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# ROYAL KUNIA PHASE II SEWER MASTER PLAN UPDATE

Ewa, Oahu, Hawaii

January 16, 2024

**PREPARED FOR:**

Haseko Royal Kunia LLC  
91-1001 Kaimalie Street, #205  
Ewa Beach, Hawaii 96706



**R. M. TOWILL CORPORATION**  
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**EXHIBIT E**

**ROYAL KUNIA PHASE II**

**SEWER MASTER PLAN UPDATE**

January 16, 2024



*Michael H. Okamoto* 4/30/2024  
SIGNATURE EXPIRATION DATE  
OF LICENSE

THIS WORK WAS PREPARED BY ME  
OR UNDER MY SUPERVISION AND  
CONSTRUCTION OF THIS PROJECT  
WILL BE UNDER MY OBSERVATION.

Prepared by:

R. M. Towill Corporation  
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Prepared for:

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**TABLE OF CONTENTS**

**1. INTRODUCTION ..... 1-1**

    1.1 General ..... 1-1

    1.2 Objectives and Scope ..... 1-1

    1.3 Project Location ..... 1-1

    1.4 Proposed Development..... 1-2

    1.5 Sustainable Communities Plan..... 1-2

**2. EXISTING CONDITIONS ..... 2-1**

    2.1 Existing and Surrounding Uses ..... 2-1

    2.2 Existing Topography ..... 2-1

    2.3 Existing Wastewater Collection System ..... 2-1

    2.4 Existing Wastewater Treatment and Disposal ..... 2-2

**3. CRITERIA AND METHODOLOGY ..... 3-1**

    3.1 Wastewater Demand Criteria ..... 3-1

    3.2 Wastewater System Design Criteria ..... 3-3

    3.3 Impacts to Wastewater Facilities ..... 3-3

**4. PROPOSED CONDITIONS ..... 4-1**

    4.1 Proposed Community Population ..... 4-1

    4.2 Proposed Wastewater Demands and Pipe Sizing ..... 4-3

    4.3 Comparison to the Revised Sewer Master Plan ..... 4-3

    4.4 Proposed Wastewater System ..... 4-4

**5. REFERENCES ..... 5-1**

**APPENDIX A – Wastewater Quantities and Hydraulic Computations**

**APPENDIX B – Royal Kunia Revised Sewer Master Plan**  
(Prepared by Parking Engineering, Inc. dated May 1996)

**LIST OF TABLES**

Table 3-1 Wastewater Demand Criteria ..... 3-2  
Table 3-2 Sewer Pipe Design Criteria ..... 3-3  
Table 4-1 Proposed Land Use ..... 4-2  
Table 4-2 Sewer Master Plan Comparison..... 4-3

**LIST OF FIGURES**

Figure 1-1 Regional Map ..... 1-3  
Figure 1-2 Location Map ..... 1-4  
Figure 1-3 Proposed Conceptual Development Plan ..... 1-5  
Figure 2-1 Existing Sewer ..... 2-3  
Figure 2-2 Regional Sewer Facilities ..... 2-4  
Figure 4-1 Conceptual Sewer Map - Proposed..... 4-5

**1. GENERAL**

**1.1 Background**

Royal Kunia Phase II (RKII) is an approximately 347-acre proposed master planned development and includes the following TMK's: 9-4-002:001, 070, 071, 078 and 079. The proposed development includes commercial and industrial uses, single family and multi-family housing, school, parks, roadways, and utility infrastructure. RKII will include a new sewer system that is intended to be dedicated to the City and County of Honolulu (City).

A Revised Sewer Master Plan for Royal Kunia (revised RKSMP) was prepared by Park Engineering Inc. in May 1996 (See Appendix B). This report updates the RKSMP for RKII.

The proposed sewer master plan for RKII is based on the existing downstream conditions and proposed RKII development plan by Haseko Royal Kunia LLC.

**1.2 Objectives and Scope**

The objective of this sewer master plan (SMP) is to describe the proposed wastewater improvements needed to support the RKII development.

The scope of this SMP includes RKII improvements within parcels 9-4-002:001, 070, 071, 078 and 079. Offsite developments include the State of Hawaii Department of Agriculture (DOA) 24 single family residential units near the northeast corner of the development, and the Robinson Kunia Land LLC development located east of RKII. The proposed infrastructure shall serve RKII and the offsite developments.

**1.3 Project Location**

RKII is located within the Ewa District on the island of Oahu and is approximately 1.3 miles north of the H-1 Freeway junction at Kunia Road. The project is located northeast of Kunia Road and north of Anoiki Street. The project site is bounded by agricultural land to the north, Waikele Gulch to the east, Anoiki Street and the Royal Kunia Phase I subdivision (RKI) and the Royal Kunia Country Club golf course to the south, and Kunia Road to the west. See **Figure 1-1 and 1-2** for the Regional and Location Map.

#### **1.4 Proposed Development**

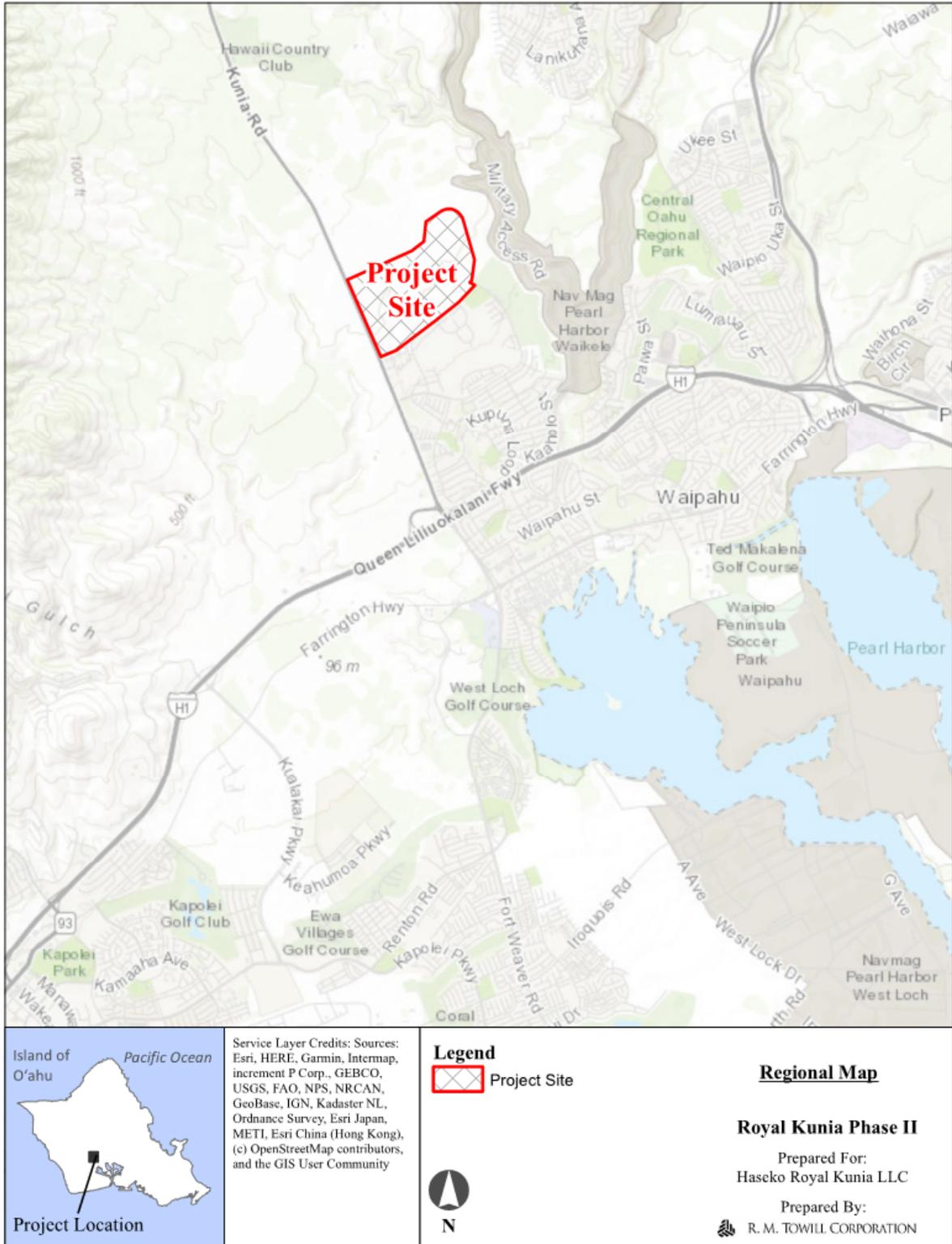
The proposed RKII land uses include single family and multi-family residential, commercial, industrial, school, and parks. The proposed development contains up to 1850 homes, 123.7 acres of commercial and industrial use, 12 acres of schoolgrounds and at least 10 acres of park. See **Figure 1-3** for the proposed conceptual development plan.

