office.

Respectfully submitted,

C.W. ASSOCIATES, INC. dba GEOLABS-HAWAII

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Clayton S. Mimura, P.E. President

Attachments: Log of Boring (Plates A and A-1) Site Plan (Plate 2)

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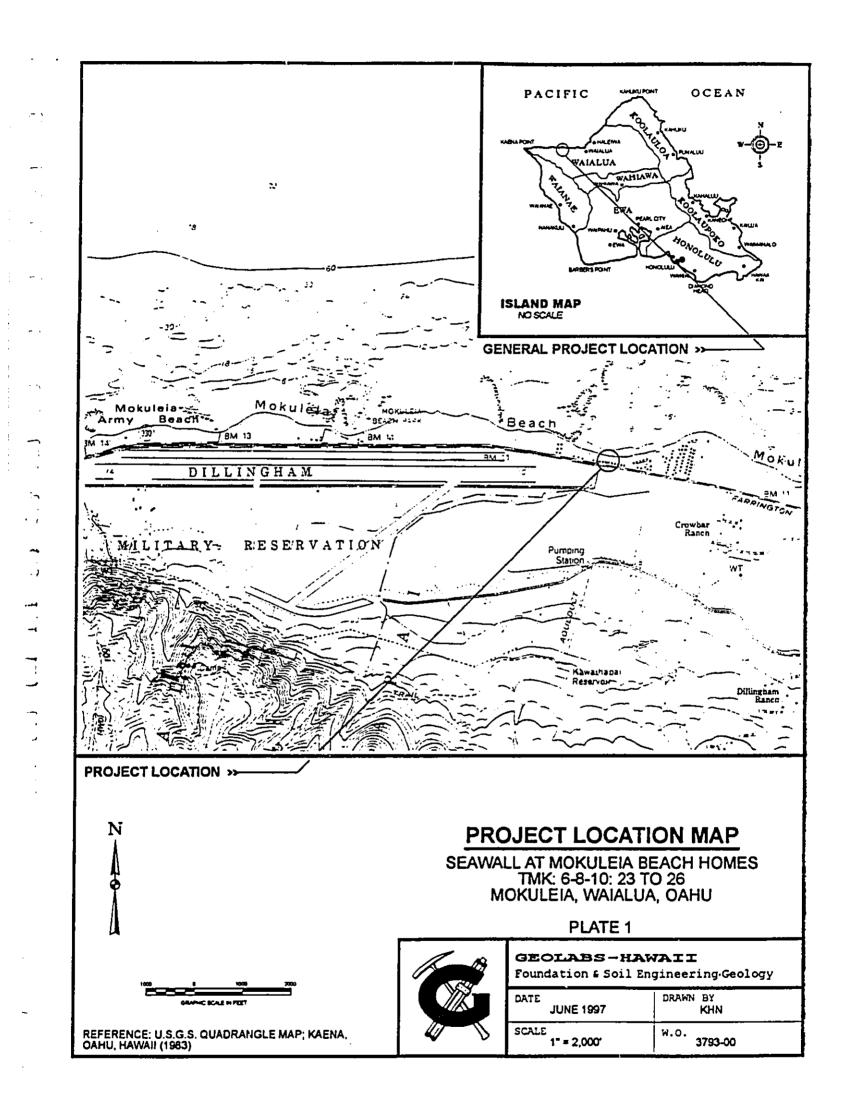
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|-------------------------------------------------------|---------------------------------------------------------------------|---------------------------------------|---------------------------------------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------------|---------|
|                                                       | T                                                                   | CLEAN<br>GRAVELS                      | GW                                                                                                | WELL-GRADED GRAVELS, GRAVEL-<br>MIXTURES, LITTLE OR NO FINES                                                    | SAND    |
|                                                       | GRAVELS                                                             | LESS THAN 5%<br>FINES                 | GP                                                                                                | POORLY-GRADED GRAVELS, GRAVI<br>MIXTURES, LITTLE OR NO FINES                                                    | EL-SAND |
| COARSE-<br>GRAINED                                    | MORE THAN 50% OF<br>COARSE FRACTION<br>RETAINED ON NO. 4<br>SIEVE   | GRAVELS WITH<br>FINES                 | GM                                                                                                | SILTY GRAVELS, GRAVEL-SAND-SIL<br>MIXTURES                                                                      | .T      |
| SOILS                                                 | 31272                                                               | MORE THAN 12%<br>FINES                | GC                                                                                                | CLAYEY GRAVELS, GRAVEL-SAND-S<br>MIXTURES                                                                       | SILT    |
| MORE THAN 50%                                         | SANDS                                                               | CLEAN SANDS                           | sw                                                                                                | WELL-GRADED SANDS, GRAVELLY S                                                                                   | SANDS.  |
| OF MATERIAL<br>RETAINED ON NO.<br>200 SIEVE           |                                                                     | LESS THAN 5%<br>FINES                 | SP                                                                                                | POORLY~GRADED SANDS, GRAVELL<br>LITTLE OR NO FINES                                                              | Y SANDS |
|                                                       | 50% OR MORE OF<br>COARSE FRACTION<br>PASSING THROUGH<br>NO. 4 SIEVE | SANDS WITH<br>FINES                   | SM                                                                                                | SILTY SANDS, SAND-SILT MIXTURES                                                                                 | 3       |
|                                                       |                                                                     | MORE THAN 12%<br>FINES                | SC                                                                                                | CLAYEY SANDS, SAND-CLAY MIXTU                                                                                   | RES     |
|                                                       |                                                                     |                                       | ML                                                                                                | INORGANIC SILTS AND VERY FINE S<br>ROCK FLOUR, SILTY OR CLAYEY F<br>SANDS OR CLAYEY SILTS WITH SI<br>PLASTICITY | INE     |
| FINE-<br>GRAINED<br>SOILS                             | LIQUID LIMIT<br>LESS THAN 50                                        | CL                                    | INORGANIC CLAYS OF LOW TO MED<br>PLASTICITY, GRAVELLY CLAYS, SA<br>CLAYS, SILTY CLAYS, LEAN CLAYS | NDY                                                                                                             |         |
| SOILS                                                 |                                                                     |                                       | OL                                                                                                | ORGANIC SILTS AND ORGANIC SILT<br>OF LOW PLASTICITY                                                             | Y CLAYS |
| 50% OR MORE OF<br>MATERIAL PASSING<br>THROUGH NO. 200 | SILIS                                                               |                                       | мн                                                                                                | INORGANIC SILT, MICACEOUS OR<br>DIATOMACEOUS FINE SAND OR SI<br>SOILS                                           | ILTY    |