#### **1.5 Sustainable Communities Plan**

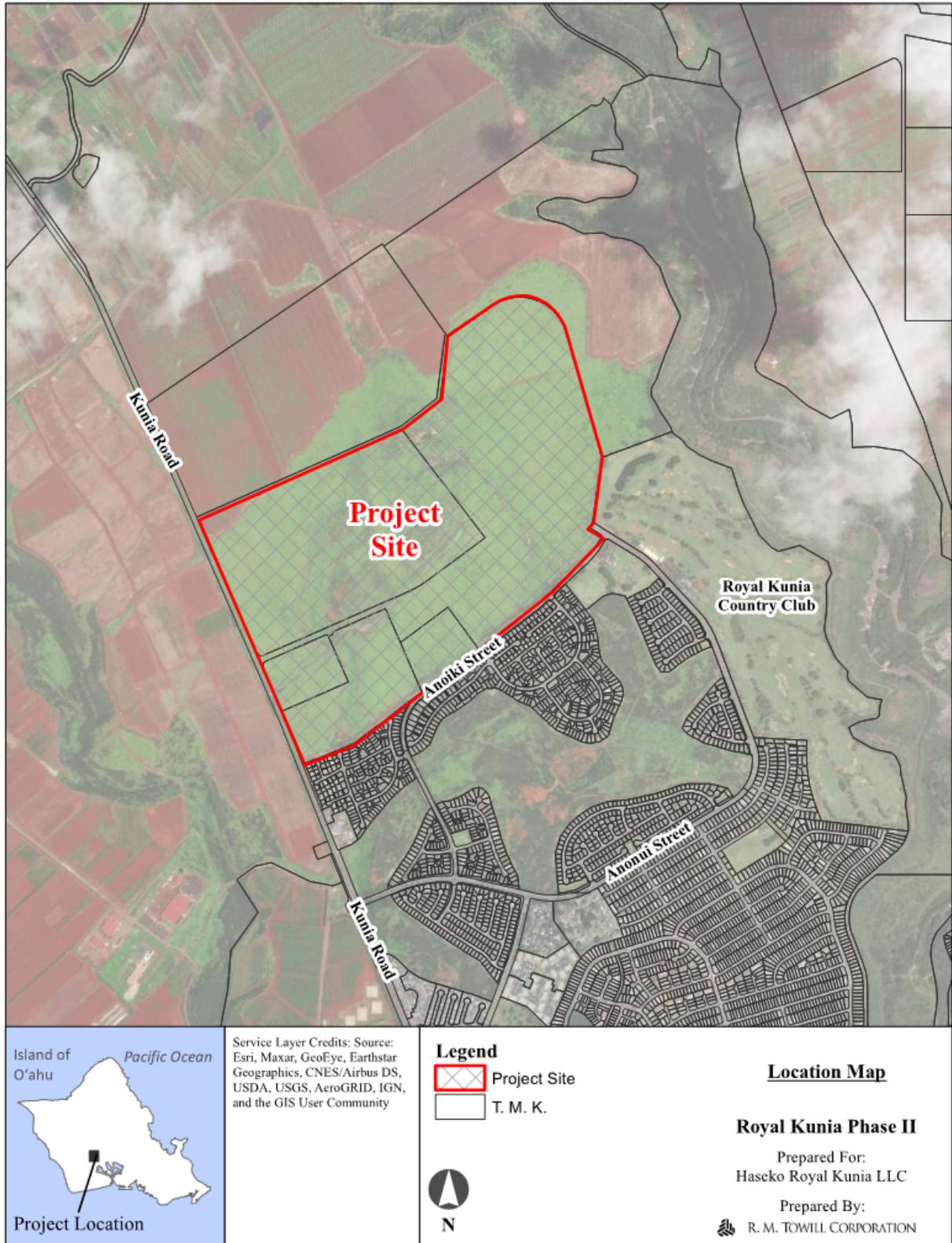
The project is located within the City's Central Oahu Sustainable Communities Plan (SCP) boundary. The Central Oahu SCP, dated March 2021 section 4.3.1, indicates wastewater treatment shall follow the following general policies:

- *“All new developments in Central Oahu to be connected to a regional or municipal sewer service system.”*
- *“Where feasible, use recycled water recovered from treated wastewater effluent as a source of non-potable water for irrigation and other uses where appropriate and approved by the Department of Health and Board of Water Supply.”*
- *“Locate wastewater treatment plants in areas shown as planned for industrial use and away from residential areas shown on the Urban Land Use Map in Appendix A of the SCP”*
- *Use a City review and approval process which provides adequate public notice and input for any major new private wastewater treatment plant. Do not require such comprehensive review and approval for other system elements, such as pump stations and mains.*

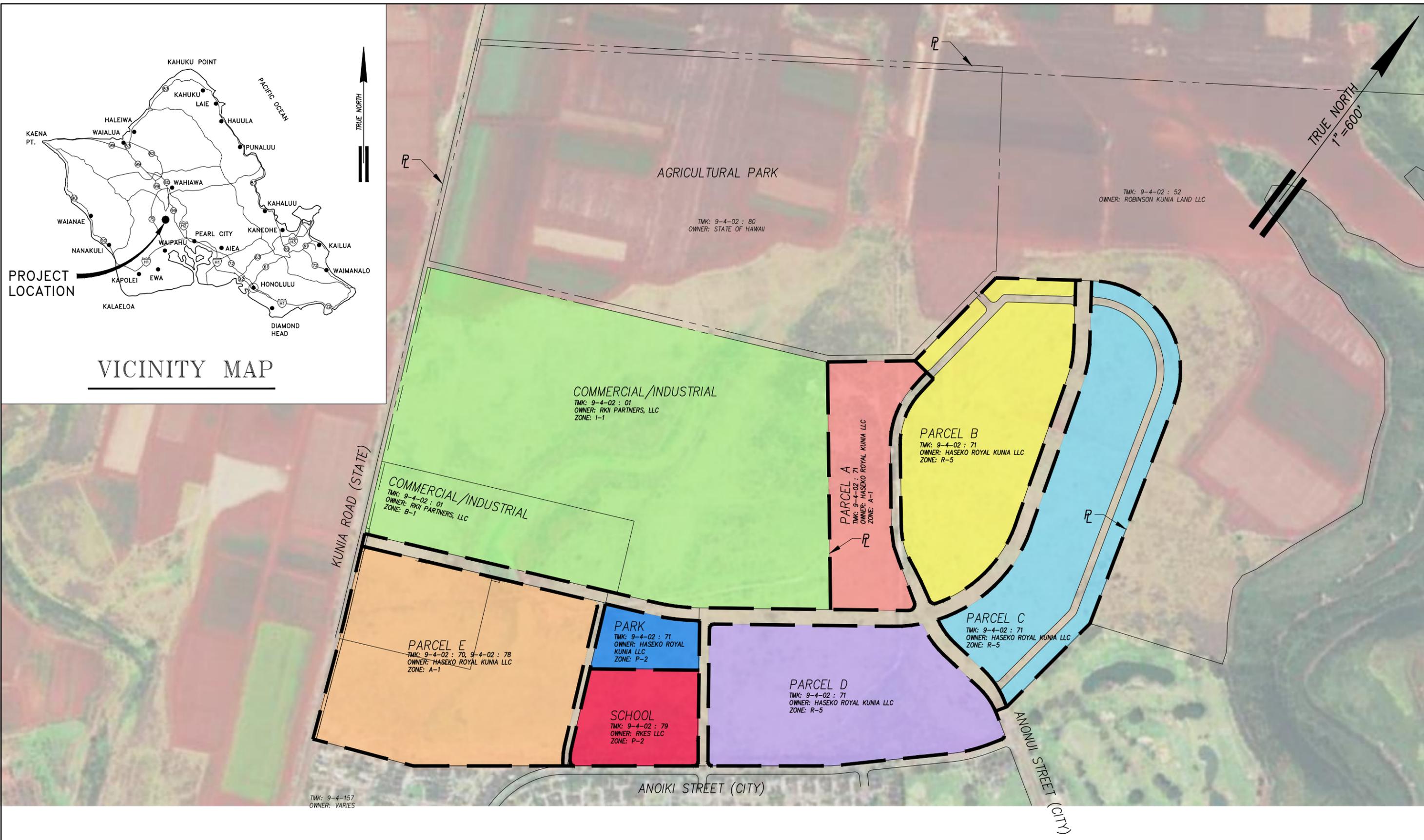
The proposed RKII sewer system supports the City's Central Oahu SCP General Policies indicated above.



**FIGURE 1-1 – Regional Map**



**Figure 1-2 – Location Map**



**2. EXISTING CONDITIONS**

**2.1 Existing and Surrounding Uses**

The existing site is unimproved with ground cover generally consisting of prairie grass, cane grass, and thin stand trees. Existing Improvements adjacent to the site include Kunia Road and the existing RKI subdivision. There are no existing sewer system improvements in the RKII Parcels.

**2.2 Existing Topography**

The overall terrain of the project site is sloping in the southward direction, with elevations varying from approximately 600 feet above mean sea level (MSL) at the north end of the site to 425 feet MSL at the south end of the site. Overall slopes generally vary between 3% to 5%, with steeper slopes of 30% or greater in the Gulch that runs through the western portion of the site.

**2.3 Existing Wastewater Collection System**

The existing RKI wastewater infrastructure is designed to accommodate the proposed RKII wastewater flows in accordance with the approved master plans. Three existing stubouts (15-inch, 18-inch and 8-inch) are located along Anoiki Street and one 18-inch stubout is located at Anonui Street. The locations of the stubouts are shown in **Figure 4-1**.

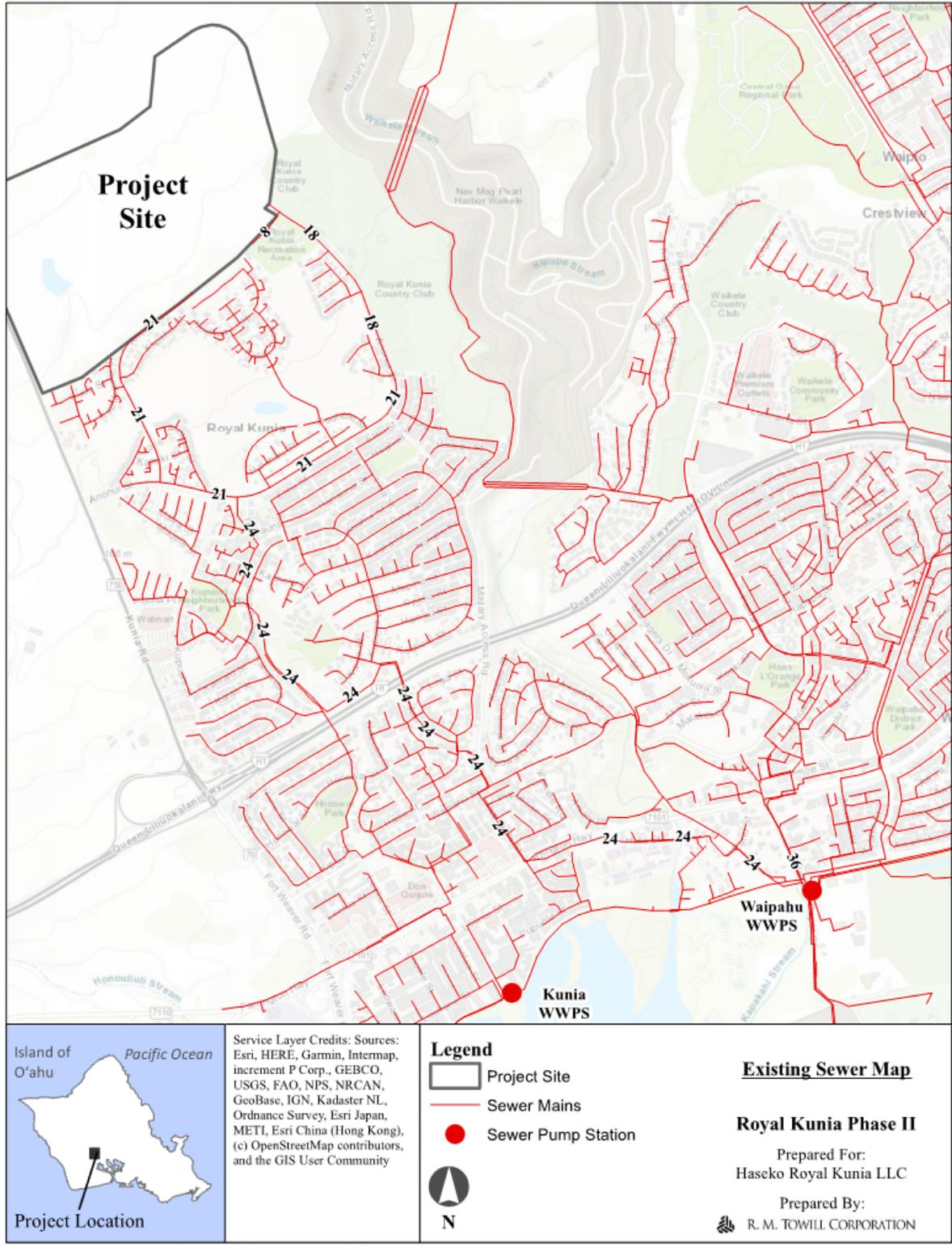
The existing regional wastewater system consisting of 18-inch to 24-inch pipes are designed to convey wastewater flows from RKII, RKI, Village Park, and the west side of Waipahu South to the Waipahu Wastewater Pump Station (Waipahu WWPS) located along Waipahu Depot Street. The Waipahu WWPS pumps collected wastewater southwest to the City's Honouliuli Wastewater Treatment Plant (HWWTP). The existing sewer system is shown in **Figure 2-1**.

**2.4 Existing Wastewater Treatment and Disposal**

The HWWTP, located along Geiger in Ewa Beach, treats and disposes all of the wastewater generated by several communities in the Honouliuli sewer basin, including Royal Kunia. The locations of the HWWTP and surrounding WWPS are shown in **Figure 2-2**. Per the City and County of Honolulu’s “Central Oahu Sustainable Communities Plan”, dated March 2021, HWWTP has an existing capacity of 38 million gallons per day (mgd).

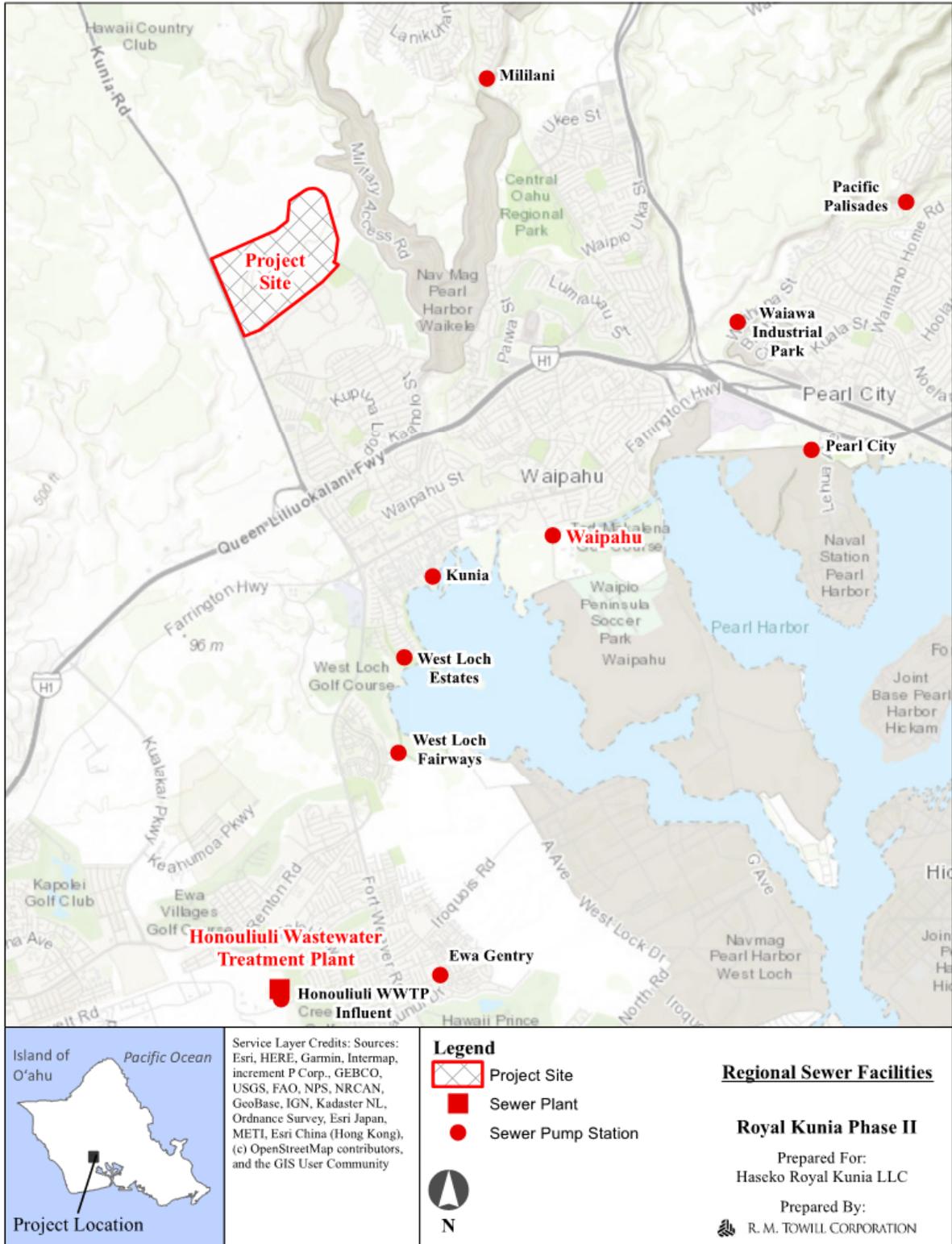
The “Development Plan Land Use Amendment Application and Final Environmental Assessment for Royal Kunia, Phase II Increment 3” prepared by William E. Wanket, Inc., dated May 1996, indicates that wastewater generated by the project “is expected to be accommodated at the treatment facility [HWWTP] after its expansion is completed in 1997. Thus, the project should not result in a significant impact to wastewater facilities serving the area.”

**SECTION 2 - Existing Conditions**



**Figure 2-1 – Existing Sewer**

**SECTION 2 - Existing Conditions**



**Figure 2-2 – Regional Sewer Facilities**

**3. CRITERIA AND METHODOLOGY**

**3.1 Wastewater Flow Criteria**

The Royal Kunia Phase II wastewater flows are based on the proposed development plans for the Royal Kunia Phase II project by Haseko Royo Kunia LLC. Proposed wastewater flow estimates and sewer system design will conform with the City and County of Honolulu, Department of Environmental Services, Wastewater System Design Standards, July 2017.

**Table 3-1** summarizes the wastewater flow criteria used to compute the wastewater design flows.

**Table 3-1: Wastewater Flow Demand Criteria**

<b>Wastewater Generation Rates</b> (for computing average daily flow)	
Single-Family Residential.....	4 persons/home @ 70 gpcd
Multi-Family Residential.....	2.8 persons/unit @ 70 gpcd
Parks (w/Comfort Station).....	5 cpa @ 70 gpcd *
Commercial/Industrial.....	100 cpa @ 70 gpcd
Schools.....	25 gpd per Student

\* Sewer Demand Rates per Royal Kunia Revised Sewer Master Plan dated May 1996

(cpa = capita per acre)

(gpcd = gallon per capita per day)

Base Sanitary Flow (BSF) = Average daily per capita wastewater flow multiplied by equivalent population.

Peak Base Sanitary Flow (PBSF) = BSF x 2.5

Average Dry Weather Flow (ADWF) = BSF + groundwater infiltration rate

Groundwater Infiltration (GWI) = 35 gallons per capita per day (gpcd)

Peak Dry Weather Flow (PDWF) = PBSF + GWI

Wet Weather Infiltration/Inflow (I/I) = 3,000 gallons per acre per day (gpad)

Design Flow (Q<sub>DES</sub>) = PDWF + I/I

**3.2. Wastewater System Design Criteria**

The sewer system will be designed to conform to City’s Wastewater System Design Standards and is proposed to be dedicated to the City. Sizes were determined for Polyvinyl Chloride (PVC) C900 and C905 pipes. The criteria used in this master plan are summarized in **Table 3-2**.

<b>Table 3-2 Gravity Sewer Design Criteria</b>	
<b>Minimum Pipe Slopes</b>	
<u>Pipe Diameter (inches)</u>	<u>Slope (ft/ft)</u>
8 (upstream terminal)	0.0100
8	0.0052
10	0.0039
12	0.0031
16	0.0021
18	0.0018
>18	0.0016
<b>Pipe Flow Velocities</b>	
Velocity (Manning’s Eq.)	$= \frac{(1.486) \cdot (r^{2/3}) \cdot (s^{1/2})}{n}$
Minimum	2.5 ft/sec at full flow
Maximum	10 ft/sec
<b>Minimum Size</b>	
Mains and branch mains: 8-inches	
Lateral sewers: 6-inches	
<b>Pipe Capacity</b>	
Maximum <sup>1</sup>	± 85% of full pipe
<b>Pipe Manning’s “n” Values</b>	
<u>Pipe Diameter</u>	<u>“n” Value</u>
All pipes	0.013

<sup>1</sup> Minimum excess pipe capacity of 15 percent of full flow capacity in consideration of potential future development.

**3.3. Impacts to Wastewater Facilities**

Wastewater flows for the proposed RKII project for full build-out conditions will be calculated to determine the sewer system sizing and to determine the impacts to the existing sewer system.

**4. PROPOSED CONDITIONS**

**4.1 Proposed Development**

The proposed RKII development contains up to 1850 single-family and multi-family residential homes, a minimum of 10 acres of park space, 123.7 acres of commercial and industrial use and 12 acres of school grounds within TMK:9-4-002: 001, 070, 071, 078 and 079.

See **Figure 4-1** for the proposed sewer master plan for RKII. A listing of the planned land uses for this development are provided in **Table 4-1**.

Arterial, collector, sub-collector, and local subdivision roads are intended to be dedicated to the City. All utility infrastructure, roadway prisms, and landscape planting will be designed to meet City standards and the requirements of utility companies.