| SIEVE                                                 | AND<br>CLAYS                                                        | 50 OR MORE                            | Сн                                                                                                | INORGANIC CLAYS OF HIGH PLASTI                                                                                  |         |
|                                                       |                                                                     |                                       | ОН                                                                                                | ORGANIC CLAYS OF MEDIUM TO HIS<br>PLASTICITY, ORGANIC SILTS                                                     |         |
|                                                       | HLY ORGANIC SO                                                      |                                       | PT                                                                                                | PEAT, HUMUS, SWAMP SOILS WITH<br>ORGANIC CONTENTS                                                               | HIGH    |
|                                                       | BOLS ARE USED T                                                     | O INDICATE BOF                        | DERLINE S                                                                                         | OIL CLASSIFICATIONS                                                                                             |         |
| LEGEND:<br>2-INCH O.D.                                | STANDARD PENETRA                                                    | TION TEST                             | u ua                                                                                              | טוס נואת                                                                                                        |         |
| З-іЮСН О.D.                                           | MODIFIED CALIFORNI                                                  | A SAMPLE                              | PI PLA                                                                                            | STICITY INDEX                                                                                                   |         |
|                                                       | BE SAMPLE                                                           |                                       | τν το                                                                                             | RVANE SHEAR (151)                                                                                               |         |
|                                                       |                                                                     |                                       |                                                                                                   | CKET PENETROMETER (131)                                                                                         |         |
| REC CORE RECO                                         |                                                                     |                                       | ¥ wa                                                                                              | TER LEVEL OBSERVED IN BORING                                                                                    |         |
|                                                       | ITY DESIGNATION                                                     |                                       |                                                                                                   |                                                                                                                 |         |
| CW ASSOCIATE                                          | S, INC. dba                                                         |                                       |                                                                                                   | G LEGEND                                                                                                        | PLA     |
|                                                       |                                                                     | · · · · · · · · · · · · · · · · · · · |                                                                                                   |                                                                                                                 |         |
| EOLABS                                                | - HAWAII                                                            |                                       | ALL AT MOI<br>3-8-10: 23 T(                                                                       | KULEIA BEACH HOMES                                                                                              | A       |