**SECTION 4 – Proposed Conditions**

<b>Table 4-1: Proposed Land Use</b>				
<b>Land Use</b>	<b>RESIDENTIAL</b>		<b>OTHER</b>	
	<b>Total</b>	<b>Units</b>	<b>Total</b>	<b>Units</b>
<b>Existing RKI Phase 1B (Per Sewer Master Plan)</b>				
Single Family	1200	units		
Multi-Family	648	units		
Private Rec Center/Park			12.1	acres
Public Park			9.5	acres
Commercial (Walmart)			9	acres
Golf Course (Domestic)				
<b>Proposed RKII</b>				
<b>PARCEL A</b>				
Single Family	0	units		
Multi-Family	380	units		
SUBTOTAL PARCEL A	380	units		
<b>PARCEL B</b>				
Single Family	300	units		
Multi-Family	0	units		
SUBTOTAL PARCEL B	300	units		
<b>PARCEL C</b>				
Single Family	325	units		
Multi-Family	0	units		
Parks, Open Space			4.5	acres
SUBTOTAL PARCEL C	325	units		
<b>PARCEL D</b>				
Single Family	260	units		
Multi-Family	64	units		
SUBTOTAL PARCEL D	324	units		
<b>PARCEL E</b>				
Single Family	0	units		
Multi-Family Low Rise (East)	221	units		
Multi-Family Low Rise (West)	300	units		
Parks, Open Space			5.5	acres
SUBTOTAL PARCEL E	521	units		
School			800	students
Commercial/Industrial			123.7	acres
TOTAL RKII	1850	units		
<b>OFFSITE</b>				
DOA Single Family	24	units		
Robinson Kunia Land LLC	<u>1,500</u>	units		
TOTAL OFFSITE	1524	units		

**4.2 Proposed Wastewater Demands and Pipe Sizing**

The RKII project site will generate a wastewater design flow rate of approximately 6.77 MGD. The sewer pipe sizes shown on the proposed sewer master plan are based on a maximum of 85% of full pipe flow capacity. Hydraulic calculations for wastewater flows and pipe sizing are shown in Appendix A.

**4.3 Comparison to the Park Engineering Sewer Master Plan**

This sewer master plan is an update to the RKSMP prepared by Park Engineering Inc., dated August 1996. The RKSMP wastewater quantities are shown in comparison to the current estimates per this master plan in **Table 4-2** below.

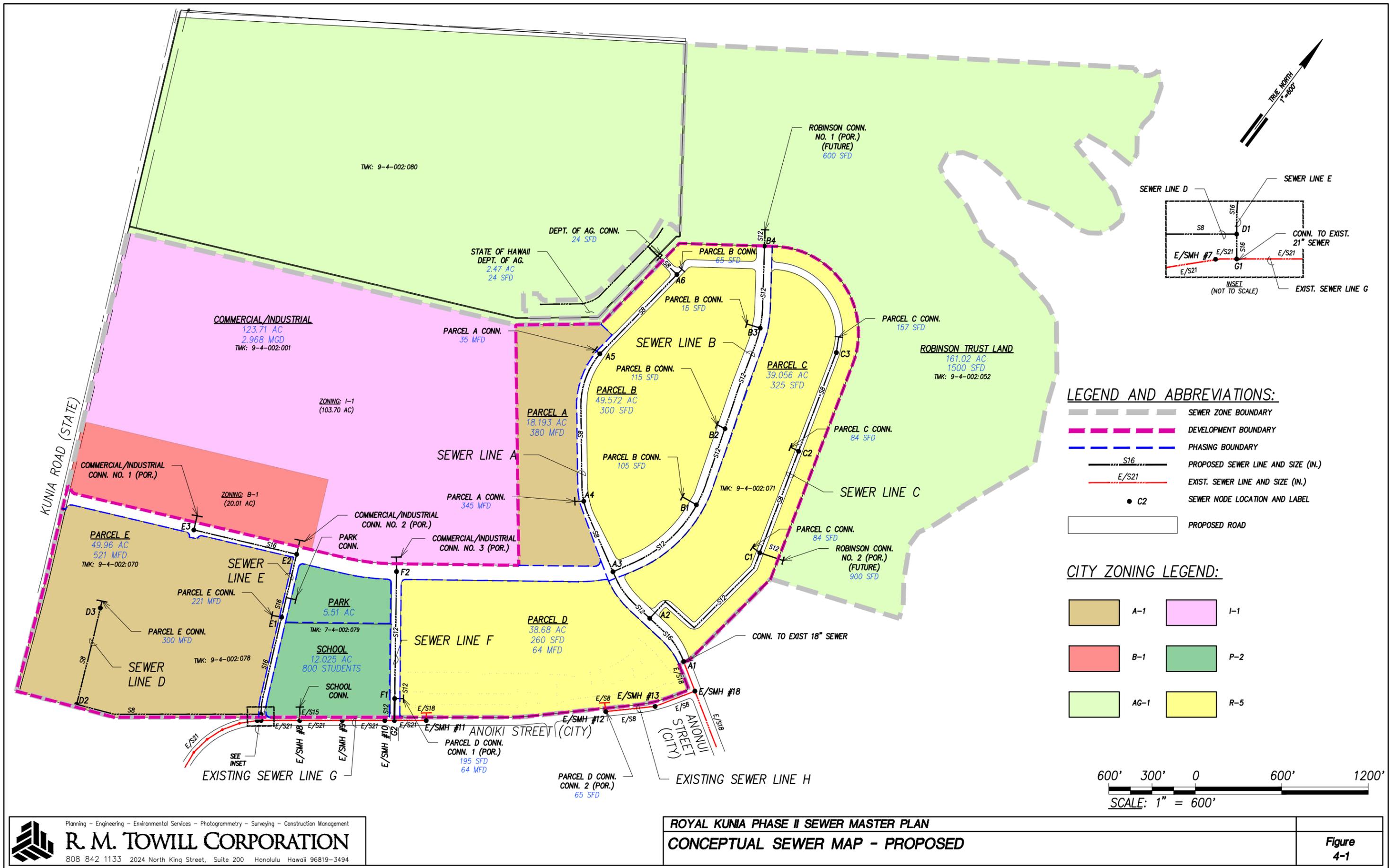
<b>Table 4-2: Sewer Master Plan Comparison</b>		
<b>Existing Sewer Manhole</b>	<b>Park Engineering Sewer Master Plan dated May 1996</b>	<b>R.M. Towill Corp. Sewer Master Plan dated November 2023</b>
SMH #7	4.16 MGD	3.844 MGD
SMH #18	2.95 MGD	2.929 MGD

**4.4 Proposed Wastewater System**

Wastewater flows from RKII will be collected by onsite gravity sewer pipes and conveyed generally south towards RKI. The RKII wastewater system will connect to the existing RKI system at 5 locations: Four along Anoiki Street and one at Anonui Street. The RKII wastewater will be conveyed by the existing RKI regional wastewater system to the Waipahu WWPS and ultimately the HWWTP in conformance with the revised RKSMP and City wastewater standards. The new RKII sewerage system is intended to be dedicated to the City.

*Per the RKSMP dated May 1996, “The remaining sewage from both Royal Kunia Phase I and Phase II will be transported to the Waipahu Sewage Pump Station located on Waipahu Depot Street via a 24” off-site trunk sewer. The new sewer line will serve the proposed Royal Kunia Subdivision, Phases I and II. It will begin within the Royal Kunia Subdivision and terminate at the existing Waipahu Sewage Pump Station on Depot Road. The sewer line design was divided into approximately six separate segments, but all segments was constructed concurrently.”*

The proposed sewer lines to be dedicated to the City shall be located within existing and proposed City right-of-way. Where sewer lines cannot be located within City right-of-way, the sewer line shall be located in the center of easements with the following widths: 15 feet (8-inch to 16-inch diameter pipes, 25 feet (pipes larger than 16-inch diameter). An all-weather access road or gravel road (reviewed and approved by the City on a case-by-case basis) shall be provided along sewer easements for maintenance of the sewer system. The maximum sewer manhole spacing shall be 350 feet for pipes up to and including 36-inch diameter in roadways, 250 feet for pipes up to and including 18-inches in diameter in non-roadway areas, 350 feet for pipes larger than 18-inches and up to and including 36-inches in diameter in non-roadway areas in accordance with city wastewater design standards. Easements shall be designated for both sewer and access purposes. The site ground elevations are high enough that groundwater is not anticipated to be encountered in any of the proposed RKII sewers.



**5. REFERENCES**

1. *Wastewater System Design Standards, Volume 1*, Department of Environmental Services, City and County of Honolulu, July 2017
2. *Royal Kunia Revised Sewer Master Plan*, May 1996, Park Engineering Inc.
3. Central Oahu Sustainable Communities Plan, March 2021, Department of Planning and Permitting, City and county of Honolulu, Hawaii
4. Development Plan Land Use Amendment Application and Final Environmental Assessment for Royal Kunia, Phase II Increment 3

## APPENDICES

## APPENDIX A

### Wastewater Quantities and Hydraulic Computations











APPENDIX B

Royal Kunia Revised Sewer Master Plan

(Prepared by Park Engineering, Inc. dated May 1996)

**ROYAL KUNIA  
REVISED SEWER MASTER PLAN**

AT HOAEAE AND WAIKELE, EWA, OAHU, HAWAII

TAX MAP KEY: 9-4-02: POR 1, 46, 49, 50, 51, & 52

May 1996

PREPARED BY:

ParEn., Inc.

dba PARK ENGINEERING

ENGINEERS, SURVEYORS, PLANNERS

SUITE 300, KAWAIAHAO PLAZA

567 SOUTH KING STREET

HONOLULU, HAWAII 96813

**Royal Kunia**  
**Revised Sewer Master Plan**

TABLE OF CONTENTS:

	<u>Page</u>
Location	3
Topography	3
Sewer Master Plan	3

Bound together with this Report:

- Exhibit A - Project Location Map
- Exhibit B - Royal Kunia Sewer Master Plan Map
- Exhibit C - Royal Kunia Sewer Master Plan Sanitary Sewage Computations
- Exhibit D - Halekua Gardens I and 5.0 Acre Industrial Site Connections to Village Park Map
- Exhibit E - Halekua Gardens I and 5.0 Acre Industrial Site Connections to Village Park Sanitary Sewage Computations

**Royal Kunia**  
**Revised Sewer Master Plan**

**LOCATION:**

The property is located approximately 1 mile north of the Kunia Road - Waipahu Street Intersection. It is bounded by Kunia Road on the west, Waikele Stream on the east, and the existing Village Park subdivision on the south.

**TOPOGRAPHY:**

The property generally slopes toward the southeast at an average slope of 4%. Steeper gradients of 20% and greater can be found in the gully adjacent to Kunia Road.

**SEWER MASTER PLAN:**

This master plan accounts for the ultimate grading and drainage plans for the Royal Kunia Development. The plan, attached as Exhibit B, was developed according to the latest Land Use Map for the Royal Kunia Development. It involves connecting two sites, Halekua Gardens I and 5.0 Acres of an Industrial Site both located at the southwest corner of Royal Kunia Phase I, to the existing sewer system within the Village Park Subdivision. The accompanying plan and computations within Village Park are included as Exhibits D and E respectively. The remaining sewage from both Royal Kunia Phase I and Phase II will be transported to the Waipahu Sewage Pump Station located on Waipahu Depot Road via a 24" off-site trunk sewer. The new sewer line will serve the proposed Royal Kunia Subdivision, Phases I and II. It will begin within the Royal Kunia Subdivision and terminate at the existing Waipahu Sewage Pump Station on Depot Road. The sewer line design was divided into approximately six separate segments, but all segments was constructed concurrently. (See Exhibit A : Project Location Map.)

The design criteria for the project was extracted from the Design Standards of the Division of Wastewater Management, Volume 1, July, 1993. The criteria are as follows:

1. Approximate unit projections and developable acreages were obtained from a preliminary Land Use Map (Exhibit B) and used to determine flows in the sewer main.
2. Average daily per capita flow = 80 gal/capita/day.

Royal Kunia  
Revised Sewer Master Plan

3. Density:
  - a. Single Family Dwelling (SFD) - 4 persons/unit
  - b. Low Density Apartment Occupancy (LDA)
    - 85 capita/acre for areas where the number of units are unknown.
    - 2.8 capita/unit for areas where the number of units are known.
  - c. Industrial - 100 capita/acre
  - d. School - 1200 students
  - e. Park/Ride - negligible
  - f. Parks - 5 capita/acre
  - g. Golf Course Club House - 1.0 Acre Club House @ 250 capita/acre
4. Factor for maximum rate of dry weather sewage flow is extracted from Babbitt's curve:

$$\text{Maximum Factor} = \frac{5}{P^{0.2}}$$

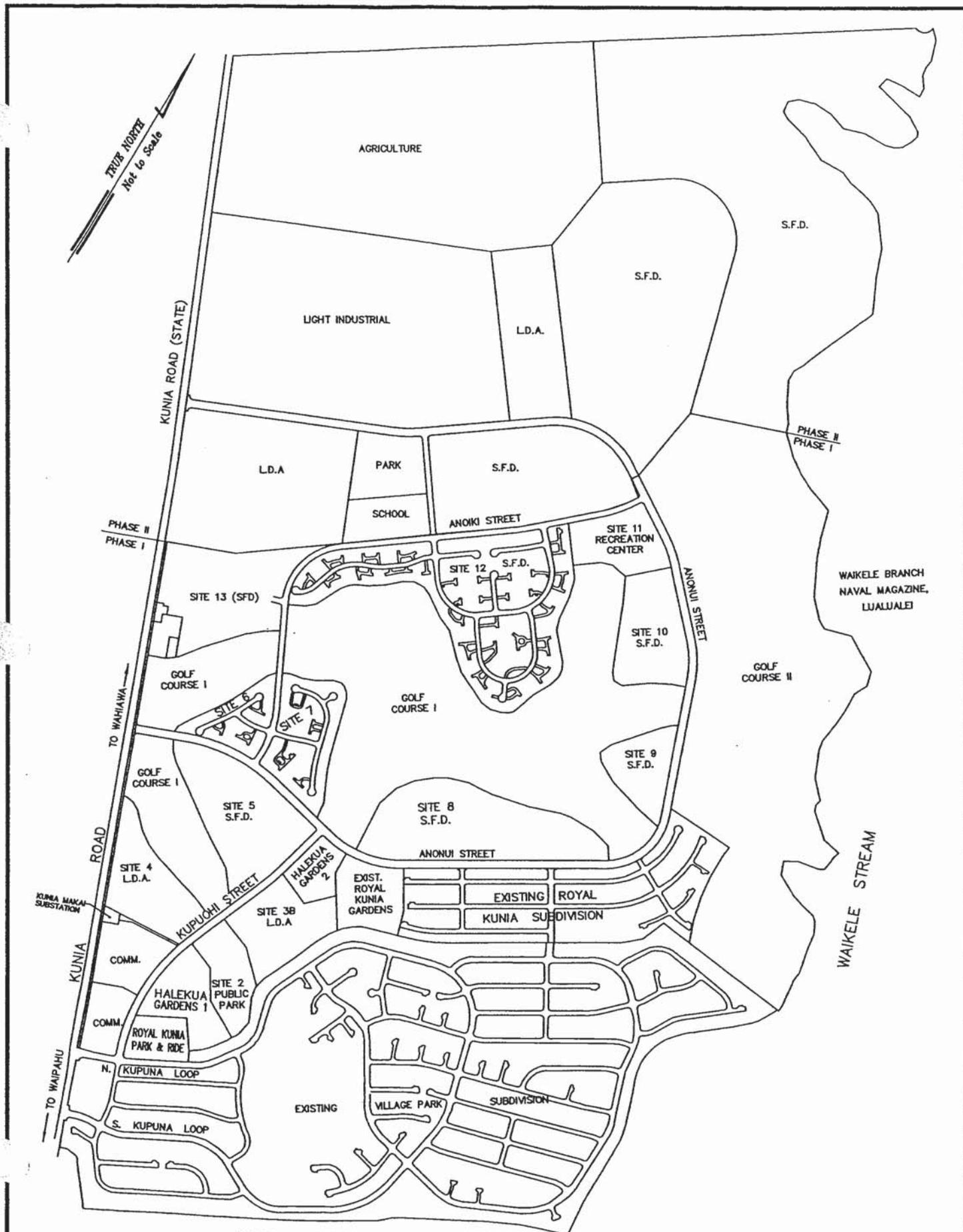
where P = population in thousands

5. Normal Infiltration = 5 gal/capita/day  
Wet Infiltration = 1,250 gal/acre/day
6. Peak Flow = summation of Maximum flow plus Infiltration.
7. Manning's formula was used as the basis for the hydraulic design of the sewer:

$$Q = \frac{1.486}{n} A R^{2/3} S^{1/2}$$

where n = roughness coefficient  
R = hydraulic radius  
A = area of pipe flow  
S = slope in ft/ft

8. Minimum Velocity = 2.0 ft/sec  
Maximum Velocity = 10.0 ft/sec



TRUE NORTH  
Not to Scale

INTERSTATE H-1

EXHIBIT A - LOCATION MAP

TRUE NORTH  
Scale: 1" = 400'

KUNIA ROAD (STATE)

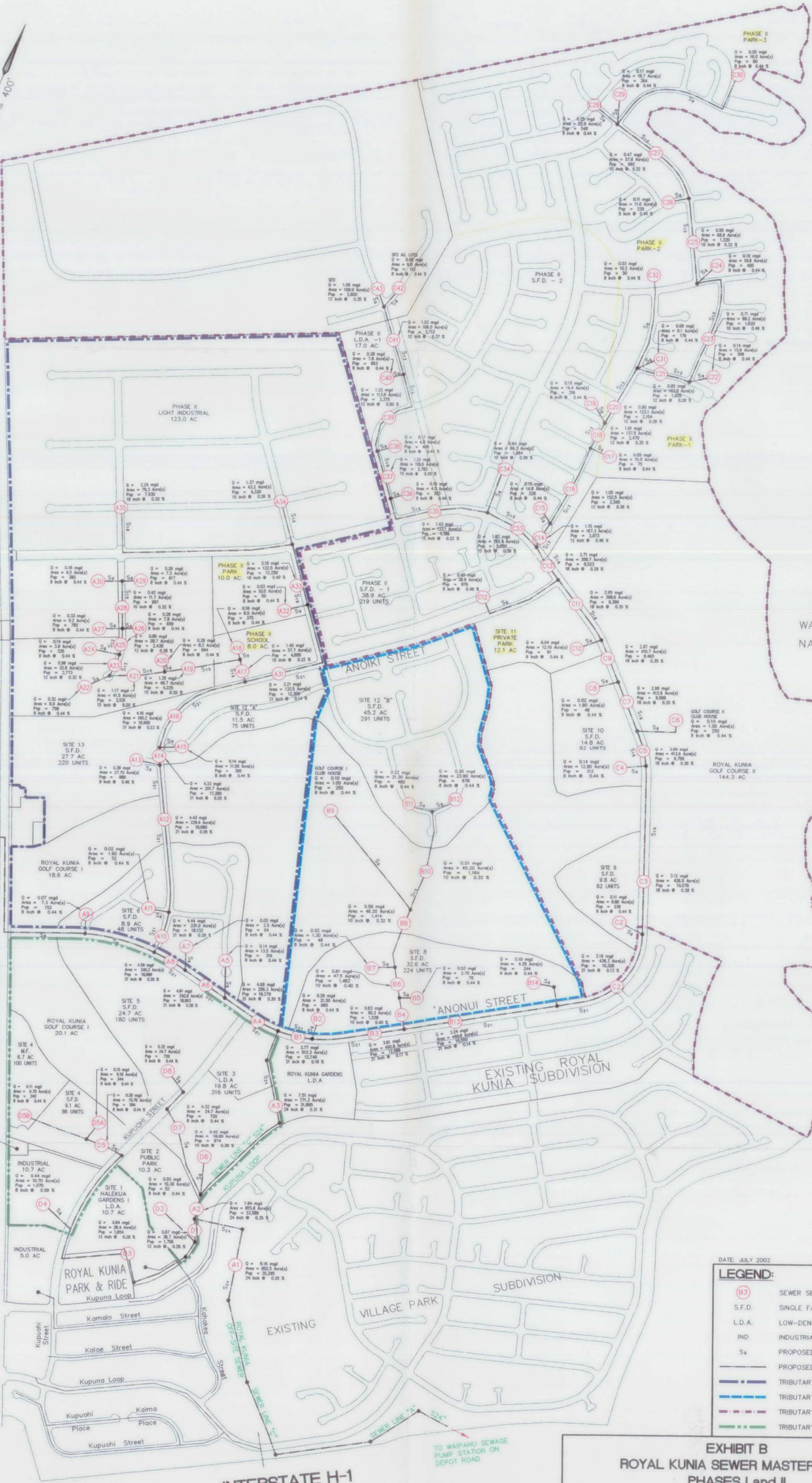
TO WAHIWA

TO WAIPAHA

WAIKELE BRANCH  
NAVAL MAGAZINE,  
LUALUALEI

WAIKELE STREAM

**NOTE:**  
Halekua Gardens I (Site 1) and the 5.0 Acre Industrial site sewer systems will both connect to the existing sewer system within Village Park Subdivision. See Exhibits D and E of this report for schematic connections and Sanitary Sewage Flow Computations.