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| <br>1          | lat              | e Starte                   | <br>d:     | 6             | /5/97                        |          |          |          | ill Rig: Diedrich D-25                                                                                            |            |
|----------------|------------------|----------------------------|------------|---------------|------------------------------|----------|----------|----------|-------------------------------------------------------------------------------------------------------------------|------------|
|                |                  | B Comp                     |            | 6             | /5/97                        |          |          | C        | illing Method: H.S. Auger (4.25")                                                                                 |            |
|                |                  | ged By                     |            | N             | <u>I. Lee</u>                |          |          | C        | riving Energy: 140 lb. wt., 30 in. drop                                                                           |            |
| -              | lot              | al Depti                   |            |               | <u>1.0 feet</u>              |          |          |          |                                                                                                                   |            |
|                | F                | IELD                       |            | BORAT         |                              |          |          |          | DESCRIPTION                                                                                                       |            |
| #              |                  | a.<br>ft                   | רב         | a<br>t        | sss.<br>ath                  |          |          | tsf      |                                                                                                                   |            |
| Depth,         | Sample           | Penetr<br>Resist<br>Blous/ | 4<br>Lisit | i stu<br>nter | Compress.<br>Strength<br>ksf | Other    | Data     | Pen,     | Approximate Surface Elevation (ft): 11*                                                                           |            |
| Ö              | Sa               | 9 8 9<br>9 8 9             | 600        | ភិក្ខុ        | ភ្លូ ហ្លូ ភ្ល                | ō        | <u>ö</u> | <u>e</u> | Dark brown CLAYEY SILT (MH) with                                                                                  |            |
|                |                  | 14                         |            | 9             |                              |          |          |          | KH rootlets stiff. drv                                                                                            | _/         |
| •              | B                |                            | [          | 1.2           | [ [                          |          |          |          | Tan CORAL SAND (SP), medium dense,<br>damp                                                                        |            |
|                | 4                | 19                         |            | 13            |                              |          |          |          | grades with some coral fragments                                                                                  |            |
| 5 -            |                  | 18                         |            | 10            |                              |          |          |          |                                                                                                                   |            |
|                | Π                |                            |            | ₽             |                              |          |          |          |                                                                                                                   |            |
|                | $\left  \right $ |                            | =          | Ŧ             | ן ו                          |          |          |          | in the second and shall from on                                                                                   | te         |
| 10             | N                | 4                          |            | 36            |                              |          |          |          | grades with more coral and shell fragment<br>(SW-SM), loose, wet                                                  | IJ         |
|                | Н                |                            |            | 1             |                              |          |          |          |                                                                                                                   |            |
|                |                  |                            |            | 1             |                              |          |          |          |                                                                                                                   |            |
| 15 -           | N                | 2                          | ļ          | l<br>l        |                              |          |          |          | grades to very loose                                                                                              |            |
|                |                  |                            |            | ļ             |                              |          |          |          |                                                                                                                   |            |
|                | $\left  \right $ | :                          | 1          |               |                              |          |          | ļ        |                                                                                                                   |            |
| 20-            |                  | 13                         |            | 27            |                              |          |          |          | grades less coarse, medium dense                                                                                  |            |
| 2.0            | ₽                |                            |            |               |                              |          |          |          |                                                                                                                   |            |
|                | 1                |                            |            | Ì             |                              | ł        |          |          | Dark brown SILTY CLAY (CH), very stiff                                                                            |            |
|                |                  |                            |            | 33            |                              | Į        |          |          |                                                                                                                   |            |
| 25 ·           | 1                | 23                         |            | 33            |                              |          |          |          |                                                                                                                   |            |
|                | $\frac{1}{2}$    | ļ                          |            |               | 1                            |          |          |          |                                                                                                                   |            |
|                | 1                |                            |            |               |                              |          |          |          | Tan SAND (SW) with coral and shell                                                                                |            |
| 30-            |                  | 3                          |            |               |                              |          |          |          | fragments, very loose                                                                                             |            |
|                | T                |                            |            |               |                              |          |          |          | Boring terminated at 31 feet<br>Groundwater level at:                                                             |            |
|                | 1                |                            |            |               |                              |          |          |          | Depth Hours Date                                                                                                  |            |
| 35             | -                |                            | 1          |               |                              |          |          |          | 8.2 ft. 1530 06/05/97                                                                                             |            |
|                | 1                |                            |            |               |                              |          |          |          | *Elevation estimated from Topographic<br>Survey map provided by Tom Nance<br>Water Resource Engineering on June 1 | ~          |
|                |                  |                            |            |               |                              |          |          | 1        | Water Resource Engineering on June 1                                                                              | 9,         |
| 40-            |                  | <br>                       |            |               |                              | <u> </u> |          |          | 1997.                                                                                                             |            |
|                |                  | RTL.8\TM.F                 |            |               |                              |          |          |          |                                                                                                                   | AT         |
| <u>+k</u><br>C | W                |                            | CIAT       | ES, II        | NC. dt                       | a        |          | L        | DG OF BURING 1                                                                                                    |            |
| G              | E                | OL                         | A B S      | 5 - H         | AW                           | AII      |          |          | AWALL AT MOKULEIA BEACH HOMES                                                                                     |            |
|                |                  |                            |            |               | dation E                     |          | ering    |          |                                                                                                                   | <b>\-1</b> |
|                |                  | ORDE                       |            |               |                              |          | Jun 97   | .   M    | DKULEIA, OAHU, HAWAII                                                                                             |            |