DATE: JULY 2002

TSW

**LEGEND:**

- (B3) SEWER SEGMENT NUMBER
- S.F.D. SINGLE FAMILY DWELLING
- L.D.A. LOW-DENSITY APARTMENT
- IND INDUSTRIAL SITE
- Sa PROPOSED 8" SEWER
- PROPOSED TRUNK SEWER
- TRIBUTARY AREA "A"
- TRIBUTARY AREA "B"
- TRIBUTARY AREA "C"
- TRIBUTARY AREA "D"

**EXHIBIT B  
ROYAL KUNIA SEWER MASTER PLAN  
PHASES I and II  
TMK : 9-4-02**

SEWER: ROYAL KUNIA SEWER MASTER PLAN (SITE 8PDH SUBDIVISION)  
 DISTRICT: WAIPAHAU  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "C"

BY: TSS

Sheet: RKIIACT-SMP(SITE8)

Normal Infiltr: 5 (gpcd) [USE 5 OR 35]  
 Wet Infiltr: 1250 (gpad) [USE 1250 OR 2750]

DIST, ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
POR SITE 12 "A" A15 + A16	A15	11.5	11.5	300	300	80	0.0240	5.00	0.1200	0.0015	0.0255	0.1215	0.0144	0.1359	8	0.44	0.45	1.99	0.30
	A14	201.7	201.7	17,200	17,200	80	1.3760	2.83	3.8948	0.0860	1.4620	3.9808	0.2521	4.2330	21	0.25	5.11	3.29	0.83
SITE 13 A13 + A14 =	A13	27.7	27.7	880	880	80	0.0704	5.00	0.3520	0.0044	0.0748	0.3564	0.0346	0.3910	8	0.46	0.46	2.04	0.85
	A12	229.4	229.4	18,080	18,080	80	1.4464	2.80	4.0535	0.0904	1.5368	4.1439	0.2867	4.4306	21	0.28	5.41	3.48	0.82
POR SITE 6 A11 + A12 =	A11	1.6	1.6	52	52	80	0.0042	5.00	0.0208	0.0003	0.0044	0.0211	0.0020	0.0231	8	0.44	0.45	1.99	0.05
	A10	231.0	231.0	18,132	18,132	80	1.4506	2.80	4.0628	0.0907	1.5412	4.1534	0.2887	4.4422	21	0.28	5.41	3.48	0.82
BWS NITRATE REMOVAL POR SITE 6 A9A + A9 + A10 =	A9A	2.0	2.0	615	615	80	0.0492	5.00	0.2460	0.0031	0.0523	0.2491	0.0025	0.2516	8	0.44	0.45	1.99	0.56
	A9	7.3	7.3	152	152	80	0.0122	5.00	0.0608	0.0008	0.0129	0.0616	0.0091	0.0707	8	0.44	0.45	1.99	0.16
	A8	240.3	240.3	18,899	18,899	80	1.5119	2.78	4.1997	0.0945	1.6064	4.2942	0.3004	4.5946	21	0.28	5.41	3.48	0.85

SEWER: ROYAL KUNIA SEWER MASTER PLAN (SITE 8PDH SUBDIVISION)  
 DISTRICT: WAIPAHU  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "C"

BY: TSS

Sheet: RKIIACT-SMP(SITE8)

Normal Infiltr: 5 (gpcd) [USE 5 OR 35]  
 Wet Infiltr: 1250 (gpad) [USE 1250 OR 2750]

DIST, ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
POR SITE 7	A7	2.5	2.5	64	64	80	0.0051	5.00	0.0256	0.0003	0.0054	0.0259	0.0031	0.0290	8	0.44	0.45	1.99	0.06
	A7 + A8 = A6	242.8	242.8	18,963	18,963	80	1.5171	2.78	4.2111	0.0948	1.6119	4.3059	0.3035	4.6094	21	0.28	5.41	3.48	0.85
POR SITE 7	A5	13.5	13.5	316	316	80	0.0253	5.00	0.1264	0.0016	0.0269	0.1280	0.0169	0.1449	8	0.44	0.45	1.99	0.32
	A5 + A6 = A4	256.3	256.3	19,279	19,279	80	1.5423	2.77	4.2671	0.0964	1.6387	4.3635	0.3204	4.6839	21	0.30	5.60	3.60	0.84
A4 + B1 = A3		771.3		31,895		80	2.5516	2.50	6.3832	0.1595	2.7111	6.5426	0.9642	7.5068	24	0.31	8.12	4.00	0.92
A3 + D6 + D7 = A2		815.8		33,589		80	2.6871	2.48	6.6530	0.1679	2.8551	6.8210	1.0198	7.8408	24	0.35	8.63	4.25	0.91
A2 + D1 = A1		852.5		35,295		80	2.8236	2.45	6.9220	0.1765	3.0001	7.0985	1.0657	8.1641	24	0.35	8.65	4.26	0.94

SEWER: ROYAL KUNIA SEWER MASTER PLAN (SITE 8PDH SUBDIVISION)  
 DISTRICT: WAIPAHU  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "C"

BY: TSS

Sheet: RKIIACT-SMP(SITE8)

Normal Infiltr: 5 (gpcd) [USE 5 OR 35]  
 Wet Infiltr: 1250 (gpad) [USE 1250 OR 2750]

DIST, ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
<b>TRIBUTARY AREA "B"</b>																			
POR SITE 12 "B"	B12	23.9	23.9	676	676	80	0.0541	5.00	0.2704	0.0034	0.0575	0.2738	0.0299	0.3037	8	0.44	0.45	1.99	0.67
POR SITE 12 "B"	B11	21.3	21.3	488	488	80	0.0390	5.00	0.1952	0.0024	0.0415	0.1976	0.0267	0.2243	8	0.44	0.45	1.99	0.50
B11 + B12 =	B10	45.2	45.2	1,164	1,164	80	0.0931	4.85	0.4517	0.0058	0.0989	0.4575	0.0565	0.5140	10	0.32	0.70	1.97	0.74
GC I CLUB HOUSE	B9	1.0	1.0	250	250	80	0.0200	5.00	0.1000	0.0013	0.0213	0.1013	0.0013	0.1025	8	0.44	0.45	1.99	0.23
B9 + B10 =	B8	46.2	46.2	1,414	1,414	80	0.1131	4.67	0.5277	0.0071	0.1202	0.5348	0.0578	0.5926	10	0.32	0.70	1.97	0.85
POR SITE 8 (MIDDLE - 1)	B7	1.9	1.9	60	60	80	0.0048	5.00	0.0240	0.0003	0.0051	0.0243	0.0023	0.0266	8	0.44	0.45	1.99	0.06
B7+B8	B6	48.1	48.1	1,474	1,474	80	0.1179	4.63	0.5456	0.0074	0.1253	0.5529	0.0601	0.6130	10	0.40	0.78	2.21	0.79
POR SITE 8 (MIDDLE - 2)	B5	2.8	2.8	68	68	80	0.0054	5.00	0.0272	0.0003	0.0058	0.0275	0.0035	0.0310	8	0.44	0.45	1.99	0.07
B5+B6	B4	50.9	50.9	1,542	1,542	80	0.1234	4.59	0.5656	0.0077	0.1311	0.5733	0.0636	0.6369	10	0.40	0.78	2.21	0.82
POR SITE 8 (RIGHT)	B14	4.5	4.5	112	112	80	0.0090	5.00	0.0448	0.0006	0.0095	0.0454	0.0056	0.0509	8	0.44	0.45	1.99	0.11
TRIBUTARY AREA "C"	C1	436.3	436.3	10,306	10,306	80	0.8244	3.14	2.5853	0.0515	0.8760	2.6369	0.5454	3.1823	21	0.13	3.68	2.37	0.86
B14 + C1 =	B13	440.8	440.8	10,418	10,418	80	0.8334	3.13	2.6078	0.0521	0.8855	2.6599	0.5510	3.2108	21	0.14	3.82	2.46	0.84
B4 + B13 =	B3	491.6	491.6	11,960	11,960	80	0.9568	3.04	2.9123	0.0598	1.0166	2.9721	0.6145	3.5866	21	0.17	4.21	2.71	0.85
POR SITE 8 (LEFT)	B2	23.4	23.4	656	656	80	0.0525	5.00	0.2624	0.0033	0.0558	0.2657	0.0293	0.2949	8	0.44	0.45	1.99	0.66
B2 + B3 =	B1	515.0	515.0	12,616	12,616	80	1.0092	3.01	3.0394	0.0631	1.0723	3.1024	0.6438	3.7462	21	0.18	4.33	2.79	0.86

SEWER: ROYAL KUNIA SEWER MASTER PLAN (SITE 8PDH SUBDIVISION)  
 DISTRICT: WAIPAHAU  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "C"

BY: TSS

Sheet: RKIIACT-SMP(SITE8)

Normal Infiltr: 5 (gpcd) [USE 5 OR 35]  
 Wet Infiltr: 1250 (gpad) [USE 1250 OR 2750]