# Appendix C

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Retaining Wall Calculations By Structural Analysis Group

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# STRUCTURAL CALCULATIONS

TOM NANCE

### FOR

# **PROPOSED SEAWALL AT: MOKULEIA BEACH HOMES**

TMK: 6-8-10: 23 TO 26 MOKULEIA, WAIALUA, OAHU, HAWAII



THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION

PREPARED BY

STRUCTURAL ANALYSIS GROUP 2353 BERETANIA STREET ROOM 201 HONOLULU, HAWAII 96826

JUNE 1997

| STRUCTURAL ANAL                  | YSIS GROUP,       | INC.  | PAGE              | -        |              |  |  |
|----------------------------------|-------------------|-------|-------------------|----------|--------------|--|--|
| 2353 SOUTH BERET                 | ANIA STREET       | #201  | DATE              |          |              |  |  |
| HONOLULU, HAWAII                 | 96826             |       | ENGIN             | PA       | PAGE SEAWALL |  |  |
| EAWALL DESIGN                    |                   |       | SEAWALL           |          |              |  |  |
| SHALLOW WALL DE<br>-0" EMBEDMENT | SIGN              |       | SEAWALL DESIGN    |          |              |  |  |
| DATA INPUT :                     |                   |       |                   |          |              |  |  |
|                                  |                   |       | MATERIAL DENSITY: |          | 100 005      |  |  |
| HEIGHT (YY1)                     | = 18.00           |       | SOIL ABOVE WATER  | ш        | 120 PCF      |  |  |
| VALL EMBED (YY2)                 | = 6.00            |       | SOIL BELOW WATER  | =        | 58 PCF       |  |  |
| VERTICAL FACE (YY5)              |                   |       | WALL ABOVE WATER  | #        | 140 PCF      |  |  |
| TOP WIDTH (XX3)                  | <b>≃</b> 1.25     |       | WALL BELOW WATER  | 8        | 78 PCF       |  |  |
| FRONT BATTER (BAT1)              |                   | X:12  | FRICTION FACTOR   | =        | 0.40         |  |  |
| REAR BATTER (BAT2)               |                   | X:12  | ALLOWABLE BEARING | -        | 3000 PSF     |  |  |
| NEGLECT (N)                      | = 0.00            |       | BEARING FACTOR    | -        | 1            |  |  |
| SLIDE FS                         | = 1.53            |       | PASSIVE PRESSURE  | =        | 62 PCF       |  |  |
| OVERTURN FS                      | = 1.70            | )     | PASSIVE FACTOR    | #        | 1            |  |  |
| MAX BEARING                      | <del>=</del> 2214 | PSF   | ACTIVE ABOVE H2O  | =        | 40 PCF       |  |  |
| AIN BEARING                      | = 1215            | PSF   | ACTIVE BELOW H2O  | =        | 90 PCF       |  |  |
| COMPUTE EXTERNAL A               | CTIVE FORCES:     |       |                   |          |              |  |  |
| PA1                              | = 2880            | LBS   | YP1               | =        | 10.00 FT     |  |  |
| A2                               | = 2880            | ) LBS | YP2               | =        | 3.00 FT      |  |  |
| PA3                              | =1620             | LBS   | YP3               | =        | 2.00 FT      |  |  |
| OTAL ACTIVE P (PA)               | = 7380            | ) LBS |                   |          |              |  |  |
| COMPUTER EXTERNAL                | PASSIVE FORCE:    |       |                   |          |              |  |  |
| FP                               | = 1116            | 5 LBS | YP                | -        | 2.00 FT      |  |  |
| COMPUTE VARIOUS LEN              | GTHS:             | ·     |                   | <u> </u> |              |  |  |
| xxı                              | = 2.88            | B FT  |                   |          |              |  |  |
| CX2                              | = 5.75            | 5 FT  |                   |          |              |  |  |
| CX4                              |                   | D FT  |                   |          |              |  |  |
| CX5                              | = 12.00           |       |                   |          |              |  |  |
| (Y3                              | = 12.00           |       |                   |          |              |  |  |
| YY4                              | = 10.00           |       |                   |          |              |  |  |
| XX6                              | = 14.8            |       |                   |          |              |  |  |
|                                  |                   |       |                   |          |              |  |  |

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| STRUCTURAL AN<br>2353 SOUTH BER<br>HONOLULU, HAW | ETANIA   | STREET #201           | PAGE<br>DATE<br>ENGIN | F                  | PAGE SEAWALL |  |  |
|--------------------------------------------------|----------|-----------------------|-----------------------|--------------------|--------------|--|--|
| SEAWALL DESIGN                                   | 4        |                       | SEAWALL               |                    |              |  |  |
| SHALLOW WALL [<br>8'-0" EMBEDMENT                |          |                       | SEAWALL DESIGN        |                    |              |  |  |
| COMPUTE WALL PROP                                | PERTIES: |                       | ~~~~~~                | فسف بی وجد حد نت ک |              |  |  |
| WALL WEIGHT:                                     |          |                       | ECCENTRICITY FROM     | TOE                |              |  |  |
| WT1                                              | =        | 4140 LBS              | XWT1                  | =                  | 13.44 FT     |  |  |
| WT2                                              | =        | 4140 LBS              | XWT2                  | =                  | 10.08 FT     |  |  |
| WT3                                              | =        | 4830 LBS              | XWT3                  | =                  | 8.17 FT      |  |  |
| NT4                                              | =        | 2100 LBS              | XWT4                  | -                  | 5.63 FT      |  |  |
| WT5                                              | =        | 3500 LBS              | XWT5                  | ~                  | 3.33 FT      |  |  |
| VT6                                              | =        | 500 LBS               | XWT6                  | =                  | 13.92 FT     |  |  |
| VT7                                              | =        | 673 LBS               | XWT7                  | =                  | 12.96 FT     |  |  |
| VT8                                              | =        | 5616 LBS              | XWT8                  | =                  | 6.00 FT      |  |  |
| OTAL DL (WT)                                     | =        | 25499 LBS             | MOMENT ABOUT TOE:     |                    |              |  |  |
|                                                  |          |                       | MWT1                  | ==                 | 55631 FT-#   |  |  |
| <b>.</b>                                         |          |                       | MWT2                  | -                  | 41745 FT-#   |  |  |
| G WT FROM TOE                                    | =        | 8.22 FT               | MWT3                  | =                  | 39445 FT-#   |  |  |
|                                                  |          |                       | MWT4                  | =                  | 11813 FT-#   |  |  |
|                                                  |          |                       | MWT5                  | =                  | 11667 FT-#   |  |  |
|                                                  |          |                       | MWT6                  | #                  | 6962 FT-#    |  |  |
|                                                  |          |                       | MWT7                  | =                  | 8718 FT-#    |  |  |
|                                                  |          |                       | MWT8                  | =                  | 33696 FT-#   |  |  |
|                                                  |          |                       | SUMMATION (MWT)       | =                  | 209676 FT-#  |  |  |
| HECK SLIDING:                                    |          |                       |                       | <u> </u>           |              |  |  |
| LIDING FORCE (SF)                                | E        | 7380 LBS              | SLIDING FS (SFS)      | =                  | 1.53         |  |  |
| UCTION FORCE                                     | =        | 10200 1 50            |                       |                    | <u> </u>     |  |  |
| ASSIVE FORCE                                     | -        | 10200 LBS             |                       |                    |              |  |  |
| OTAL RESIST (RF)                                 |          | 1116 LBS<br>11316 LBS |                       |                    |              |  |  |

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| STRUCTURAL A<br>2353 SOUTH BE<br>HONOLULU, HA | RETANIA S | TREET #201  | PAGE<br>DATE<br>ENGIN | PA      | PAGE SEAWAL |  |  |
|-----------------------------------------------|-----------|-------------|-----------------------|---------|-------------|--|--|
| SEAWALL DESIG                                 | GN        |             | SEAWALL               |         |             |  |  |
| SHALLOW WAL<br>8'-0" EMBEDME                  |           |             | SEAWALL DESIGN        |         |             |  |  |
| CHECK BEARING:                                |           |             |                       |         |             |  |  |
| P/A (SPA)                                     | =         | 1714 PSF    | MOMENT ABOUT CL F     | OOTING: |             |  |  |
| M/S (SMS)                                     | a         | 500 PFS     | MPA1                  | =       | 28800 FT-#  |  |  |
|                                               |           |             | MPA2                  |         | 8640 FT-#   |  |  |
| SMAX                                          | =         | 2214 PSF    | MPA3                  | #       | 3240 FT-#   |  |  |
| SMIN                                          | =         | 1215 PSF    | MPA SUM               | -       | 40680 FT-#  |  |  |
|                                               |           |             | MFP (PASSIVE)         | =       | -2232 FT-#  |  |  |
|                                               |           |             | MWTCL                 | =       | -20027 FT-# |  |  |
|                                               |           |             | MCL (AT XX6/2)        | =       | 18421 FT-#  |  |  |
|                                               |           |             | S                     | =       | 36.88 FT^3  |  |  |
| CHECK OVERTURN                                | ING:      |             | <u> </u>              | ;       | <u> </u>    |  |  |
| OTM                                           | =         | 38448 FT-LB | XX7                   | =       | 5.67 FT     |  |  |
| MR                                            | =         | 65187 FT-LB |                       |         |             |  |  |