DIST, ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
<b>TRIBUTARY AREA "C"</b>																			
SFD C, PH II AG. LOTS C42 + C43	C43	100.0	100.0	2,600	2,600	80	0.2080	4.13	0.8591	0.0130	0.2210	0.8721	0.1250	0.9971	12	0.35	1.18	2.33	0.84
	C42	6.0	6.0	112	112	80	0.0090	5.00	0.0448	0.0006	0.0095	0.0454	0.0075	0.0529	8	0.44	0.45	1.99	0.12
	C41	106.0	106.0	2,712	2,712	80	0.2170	4.10	0.8886	0.0136	0.2305	0.9021	0.1325	1.0346	12	0.37	1.22	2.40	0.85
LDA - 2 (C) C40 + C41	C40	7.8	7.8	663	663	80	0.0530	5.00	0.2652	0.0033	0.0564	0.2685	0.0098	0.2783	8	0.44	0.45	1.99	0.62
	C39	113.8	113.8	3,375	3,375	80	0.2700	3.92	1.0585	0.0169	0.2869	1.0753	0.1423	1.2176	12	0.50	1.41	2.79	0.86
LDA - 2 (B) C38 + C39	C38	4.8	4.8	408	408	80	0.0326	5.00	0.1632	0.0020	0.0347	0.1652	0.0060	0.1712	8	0.44	0.45	1.99	0.38
	C37	118.6	118.6	3,783	3,783	80	0.3026	3.83	1.1597	0.0189	0.3216	1.1786	0.1483	1.3268	15	0.20	1.62	2.04	0.82
LDA - 2 (A) C36 + C37	C36	4.5	4.5	383	383	80	0.0306	5.00	0.1530	0.0019	0.0325	0.1549	0.0056	0.1605	8	0.44	0.45	1.99	0.36
	C35	123.1	123.1	4,166	4,166	80	0.3332	3.76	1.2525	0.0208	0.3541	1.2734	0.1539	1.4272	15	0.22	1.70	2.14	0.84
SFD - 2 (I) C34 + C35	C34	69.3	69.3	1,484	1,484	80	0.1187	4.62	0.5485	0.0074	0.1261	0.5560	0.0866	0.6426	10	0.38	0.76	2.15	0.85
	C33	192.4	192.4	5,650	5,650	80	0.4520	3.54	1.5983	0.0282	0.4802	1.6266	0.2405	1.8671	15	0.38	2.24	2.82	0.84
PARK - 3 SFD - 2(H) SFD - 2(G) C28 + C29 + C30	C30	16.0	16.0	80	80	80	0.0064	5.00	0.0320	0.0004	0.0068	0.0324	0.0200	0.0524	8	0.44	0.45	1.99	0.12
	C29	18.7	18.7	364	364	80	0.0291	5.00	0.1456	0.0018	0.0309	0.1474	0.0234	0.1708	8	0.44	0.45	1.99	0.38
	C28	22.9	22.9	548	548	80	0.0438	5.00	0.2192	0.0027	0.0466	0.2219	0.0286	0.2506	8	0.44	0.45	1.99	0.56
	C27	57.6	57.6	992	992	80	0.0794	5.00	0.3968	0.0050	0.0843	0.4018	0.0720	0.4738	10	0.32	0.70	1.97	0.68
SFD - 2(F) C26 + C27	C26	11.0	11.0	228	228	80	0.0182	5.00	0.0912	0.0011	0.0194	0.0923	0.0138	0.1061	8	0.44	0.45	1.99	0.24
	C25	68.6	68.6	1,220	1,220	80	0.0976	4.81	0.4690	0.0061	0.1037	0.4751	0.0858	0.5608	10	0.32	0.70	1.97	0.81
SFD - 2(E) C24 + C25	C24	19.6	19.6	400	400	80	0.0320	5.00	0.1600	0.0020	0.0340	0.1620	0.0245	0.1865	8	0.44	0.45	1.99	0.41
	C23	88.2	88.2	1,620	1,620	80	0.1296	4.54	0.5884	0.0081	0.1377	0.5965	0.1103	0.7067	10	0.46	0.83	2.37	0.85
SFD - 2(D) C22 + C23	C22	15.6	15.6	308	308	80	0.0246	5.00	0.1232	0.0015	0.0262	0.1247	0.0195	0.1442	8	0.44	0.45	1.99	0.32
	C21	103.8	103.8	1,928	1,928	80	0.1542	4.38	0.6763	0.0096	0.1639	0.6859	0.1298	0.8157	12	0.28	1.06	2.08	0.77
PARK - 2 SFD - 2(C) C21 + C31 + C32	C32	10.2	10.2	50	50	80	0.0040	5.00	0.0200	0.0003	0.0043	0.0203	0.0128	0.0330	8	0.44	0.45	1.99	0.07
	C31	9.1	9.1	176	176	80	0.0141	5.00	0.0704	0.0009	0.0150	0.0713	0.0114	0.0827	8	0.44	0.45	1.99	0.18
	C20	123.1	123.1	2,154	2,154	80	0.1723	4.29	0.7390	0.0108	0.1831	0.7498	0.1539	0.9037	12	0.28	1.06	2.08	0.85
SFD - 2(B) C19 + C20	C19	14.4	14.4	316	316	80	0.0253	5.00	0.1264	0.0016	0.0269	0.1280	0.0180	0.1460	8	0.44	0.45	1.99	0.32
	C18	137.5	137.5	2,470	2,470	80	0.1976	4.17	0.8246	0.0124	0.2100	0.8369	0.1719	1.0088	12	0.35	1.18	2.33	0.85
PARK - 1 C17 + C18	C17	15.0	15.0	75	75	80	0.0060	5.00	0.0300	0.0004	0.0064	0.0304	0.0188	0.0491	8	0.44	0.45	1.99	0.11
	C16	152.5	152.5	2,545	2,545	80	0.2036	4.15	0.8445	0.0127	0.2163	0.8572	0.1907	1.0479	12	0.38	1.23	2.43	0.85

SEWER: ROYAL KUNIA SEWER MASTER PLAN (SITE 8PDH SUBDIVISION)  
 DISTRICT: WAIPAHAU  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "C"

BY: TSS

Sheet: RKIIACT-SMP(SITE8)

Normal Infiltr: 5 (gpcd) [USE 5 OR 35]  
 Wet Infiltr: 1250 (gpad) [USE 1250 OR 2750]

DIST, ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
SFD - 2 (A) C15 + C16	C15	14.8	14.8	328	328	80	0.0262	5.00	0.1312	0.0016	0.0279	0.1328	0.0185	0.1513	8	0.44	0.45	1.99	0.34
	C14		167.3		2,873	80	0.2298	4.05	0.9305	0.0144	0.2442	0.9449	0.2092	1.1540	12	0.46	1.36	2.67	0.85
C14 + C33	C13		359.7		8,523	80	0.6818	3.26	2.2208	0.0426	0.7244	2.2634	0.4497	2.7131	18	0.29	3.18	2.78	0.85
SFD - 1 C12 + C13 =	C12	38.9	38.9	876	876	80	0.0701	5.00	0.3504	0.0044	0.0745	0.3548	0.0486	0.4034	8	0.49	0.47	2.10	0.85
	C11		398.6		9,399	80	0.7519	3.19	2.4016	0.0470	0.7989	2.4486	0.4983	2.9469	18	0.35	3.49	3.05	0.84
PRIVATE PARK C10 + C11 =	C10	12.1	12.1	61	61	80	0.0049	5.00	0.0244	0.0003	0.0052	0.0247	0.0151	0.0398	8	0.44	0.45	1.99	0.09
	C9		410.7		9,460	80	0.7568	3.19	2.4141	0.0473	0.8041	2.4614	0.5134	2.9748	18	0.35	3.49	3.05	0.85
SITE 10 (NORTH) C8 + C9 =	C8	1.9	1.9	48	48	80	0.0038	5.00	0.0192	0.0002	0.0041	0.0194	0.0024	0.0218	8	0.44	0.45	1.99	0.05
	C7		412.6		9,508	80	0.7606	3.19	2.4239	0.0475	0.8081	2.4714	0.5158	2.9872	18	0.35	3.49	3.05	0.86
GC II CLUB HOUSE C6 + C7 =	C6	1.0	1.0	250	250	80	0.0200	5.00	0.1000	0.0013	0.0213	0.1013	0.0013	0.1025	8	0.44	0.45	1.99	0.23
	C5		413.6		9,758	80	0.7806	3.17	2.4747	0.0488	0.8294	2.5235	0.5170	3.0406	18	0.36	3.54	3.10	0.86
SITE 10 (SOUTH) C4 + C5 =	C4	12.9	12.9	312	312	80	0.0250	5.00	0.1248	0.0016	0.0265	0.1264	0.0161	0.1425	8	0.44	0.45	1.99	0.32
	C3		426.5		10,070	80	0.8056	3.15	2.5379	0.0503	0.8559	2.5882	0.5332	3.1213	18	0.38	3.64	3.18	0.86
SITE 9 C2 + C3 =	C2	9.8	9.8	236	236	80	0.0189	5.00	0.0944	0.0012	0.0201	0.0956	0.0123	0.1078	8	0.44	0.45	1.99	0.24
	C1		436.3		10,306	80	0.8244	3.14	2.5853	0.0515	0.8760	2.6369	0.5454	3.1823	21	0.13	3.68	2.37	0.86

SEWER: ROYAL KUNIA SEWER MASTER PLAN (SITE 8PDH SUBDIVISION)  
 DISTRICT: WAIPAHU  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "C"

BY: TSS

Sheet: RKIIACT-SMP(SITE8)

Normal Infiltr: 5 (gpcd) [USE 5 OR 35]  
 Wet Infiltr: 1250 (gpad) [USE 1250 OR 2750]

DIST, ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
TRIBUTARY AREA "D"																			
SITE 5	D8	24.7	24.7	720	720	80	0.0576	5.00	0.2880	0.0036	0.0612	0.2916	0.0309	0.3225	8	0.44	0.45	1.99	0.72
	D7		24.7		720	80	0.0576	5.00	0.2880	0.0036	0.0612	0.2916	0.0309	0.3225	8	0.44	0.45	1.99	0.72
HALEKUA GDNS II	D6	19.8	19.8	974	974	80	0.0780	5.00	0.3898	0.0049	0.0828	0.3946	0.0248	0.4194	8	0.53	0.49	2.19	0.85
SITE 4A (CPR)	D5A	9.1	9.1	344	344	80	0.0275	5.00	0.1376	0.0017	0.0292	0.1393	0.0113	0.1506	8	0.44	0.45	1.99	0.33
SITE 4B (CPR)	D5B	6.7	6.7	240	240	80	0.0192	5.00	0.0960	0.0012	0.0204	0.0972	0.0083	0.1055	8	0.44	0.45	1.99	0.23
	D5A + D5B		15.7		584	80	0.0467	5.00	0.2336	0.0029	0.0496	0.2365	0.0196	0.2561	8	0.44	0.45	1.99	0.57
INDUSTRIAL	D4	10.7	10.7	1,070	1,070	80	0.0856	4.93	0.4222	0.0054	0.0910	0.4276	0.0134	0.4410	8	0.59	0.52	2.31	0.85
	D4 + D5 =		26.4		1,654	80	0.1323	4.52	0.5983	0.0083	0.1406	0.6065	0.0330	0.6395	12	0.28	1.06	2.08	0.80
PUBLIC PARK	D2	10.3	10.3	52	52	80	0.0042	5.00	0.0208	0.0003	0.0044	0.0211	0.0129	0.0339	8	0.44	0.45	1.99	0.08
	D2 + D3 =		36.7		1,706	80	0.1365	4.49	0.6133	0.0085	0.1450	0.6218	0.0459	0.6677	12	0.28	1.06	2.08	0.63