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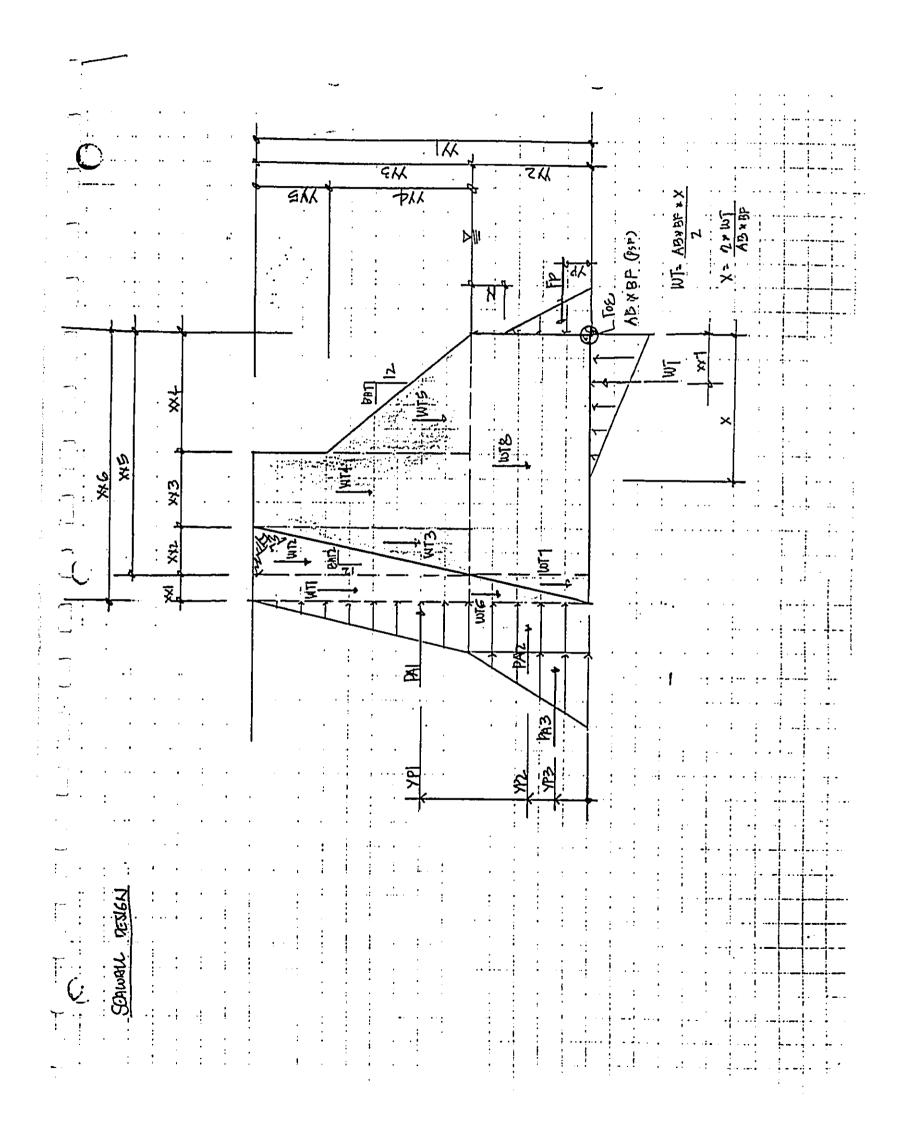
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# APPLICATION FOR SHORELINE SETBACK VARIANCE AND FINAL ENVIRONMENTAL ASSESSMENT Reconstruction of Seawall at 68-705 Farrington Highway, Mokuleia

## EXHIBIT D

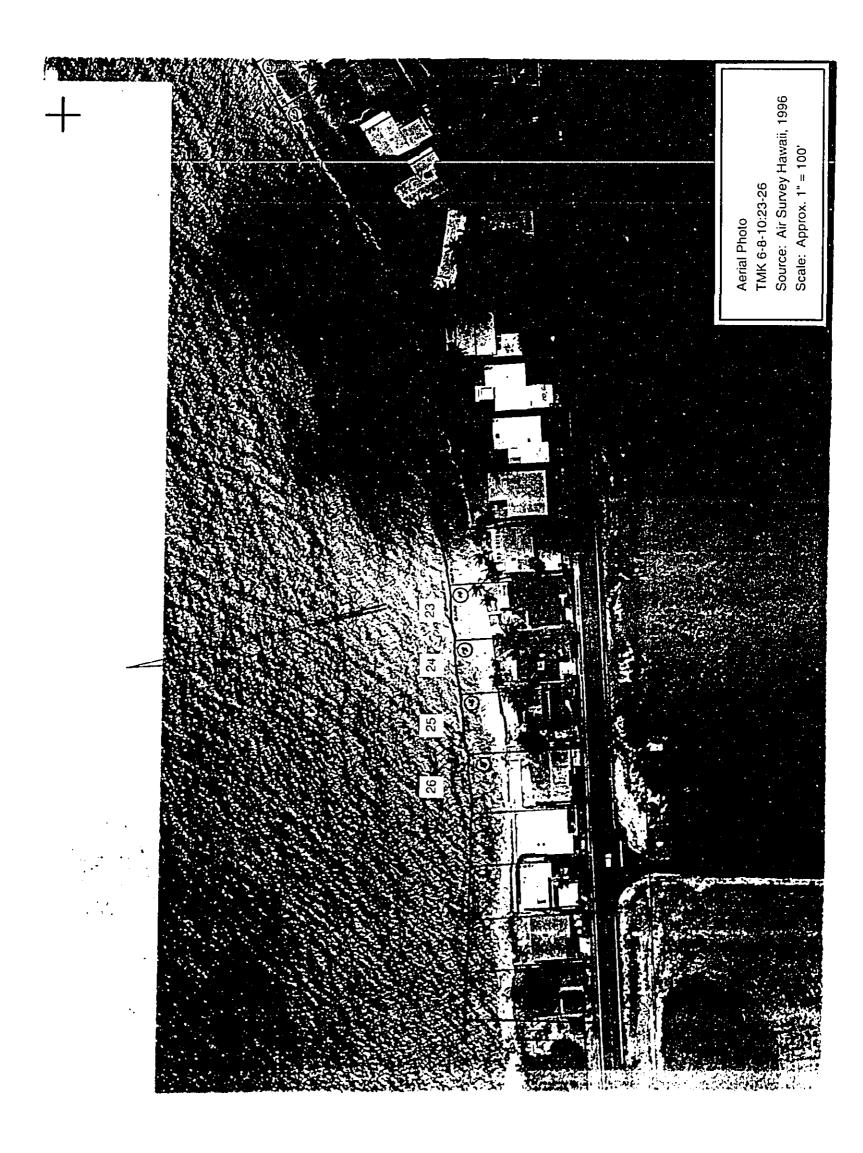
### **AERIAL PHOTOGRAPHS** for TMK 6-8-10:23-26

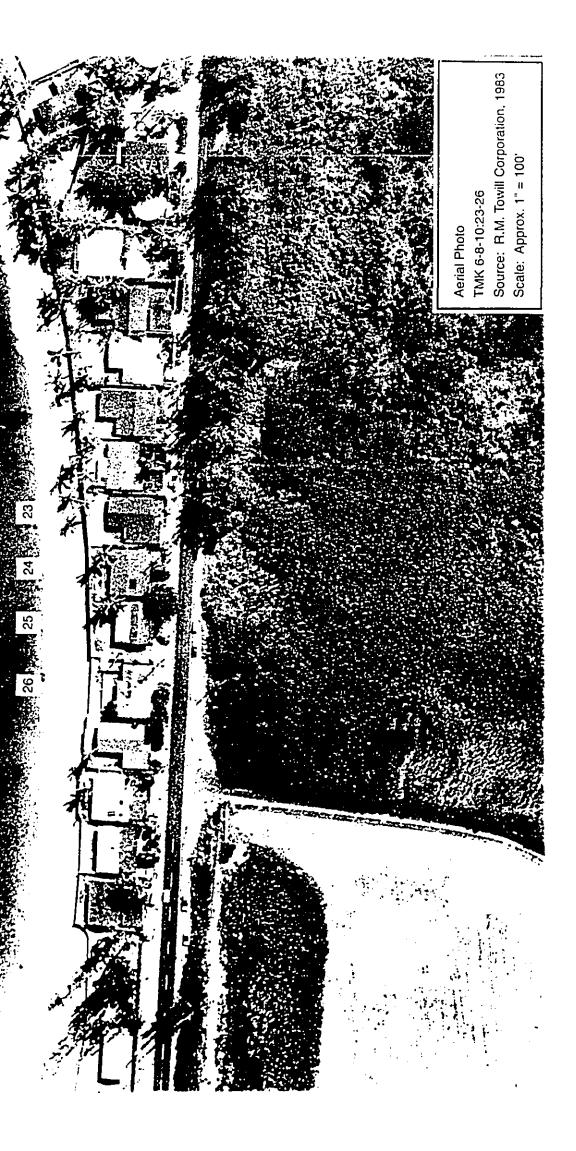
1949 - 1996

Mokuleia, Oahu, Hawaii

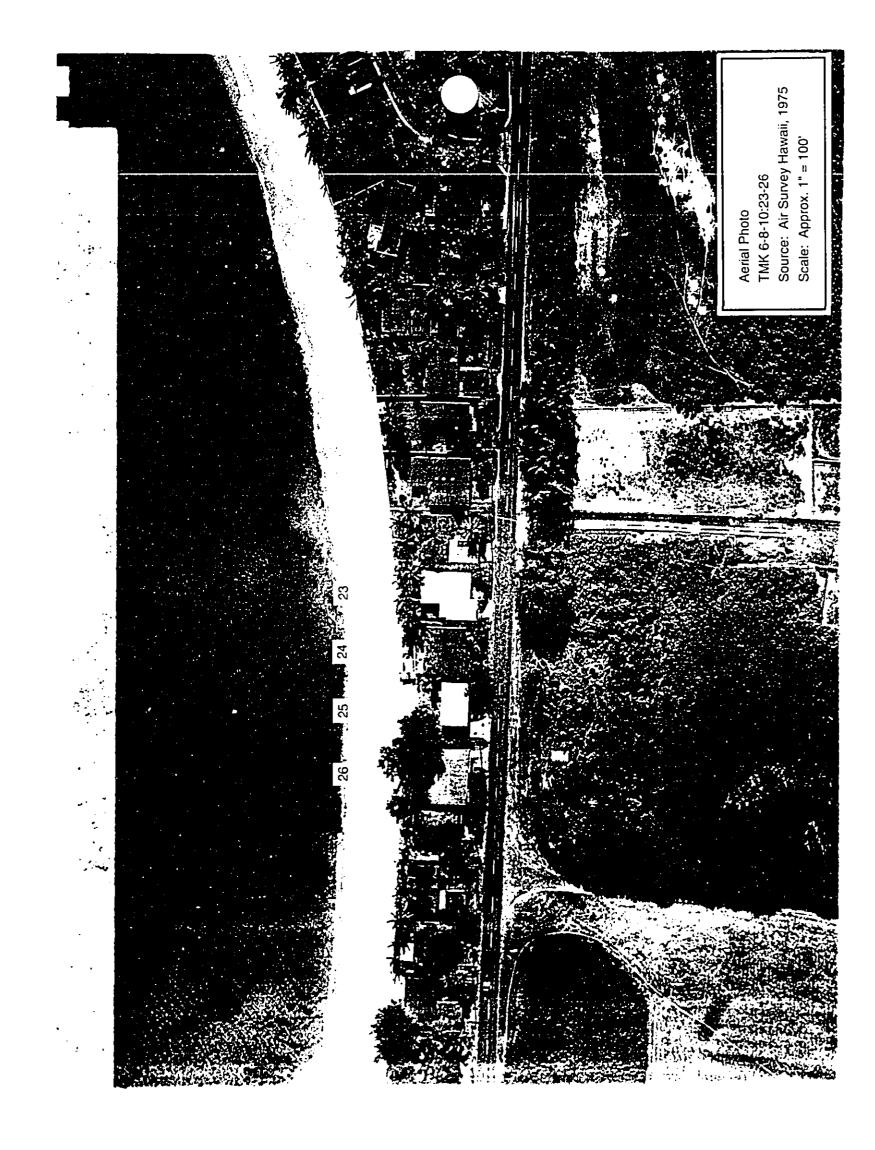