TRUE NORTH  
NO SCALE

SITE 5  
S.F.D.  
24.7 AC  
142 UNITS

GOLF COURSE I  
20.1 AC

SITE 4  
L.D.A.  
17.3 AC  
277 UNITS

SITE 3  
HALEKUA GARDENS II  
L.D.A.  
19.8 AC

SITE 2  
PUBLIC PARK  
10.3 AC

SITE 1  
HALEKUA GARDENS I  
L.D.A.  
10.7 AC  
152 UNITS

INDUSTRIAL  
10.7 AC

ROYAL KUNIA INDUSTRIAL  
5.0 AC

ROYAL KUNIA PARK & RIDE  
5.7 AC

VILLAGE PARK INDUSTRIAL  
3.7 AC

VILLAGE PARK INDUSTRIAL  
1.2 AC

KAUWEKE PLACE

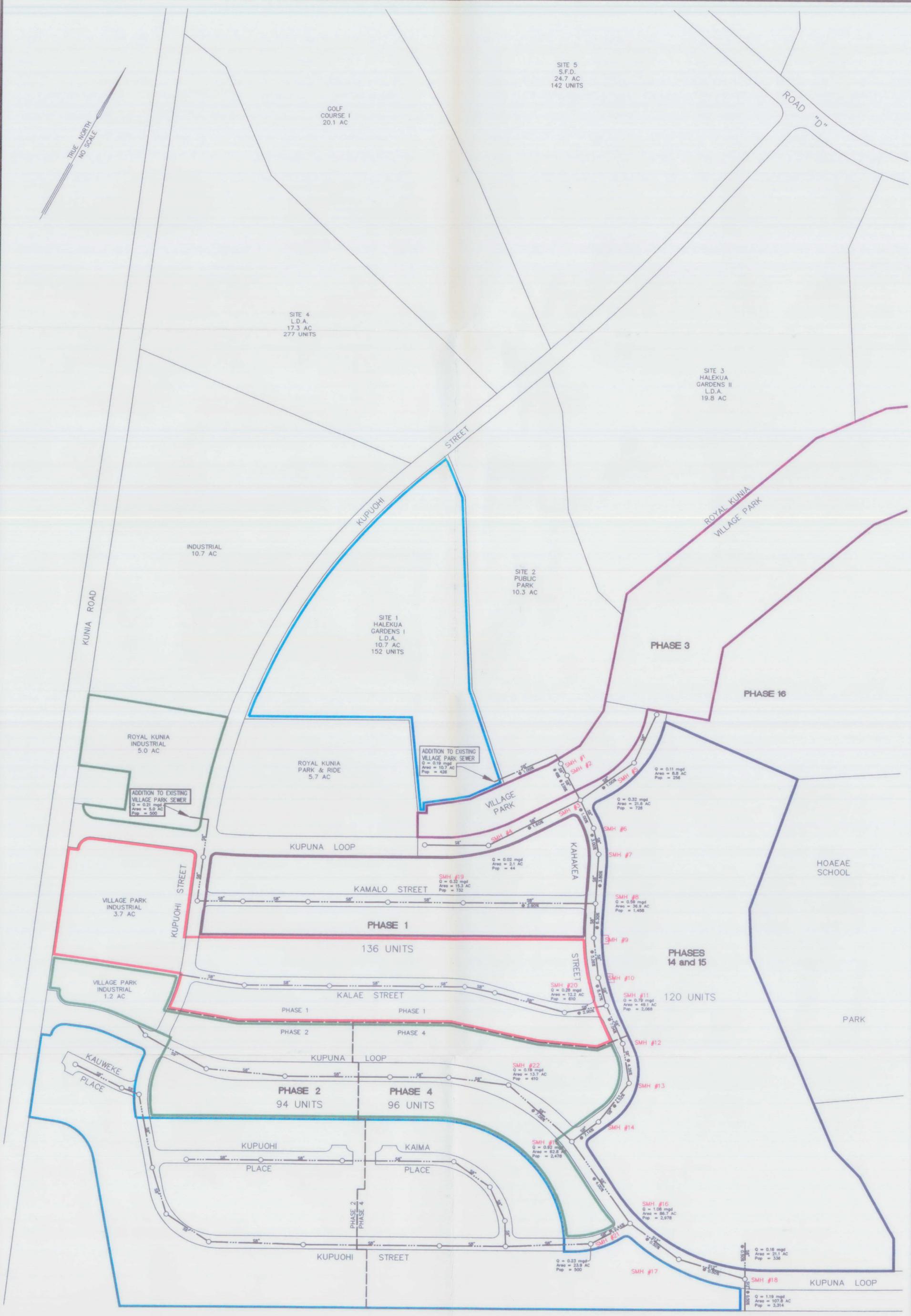
PHASE 2  
94 UNITS

PHASE 4  
96 UNITS

INTERSTATE H-1

TO HONOLULU

EXHIBIT D  
HALEKUA GARDENS I and 5.0 Acre INDUSTRIAL  
CONNECTIONS TO VILLAGE PARK  
TMK : 9-4-02



SEWER: HALEKUA GARDENS I & 5.0 AC INDUSTRIAL CONNECTIONS TO VILLAGE PARK  
 DISTRICT: WAIPAHI  
 REFERENCE MAPS: TMK : 9-4-02

EXHIBIT "E"

BY: TSS  
 FILE: VPIISMP.XLS

Normal Infil: 5 (gpcd) [USE 5 OR 35]  
 Wet Infil: 1250 (gpad) [USE 1250 OR 2750]

DIST. ZONE, OR ST	POINT OR SEGMENT	TRIB. AREA		TRIB. POP.		FLOW gpcd	AVE. FLOW (mgd)	MAX FLOW FACT.	MAX FLOW (mgd)	NORM DESIGN AVE		DESIGN HRLY FLOW (mgd)	WET INFILT. (mgd)	DESIGN PEAK FLOW (mgd)	Pipe Size (IN)	Slope (%)	Capacity (mgd)	Vel. (fps)	Qreq /Qall (%)
		Increment	Total	Increment	Total					INFIL (mgd)	FLOW (mgd)								
HALEKUA I TO SMH #1		10.7	10.7	426	426	80	0.0341	5.00	0.1704	0.0021	0.0362	0.1725	0.0134	0.1859	8	1.00	0.68	3.01	0.27
SMH #1 to SMH #2			10.7		426	80	0.0341	5.00	0.1704	0.0021	0.0362	0.1725	0.0134	0.1859	8	40.00	4.29	19.01	0.04
SMH #2 to SMH #3			10.7		426	80	0.0341	5.00	0.1704	0.0021	0.0362	0.1725	0.0134	0.1859	8	2.00	0.96	4.25	0.19
SMH #3 to SMH #4		2.1	2.1	44	44	80	0.0035	5.00	0.0176	0.0002	0.0037	0.0178	0.0026	0.0204	8	1.60	0.86	3.80	0.02
SMH #3 to SMH #5		8.8	8.8	256	256	80	0.0205	5.00	0.1024	0.0013	0.0218	0.1037	0.0110	0.1147	8	1.00	0.68	3.01	0.17
SMH #3 to SMH #6			21.6		726	80	0.0581	5.00	0.2904	0.0036	0.0617	0.2940	0.0270	0.3210	8	1.00	0.68	3.01	0.47
SMH #6 to SMH #7			21.6		726	80	0.0581	5.00	0.2904	0.0036	0.0617	0.2940	0.0270	0.3210	8	3.00	1.18	5.21	0.27
SMH #7 to SMH #8			21.6		726	80	0.0581	5.00	0.2904	0.0036	0.0617	0.2940	0.0270	0.3210	8	3.80	1.32	5.86	0.24
SMH #8 to SMH #19		15.3	15.3	732	732	80	0.0586	5.00	0.2928	0.0037	0.0622	0.2965	0.0191	0.3156	8	2.90	1.16	5.12	0.27
SMH #8 to SMH #9			36.9		1,458	80	0.1166	4.64	0.5408	0.0073	0.1239	0.5481	0.0461	0.5943	8	6.30	1.70	7.55	0.35
SMH #9 to SMH #10			36.9		1,458	80	0.1166	4.64	0.5408	0.0073	0.1239	0.5481	0.0461	0.5943	8	5.26	1.56	6.90	0.38
SMH #10 to SMH #11			36.9		1,458	80	0.1166	4.64	0.5408	0.0073	0.1239	0.5481	0.0461	0.5943	8	8.47	1.97	8.75	0.30
SMH #11 to SMH #20		12.2	12.2	610	610	80	0.0488	5.00	0.2440	0.0031	0.0519	0.2471	0.0153	0.2623	8	2.90	1.16	5.12	0.23
SMH #11 to SMH #12			49.1		2,068	80	0.1654	4.32	0.7153	0.0103	0.1758	0.7257	0.0614	0.7870	8	7.04	1.80	7.98	0.44
SMH #12 to SMH #13			49.1		2,068	80	0.1654	4.32	0.7153	0.0103	0.1758	0.7257	0.0614	0.7870	8	4.26	1.40	6.21	0.56
SMH #13 to SMH #14			49.1		2,068	80	0.1654	4.32	0.7153	0.0103	0.1758	0.7257	0.0614	0.7870	8	4.52	1.44	6.39	0.55
SMH #14 to SMH #15			49.1		2,068	80	0.1654	4.32	0.7153	0.0103	0.1758	0.7257	0.0614	0.7870	8	7.14	1.81	8.03	0.43
SMH #15 to SMH #22		13.7	13.7	410	410	80	0.0328	5.00	0.1640	0.0021	0.0349	0.1661	0.0171	0.1832	8	7.08	1.81	8.00	0.10
SMH #15 to SMH #16			62.8		2,478	80	0.1982	4.17	0.8267	0.0124	0.2106	0.8391	0.0785	0.9176	8	4.00	1.36	6.01	0.68
SMH #16 to SMH #21		23.9	23.9	500	500	80	0.0400	5.00	0.2000	0.0025	0.0425	0.2025	0.0299	0.2324	8	0.44	0.45	1.99	0.52
SMH #16 to SMH #17			86.7		2,978	80	0.2382	4.02	0.9576	0.0149	0.2531	0.9725	0.1084	1.0809	12	0.50	1.41	2.79	0.76
SMH #17 to SMH #18			86.7		2,978	80	0.2382	4.02	0.9576	0.0149	0.2531	0.9725	0.1084	1.0809	12	0.50	1.41	2.79	0.76
VP 14-15 to SMH #18		21.1	21.1	336	336	80	0.0269	5.00	0.1344	0.0017	0.0286	0.1361	0.0264	0.1625	8	0.50	0.48	2.13	0.34
SMH #18 to OUTLET			107.8		3,314	80	0.2651	3.93	1.0431	0.0166	0.2817	1.0597	0.1348	1.1945	12	0.50	1.41	2.79	0.84