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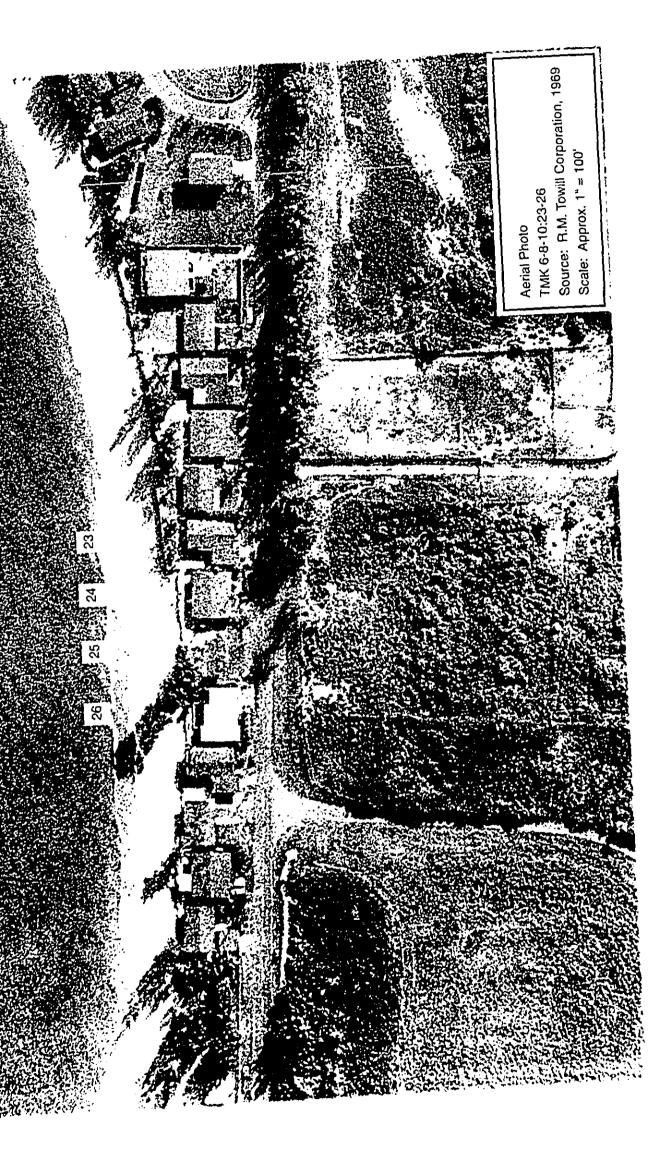
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